

PUBLIC UTILITY DISTRICT NO. 1 *of* CHELAN COUNTY
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February 26, 2013

VIA ELECTRONIC FILING

Honorable Kimberly D. Bose, Secretary, and
Nathaniel J. Davis, Sr., Deputy Secretary
FEDERAL ENERGY REGULATORY COMMISSION
888 First Street, NE
Washington, DC 20426

Re: Rocky Reach Hydroelectric Project No. 2145
Request for Approval of Rocky Reach Trail Contract Plans and Specifications

Dear Ms. Bose and Mr. Davis:

On October 12, 2010, the Federal Energy Regulatory Commission (Commission) issued the "*Order Modifying and Approving Revised Recreation Management Plan*" for the Rocky Reach Hydroelectric Project (Project). Ordering Paragraph (C) requested that the Public Utility District No. 1 of Chelan County, Washington (Chelan PUD) shall file final construction drawings for the Rocky Reach Trail, including erosion and sedimentation control measures, trash clean-up and removal provisions, and a discussion of how the needs of the disabled were considered in the planning and design of the trail. Chelan PUD respectfully requests expedited review of this filing to accommodate the proposed construction schedule, which is anticipated to begin May 20, 2013.

Plans for the Rocky Reach Trail on Chelan PUD property near the dam were outlined in the Recreation Resources Management and Implementation Plan (RRMIP) submitted to the Federal Energy Regulatory Commission February 12, 2010. The plan anticipated construction of an approximately 1-mile non-motorized trail from Lincoln Rock State Park to an overlook above the fish bypass pipe located downstream from the Rocky Reach Project. The trail would include signs and a bench.

During 2011, consultation with the Washington State Parks and Washington Department of Transportation (an adjacent land-owner) resulted in modifications to the original trail route and design. Specifically, the location of the trail has been moved to ensure a safe distance from the Rocky Reach switchyard, to better follow the topography of the land, and to provide the best aesthetic views of the area. The trail will be about 1-mile in length and will include a foot-bridge to span a small ravine on Chelan PUD property to integrate with future connecting phases of the

trail that is being constructed by other entities on adjacent property owned by the Washington Department of Transportation. A proposed Rocky Reach Trail location map is attached.

In consultation with Washington State Parks, an overlook will not be constructed above the fish bypass pipe due to aesthetic and visual limitations of the site and close proximity to the Rocky Reach Switchyard. Chelan PUD will be contracting with Washington State Parks to construct the Rocky Reach Trail and Chelan PUD's Parks Manager will supervise construction activities.

In accordance with the above Order, Chelan PUD hereby files electronically, with this letter, an electronic file for each of the items listed below. Additionally, in accordance with license article 302, hard copies will be filed with the Commission's Director of Division of Dam Safety and Inspections and the Portland Regional Office by copy of this letter.

1. Contract documents including Technical Specifications
2. Construction Drawings
3. Soil Erosion and Sediment Control Plan
4. Quality Control Inspection Plan

Trash clean up during construction is the responsibility of the contractor as outlined in the "Standard Specifications for Road, Bridge and Municipal Construction, 2012 edition" as issued by Washington State Department of Transportation (WSDOT) and American Public Works Association (APWA). The Standard Specifications, as modified or supplemented by the Amendments to the Standard Specifications and the Special Provisions, all of which are made part of the Contract Documents, shall govern all of the work. http://www.wsdot.wa.gov/Publications/Manuals/M41-10.htm

Following construction, work provisions have been made to locate garbage receptacles along the trail and Washington State Parks under their operating contract with Chelan PUD for Lincoln Rock State Park will be responsible to pick up and collect trash on a regular basis.

The needs of the disabled were taken into consideration on every aspect of the trail design from width and grade of trails to access to the overlook built to American Disabilities Act standards, as is evidenced on the construction drawings.

As part of the planning process, Washington State Parks in consultation with Chelan PUD has received all of the necessary permits: the Douglas County Recreational Overlay and Site Plan Development Permit, Douglas County Shoreline Substantial Development Permit, Construction Stormwater General Permit and Washington Department of Fish and Wildlife Hydraulic Project approval (HPA).

Construction is scheduled to begin about May 20, 2013 and be completed September, 2013. Within 60 days from the date of completion of the Rocky Reach Trail, Chelan PUD will submit for Commission approval, an as-built drawing showing the location, type and layout of the existing and newly completed trail in relation to the project boundary.

Chelan PUD would appreciate any comments you may have as soon as practicable, as it would be helpful in our efforts to maintain the overall project schedule.

Please contact Kris Pomianek of my office at (509) 661-4186 or me if you have any questions or require additional information.

Sincerely,

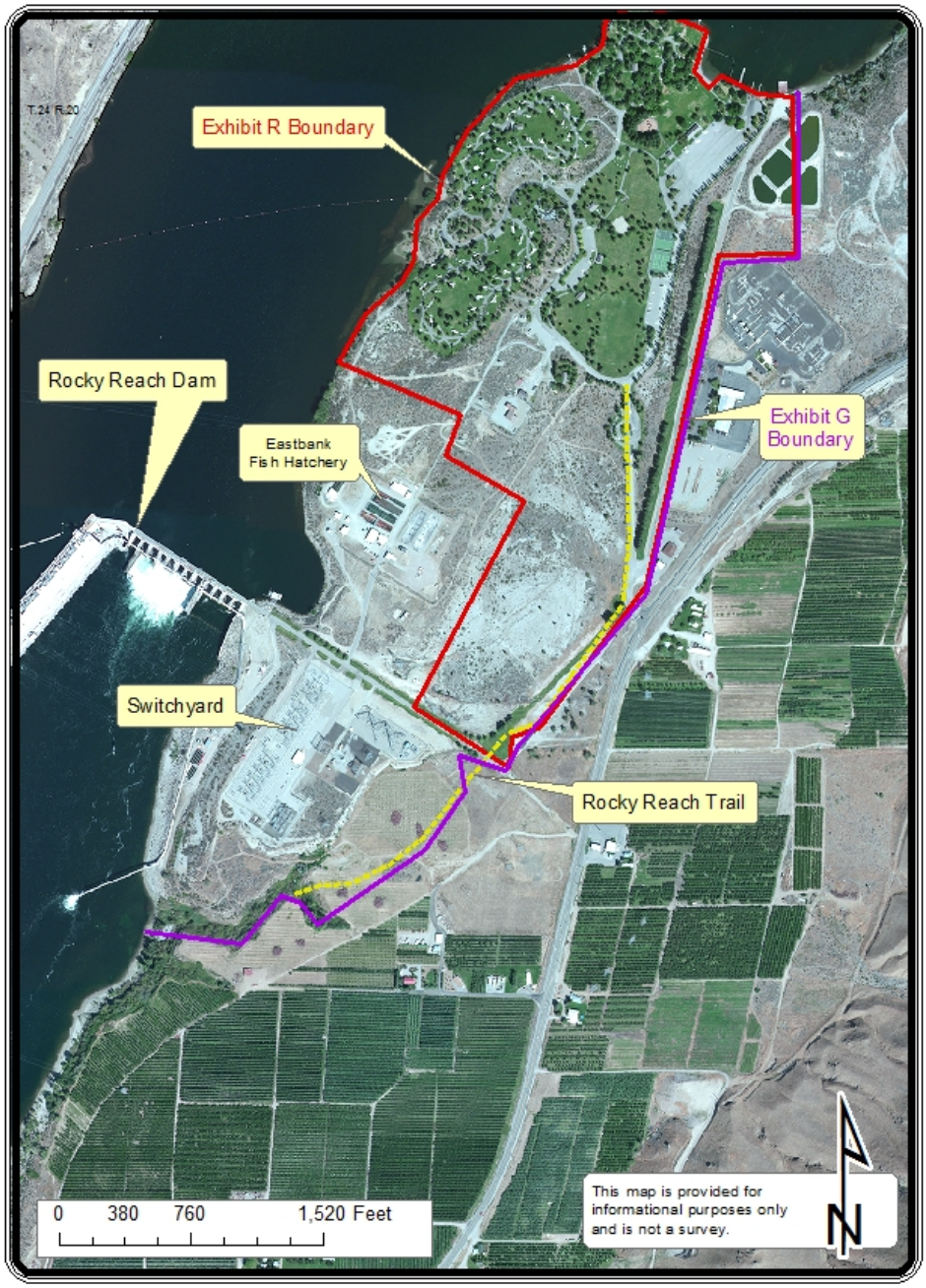
Michelle Smith
Licensing & Compliance Manager
(509) 661-4180
michelle.smith@chelanpud.org

Attachment: Proposed Rocky Reach Trail location map
Association construction specifications, drawings and plans

cc: Commission's Director of Division of Dam Safety & Inspections (hard copy)
Commission's Portland Regional Office (hard copy)
Tony Rapozo, Washington State Parks (letter only)

Proposed Rocky Reach Trail Location Map

Proposed Rocky Reach Trail Location



Contract Provisions and Plans
For Construction of:

Rocky Reach Trail
Phase One

Douglas County, Washington

CONTRACT PROVISIONS

For Construction of:

ROCKY REACH TRAIL
Phase One

Douglas County, Washington

Federal Aid No.: STPE 2009 (020)
Contract No.: _____
Bid Opening Date: _____

WASHINGTON STATE PARKS & RECREATION COMMISSION
1111 ISRAEL ROAD SW
P.O. BOX 42650
OLYMPIA, WASHINGTON 98504-2650

ROCKY REACH TRAIL

Notice to All Plan holders:
Information pertaining to this project may be obtained at the following location:

Washington State Parks and Recreation Commission
Eastern Region Office
270 Ninth Street NE, Suite 200
East Wenatchee, Washington 98802
Telephone: (509) 665-4319

All ***technical questions*** should be directed to Project Manager, Mr. George A. Rapozo, Jr., P.E., at (509) 665-4338.

As the Landscape Architect in direct responsible charge of developing these contract provisions, I certify these provisions have been developed or incorporated into this project under my direct supervision or as a result of certified specifications provided by other licensed professionals.

STATE OF
WASHINGTON
LICENSED
LANDSCAPE ARCHITECT

CONNIE RECKORD
LICENSE NO. 418
EXPIRES ON 4/25/2013

Insert Parks Vicinity Map prior to advertisement.

Insert Letter of Advertisement prior to bidding.

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INTRODUCTION

The following Amendments and Special Provisions shall be used in conjunction with the 2012 Standard Specifications for Road, Bridge, and Municipal Construction.

AMENDMENTS TO THE STANDARD SPECIFICATIONS

The following Amendments to the Standard Specifications are made a part of this contract and supersede any conflicting provisions of the Standard Specifications. For informational purposes, the date following each Amendment title indicates the implementation date of the Amendment or the latest date of revision.

Each Amendment contains all current revisions to the applicable section of the Standard Specifications and may include references which do not apply to this particular project.

Section 1-01, Definition and Terms
August 6, 2012

1-01.3 Definitions

The definition for **"Bid Documents"** is revised to read:

The component parts of the proposed Contract which may include, but are not limited to, the Proposal Form, the proposed Contract Provisions, the proposed Contract Plans, Addenda, and, for projects with Contracting Agency subsurface investigations, the Summary of Geotechnical Conditions and subsurface boring logs (if any).

The definition for **"Superstructures"** is revised to read:

The part of the Structure *above*:

1. The bottom of the grout pad for the simple and continuous span bearing, or

2. The bottom of the block supporting the girder, or

3. Arch skewback and construction joints at the top of vertical abutment members or rigid frame piers.

Longitudinal limits of the Superstructure extend from end to end of the Structure in accordance with the following criteria:

1. From the face of end diaphragm abutting the bridge approach embankment for end piers without expansion joints, or

2. From the end pier expansion joint for bridges with end pier expansion joints.

Superstructures include, but are not limited to, the bottom slab and webs of box girders, the bridge deck and diaphragms of all bridges, and the sidewalks when shown on the bridge deck. The Superstructure also includes the girders, expansion joints, bearings, barrier, and railing attached to the Superstructure when such Superstructure components are not otherwise covered by separate unit measured or lump sum bid items.

Superstructures do not include endwalls, wingwalls, barrier and railing attached to the wingwalls, and cantilever barriers and railings unless supported by the Superstructure.

Section 1-02, Bid Procedures and Conditions
January 2, 2012

1-02.4(2) Subsurface Information
The first two sentences in the first paragraph are revised to read:

If the Contracting Agency has made subsurface investigation of the site of the proposed work, the boring log data, soil sample test data, and geotechnical recommendations reports obtained by the Contracting Agency will be made available for inspection by the Bidders at the location specified in the Special Provisions. The Summary of Geotechnical Conditions, as an appendix to the Special Provisions, and the boring logs shall be considered as part of the Contract.

Section 1-03, Award and Execution of Contract
April 2, 2012

1-03.1(1) Tied Bids
This section's title is revised to read:

1-03.1(1) Identical Bid Totals

Section 1-05, Control of Work
August 6, 2012

1-05.13(1) Emergency Contact List
The second sentence in the first paragraph is revised to read:

The list shall include, at a minimum, the Prime Contractor's Project Manager, or equivalent, the Prime Contractor's Project Superintendent, the Erosion and Sediment Control (ESC) Lead and the Traffic Control Supervisor.

Section 1-06, Control of Material
January 7, 2013

1-06.1(4) Fabrication Inspection Expense
The first paragraph is revised to read:

In the event the Contractor elects to have items fabricated beyond 300 miles from Seattle, Washington, the Contracting Agency will deduct from payment due the Contractor costs to perform fabrication inspection on the following items:

- Bridge Bearings (Cylindrical, Disc, Fabric Pad, Pin, Pendulum, Rocker, and Spherical)
- Cantilever Sign Structures and Sign Bridges
- Epoxy-Coated Reinforcing Steel
- Metal Bridge Railing and Handrail
- Modular Expansion Joints
- Painted Piling and Casing
- Painted and Powder-Coated Luminaire and Signal Poles

- Precast Concrete Catch Basins, Manholes, Inlets, Drywells, and Risers
- Precast Concrete Drain, Perforated Underdrain, Culvert, Storm Sewer, and Sanitary Sewer Pipe
- Precast Concrete Three Sided Structures
- Precast Concrete Junction Boxes, Pull Boxes, Cable Vaults, Utility Vaults, and Box Culverts
- Precast Concrete Traffic Barrier
- Precast Concrete Marine Pier Deck Panels
- Precast Concrete Floor Panels
- Precast Concrete Structural Earth Walls, Noise Barrier Walls, and Wall Stem Panels
- Precast Concrete Retaining Walls, including Lagging Panels
- Prestressed Concrete Girders and Precast Bridge Components
- Prestressed Concrete Piles
- Seismic Retrofit Earthquake Restrainers
- Soldier Piles
- Steel Bridges and Steel Bridge Components
- Steel Column Jackets
- Structural Steel for Ferry Terminals, including items such as Dolphins, Wingwalls, and Transfer Spans
- Treated Timber and Lumber 6-inch by 6-inch or larger
- Timber
- Additional items as may be determined by the Engineer

The footnote below the table is revised to read:

* An inspection day includes any calendar day or portion of a calendar day spent by one inspector inspecting, on standby, or traveling to and from a place of fabrication. An additional cost per inspection day will be assessed for each additional inspector. Reimbursement will be assessed at $280.00 per day for weekends and holidays for each on site inspector in travel status, but not engaged in inspection or travel activities when fabrication activities are not taking place.

Section 1-07, Legal Relations and Responsibilities to the Public
January 7, 2013

1-07.1 Laws to be Observed

The following two sentences are inserted after the first sentence in the third paragraph:

In particular the Contractor's attention is drawn to the requirements of WAC 296.800 which requires employers to provide a safe workplace. More specifically WAC 296.800.11025 prohibits alcohol and narcotics from the workplace.

1-07.9(2) Posting Notices

This section is revised to read:

Notices and posters shall be placed in areas readily accessible to read by employees. The Contractor shall ensure the following are posted:

1. EEOC - P/E-1 (revised 11/09) - Equal Employment Opportunity is THE LAW published by US Department of Labor. Post for projects with federal-aid funding

2. FHWA-1022 (revised 11/11) - NOTICE Federal-Aid Project published by Federal Highway Administration (FHWA). Post for projects with federal-aid funding

3 WH 1321 (revised 04/09) - Employee Rights under the Davis-Bacon Act published by US Department of Labor. Post for projects with federal-aid funding

4. WHD 1088 (revised 07/09) - Employee Rights under the Fair Labor Standards Act published by US Department of Labor. Post on all projects

5. WHD - 1420 (revised 01/09) - Employee Rights and Responsibilities under The Family and Medical Leave Act published by US Department Of Labor. Post on all projects

6. WHD-1462 (revised 01/12) – Employee Polygraph Protection Act published by US Department of Labor. Post on all projects

7. F416-081-909 (revised 12/12) - Job Safety and Health Law published by Washington State Department of Labor and Industries. Post on all projects

8. F242-191-909 (revised 12/12) - Notice to Employees published by Washington State Department of Labor and Industries. Post on all projects

9. F700-074-909 (revised 12/12) - Your Rights as a Worker in Washington State by Washington State Department of Labor and Industries (L&I). Post on all projects

10. EMS 9874 (revised 04/12) - Unemployment Benefits published by Washington State Employee Security Department. Post on all projects

11. Post one copy of the approved “Statement of Intent to Pay Prevailing Wages” for the Contractor, each Subcontractor, each lower tier subcontractor, and any other firm (Supplier, Manufacturer, or Fabricator) that falls under the provisions of RCW 39.12 because of the definition of “Contractor” in WAC 296-127-010

12. Post one copy of the prevailing wage rates for the project

1-07.14 Responsibility for Damage

The fifth paragraph is revised to read:

Pursuant to RCW 4.24.115, if such claims, suits, or actions result from the concurrent negligence of (a) the indemnitee or the indemnitee’s agents or employees and (b) the Contractor or the Contractor’s agent or employees, the indemnity provisions provided in the preceding paragraphs of this Section shall be valid and enforceable only to the extent of the Contractor’s negligence or the negligence of its agents and employees.

1-07.15 Temporary Water Pollution/Erosion Control

The third paragraph is deleted.

Section 1-08, Prosecution and Progress
April 2, 2012

1-08.1 Subcontracting
In the eighth paragraph, “Contracting Agency” is revised to read “WSDOT”.

1-08.3(1) General Requirements
The following new paragraph is inserted after the first paragraph:

Total float belongs to the project and shall not be for the exclusive benefit of any party.

1-08.7 Maintenance During Suspension
The second paragraph is revised to read:

At no expense to the Contracting Agency, the Contractor shall provide through the construction area safe, smooth, and unobstructed roadways and pedestrian access routes for public use during the suspension (as required in Section 1-07.23 or the Special Provisions.) This may include a temporary road, alternative pedestrian access route or detour.

Section 1-09, Measurement and Payment
August 6, 2012

1-09.1 Measurement of Quantities
The following new sentence is inserted after the sentence ““Ton”:2,000 pounds of avoirdupois weight”:

Items of payment that have “Lump Sum” or “Force Account” in the Bid Item of Work shall have no specific unit of measurement requirement.

1-09.2(5) Measurement
The second sentence in the first paragraph is revised to read:

The frequency of verification checks will be such that at least one test weekly is performed for each scale used in weighing contract items of Work.

Section 3-04, Acceptance of Aggregate
April 2, 2012

3-04.3(7)D4 An Entire Lot
The last sentence is deleted.

3-04.5 Payment
In the second paragraph, the reference “Section 3-04.3(6)C “ is revised to read “Section 3-04.3(8)“.

In Table 1, the row containing the item “Gravel Borrow for Geosynthetic Retaining Wall” is revised to read:

9-03.14(4)	Gravel Borrow for Geosynthetic Retaining Wall	4000	2000	$30	$60

Section 5-01, Cement Concrete Pavement Rehabilitation
April 2, 2012

5-01.3(2)B Portland Cement Concrete
The fifth sentence in the third paragraph is revised to read:

The lower Specification limit for compressive strength shall be 4,000-psi.

The last two sentences in the third paragraph are deleted.

5-01.3(11) Concrete Slurry
This section including title is revised to read:

5-01.3(11) Concrete Slurry and Grinding Residue

All concrete slurry and grinding residue shall be removed from the pavement surface on a continual basis immediately behind the grinding or cutting operations. Slurry shall not be allowed to drain into an area open to traffic, off of the paved surface or into any drainage structure.

The Contractor shall collect the concrete slurry and grinding residue from the pavement surface and dispose of it in accordance with Section 2-03.3(7)C.

Opening to traffic shall meet the requirements of Section 5-05.3(17).

Section 5-02, Bituminous Surface Treatment
August 6, 2012

5-02.2 Materials
The following new paragraph is inserted after the second paragraph:

Each source of aggregate for bituminous surface treatment shall be evaluated separately for acceptance in accordance with Section 3-04.

Section 5-04, Hot Mix Asphalt
January 7, 2013

5-04.2 Materials
The fourth paragraph is revised to read:

The grade of asphalt binder shall be as required by the Contract. Blending of asphalt binder from different sources is not permitted.

5-04.3(7)A1 General
This section is supplemented with the following:

The Contractor shall include the brand and type of anti-stripping additive in the mix design submittal and provide certification from the asphalt binder manufacture that the anti-stripping

additive is compatible with the crude source and formulation of asphalt binder proposed in mix design.

5-04.3(7)A3 Commercial Evaluation
The second sentence in the second paragraph is deleted.

5-04.3(10)B3 Longitudinal Joint Density
The section including title is revised to read:

5-04.3(10)B3 Vacant

5-04.3(11)D General
The last sentence in the first paragraph is deleted.

5-04.3(20) Anti-Stripping Additive
This section is revised to read:

Anti-stripping additive shall be added to the liquid asphalt by the asphalt supplier prior to shipment to the asphalt mixing plant. For HMA accepted by statistical and nonstatistical evaluation the anti-stripping additive shall be added in the amount designated in the WSDOT mix design/anti-strip evaluation report provided by the Contracting Agency. For HMA accepted by commercial evaluation the Project Engineer will determine the amount of anti-strip to be added; paving shall not begin before the anti-strip requirements have been provided to the Contractor.

5-04.4 Measurement
The last paragraph is deleted.

5-04.5 Payment
The bid item “Longitudinal Joint Density Price Adjustment”, by calculation and paragraph following bid item are deleted.

Section 6-02, Concrete Structures
January 7, 2013

6-02.3(2) Proportioning Materials
The Lean Concrete value in the column “Minimum Cemetitious Content (pounds)” in the table titled “Cementitious Requirement for Concrete” is revised to read:

****145

The following new note is inserted after the note “*** No maximum specified” in the table titled “Cementitious Requirement for Concrete”:

****Maximum of 200 pounds

The paragraph following the table “Cementitious Requirements for Concrete” is revised to read:

When both ground granulated blast furnace slag and fly ash are included in the concrete mix, the total weight of both these materials is limited to 40 percent by weight of the total

cementitious material for concrete Class 4000D and 4000A, and 50 percent by weight of the total cementitious material for all other classes of concrete.

6-02.3(2)B Commercial Concrete

The second paragraph is revised to read:

Where concrete Class 3000 is specified for items such as, culvert headwalls, plugging culverts, concrete pipe collars, pipe anchors, monument cases, Type PPB, PS, I, FB and RM signal standards, pedestals, cabinet bases, guardrail anchors, fence post footings, sidewalks, curbs, and gutters, the Contractor may use commercial concrete. If commercial concrete is used for sidewalks, curbs, and gutters, it shall have a minimum cementitious material content of 564 pounds per cubic yard of concrete, shall be air entrained, and the tolerances of Section 6-02.3(5)C shall apply.

6-02.3(2)D Lean Concrete

This section is revised to read:

Lean concrete shall meet the cementitious requirements of Section 6-02.3(2) and have a maximum water/cement ratio of 2.

6-02.3(4)A Qualification of Concrete Suppliers

The first paragraph is revised to read :

Batch Plant Prequalification requires a certification by the National Ready Mix Concrete Association (NRMCA). Information concerning NRMCA certification may be obtained from the NRMCA at 900 Spring Street, Silver Springs, MD 20910 or online at www.nrmca.org. The NRMCA certification shall be valid for a 2-year period from the date of certificate. The following documentation shall be submitted to the Project Engineer; a copy of the current NRMCA Certificate of Conformance, the concrete mix design(s) (WSDOT Form 350-040), along with copies of the truck list, batch plant scale certification, admixture dispensing certification, and volumetric water batching devices (including water meters) verification.

6-02.3(5)G Sampling and Testing Frequency for Temperature, Consistency, and Air Control

The last sentence in the second paragraph is revised to read:

Sampling shall be performed in accordance with WSDOT FOP for WAQTC TM 2 and random samples shall be selected in accordance with WSDOT TM 716.

6-02.3(14)C Pigmented Sealer for Concrete Surfaces

This section is revised to read:

The Contractor shall submit the pigmented sealer manufacturer's written instructions covering, at a minimum, the following:

1. Surface preparation
2. Application methods
3. Requirements for concrete curing prior to sealer application

4. Temperature, humidity and precipitation limitations for application

5. Rate of application and number of coats to apply

The Contractor shall not begin applying pigmented sealer to the surfaces specified to receive the sealer until receiving the Engineer's approval of the submittal.

All surfaces specified in the Plans to receive pigmented sealer shall receive a Class 2 surface finish (except that concrete barrier surfaces shall be finished in accordance with Section 6-02.3(11)A). The Contractor shall not apply pigmented sealer from a batch greater than 12 months past the initial date of color sample approval of that batch by the Engineer.

The pigmented sealer color or colors for specific concrete surfaces shall be as specified in the Special Provisions.

The final appearance shall be even and uniform without blotchiness, streaking or uneven color. Surface finishes deemed unacceptable by the Engineer shall be re-coated in accordance with the manufacturer's recommendations at no additional expense to the Contracting Agency.

For concrete surfaces such as columns, retaining walls, pier walls, abutments, concrete fascia panels, and noise barrier wall panels, the pigmented sealer shall extend to 1 foot below the finish ground line, unless otherwise shown in the Plans.

6-02.3(16) Plans for Falsework and Formwork

Item No. 4 in the seventh paragraph is revised to read:

4. Conditions required by other Sections of 6-02.3(17), Falsework and Formwork.

Item's No. 5, 6, 7, and 8 in the seventh paragraph are deleted.

The following paragraph is inserted after the seventh paragraph:

Plan approval can be done by the Project Engineer for footings and walls 4 to 8 feet high (excluding pedestal height) provided:

1. Concrete placement rate is 4 feet per hour or less.

2. Facing is ¾-inch plywood with grades as specified per Section 6-02.3(17)I.

3. Studs, with plywood face grain perpendicular, are 2 by 4's spaced at 12 inches.

4. Walers with 3,000 pound safe working load ties spaced at 24 inches are two 2 by 4's spaced at 24 inches.

6-02.3(17)F Bracing

In the first paragraph, the phrase "per Section 6-02.3(17)I" is revised to read "in accordance with Section 6-02.3(17)I".

This section is supplemented with the following new sub-section:

6-02.3(17)F5 Temporary Bracing for Bridge Girders During Diaphragm and Bridge Deck Concrete Placement

Prestressed concrete girders shall be braced to resist forces that would cause rotation or torsion in the girders caused by the placing of precast concrete deck panels and concrete for the bridge deck.

Bracing shall be designed and detailed by the Contractor and shall be shown in the falsework/formwork plans submitted to the Engineer for approval. These braces shall be furnished, installed, and removed by the Contractor at no additional cost to the Contracting Agency. The Contractor may consider the bracing effects of the diaphragms in developing the falsework/formwork plans. The Contractor shall account for the added load from concrete finishing machines and other construction loadings in the design of the bracing.

Falsework support brackets and braces shall not be welded to structural steel bridge members or to steel reinforcing bars.

6-02.3(17)F4 Temporary Bracing for Bridge Girders

This section including title is revised to read:

6-02.3(17)F4 Temporary Bracing for Bridge Girders During Erection

Steel girders shall be braced in accordance with Section 6-03.3(7)A.

Prestressed concrete girders shall be braced sequentially during girder erection. The bracing shall be designed and detailed by the Contractor and shall be shown in the falsework/formwork plans submitted to the Engineer for approval. The Contractor shall furnish, install, and remove the bracing at no additional cost to the Contracting Agency.

At a minimum, the Contractor shall brace girders at each end and at midspan to prevent lateral movement or rotation. This bracing shall be placed prior to the release of each girder from the erection equipment. If the bridge is constructed with cast-in-place concrete diaphragms, the bracing may be removed once the concrete in the diaphragms has been placed and cured for a minimum of 24 hours.

6-02.3(17)H Formwork Accesories

The first paragraph is deleted and replaced with the following two new paragraphs:

Formwork accessories such as form ties, form anchors, form hangers, anchoring inserts, and similar hardware shall be specifically identified in the formwork plans including the name and size of the hardware, manufacturer, safe working load, and factor of safety. The grade of steel shall also be indicated for threaded rods, coil rods, and similar hardware. Wire form ties shall not be used. Welding or clamping formwork accessories to Contract Plan reinforcing steel will not be allowed. Driven types of anchorages for fastening forms or form supports to concrete, and Contractor fabricated “J” hooks shall not be used. Field drilling of holes in prestressed girders is not allowed.

Taper ties may be used provided the following conditions are met:

1. The structure is not designed to resist water pressure (pontoons, floating dolphins, detention vaults, etc.)

2. After the taper tie is removed, plugs designed and intended for plugging taper tie holes shall be installed at each face of concrete. The plug shall be installed a minimum of 1 ½" clear from the face of concrete.

3. After the plug is installed, the hole shall be cleaned of all grease, contamination and foreign matter.

4. Holes on the exposed faces of concrete shall be patched and finished to match the surrounding concrete.

6-02.3(25)N Prestressed Concrete Girder Erection

The third sentence in the fifth paragraph is revised to read:

The girders shall be braced in accordance with Sections 6-02.3(17)F4 and 6-02.3(17)F5.

6-02.3(26)E5 Leak Tightness Testing

The first sentence in the first paragraph is revised to read:

The Contractor shall test each completed duct assembly for leak tightness after placing concrete but prior to placing post tensioning reinforcement.

The second paragraph is revised to read:

Prior to testing, all grout caps shall be installed and all vents, grout injection ports, and drains shall either be capped or have their shut-off valves closed. The Contractor shall pressurize the completed duct assembly to an initial air pressure of 50 psi. This pressure shall be held for five minutes to allow for internal adjustments within the assembly. After five minutes, the air supply valve shall be closed. The Contractor shall monitor and measure the pressure maintained within the closed assembly, and any subsequent loss of pressure, over a period of one minute following the closure of the air supply valve. The maximum pressure loss for duct assemblies equal to or less than 150 feet in length shall be 25 psig. The maximum pressure loss for duct assemblies greater than 150 feet in length shall be 15 psig. If the pressure loss exceeds the allowable, locations of leakage shall be identified, repaired or reconstructed using methods approved by the Engineer. The repaired system shall then be retested. The cycle of testing, repair and retesting of each completed duct assembly shall continue until the completed duct assembly completes a test with pressure loss within the specified amount.

Section 6-03, Steel Structures
April 2, 2012

6-03.3(28)A Method of Shop Assembly

The first sentence in Item 2.C. is revised to read:

For Trusses and Girders – After the first stage has been completed, each subsequent stage shall be assembled to include: at least one truss panel or girder shop section of the previous stage and two or more truss panels or girder shop sections added at the advancing end.

Section 6-05, Piling
August 6, 2012

6-05.5 Payment
The paragraph following the bid item, “Driving St. Pile”, per each is revised to read:

The unit Contract price per each for “Driving (type) Pile (____)” shall be full pay for driving the pile to the ultimate bearing and/or penetration specified.

Section 6-06, Bridge Railings
August 6, 2012

6-06.3(2) Metal Railings
The third paragraph is revised to read:

Anchor bolts shall be positioned with a template to ensure that bolts match the hole spacing of the bottom channels or anchorage plates.

Section 6-07, Painting
April 2, 2012

6-07.3(9)A Paint System
The first sentence in the second paragraph is revised to read:

All paint coating components of the selected paint system shall be produced by the same manufacturer.

6-07.3(10)H Paint System
The first and second sentences in the second paragraph are revised to read:

All paint coating components of the selected paint system shall be produced by the same manufacturer.

Section 6-10, Concrete Barrier
August 6, 2012

6-10.3 Construction Requirements
This section is supplemented with the following:

Steel welded wire reinforcement deformed, conforming to Section 9-07.7, may be substituted in concrete barrier in place of deformed steel bars conforming to Section 9-07.2, subject to the following conditions:

1. Steel welded wire reinforcement spacing shall be the same as the deformed steel bar spacing as shown in the Standard Plans.

2. The minimum cross sectional area for steel welded wire reinforcement shall be no less than 86 percent of the cross sectional area for the deformed steel bars being substituted.

3. Development lengths and splice lengths shall conform to requirements specified in the AASHTO LRFD Bridge Design Specifications, current edition.

6-10.5 Payment

In the second paragraph, the bid item “Conc. Class 4000” is revised to read:

“Conc. Class 4000___”

Section 6-12, Noise Barrier Walls
August 6, 2012

6-12.3(3) Shaft Construction

The third sentence in the fifth paragraph is revised to read:

When efforts to advance past the obstruction to the design shaft tip elevation result in the rate of advance of the shaft drilling equipment being significantly reduced relative to the rate of advance for the rest of the shaft excavation, then the Contractor shall remove the obstruction under the provisions of Section 6-12.5.

6-12.3(6) Precast Concrete Panel Fabrication and Erection

The second sentence in item number 3 is deleted.

6-12.5 Payment

This section is supplemented with the following:

“Removing Noise Barrier Wall Shaft Obstructions”, estimated.

Payment for removing obstructions, as defined in Section 6-12.3(3), will be made for the changes in shaft construction methods necessary to remove the obstruction. The Contractor and the Engineer shall evaluate the effort made and reach agreement on the equipment and employees utilized, and the number of hours involved for each. Once these cost items and their duration have been agreed upon, the payment amount will be determined using the rate and markup methods specified in Section 1-09.6. For the purpose of providing a common proposal for all bidders, the Contracting Agency has entered an amount for the item "Removing Noise Barrier Wall Shaft Obstructions" in the bid proposal to become a part of the total bid by the Contractor.

If the shaft construction equipment is idled as a result of the obstruction removal work and cannot be reasonably reassigned within the project, then standby payment for the idled equipment will be added to the payment calculations. If labor is idled as a result of the obstruction removal work and cannot be reasonably reassigned within the project, then all labor costs resulting from Contractor labor agreements and established Contractor policies will be added to the payment calculations.

The Contractor shall perform the amount of obstruction work estimated by the Contracting Agency within the original time of the contract. The Engineer will consider a time adjustment and additional compensation for costs related to the extended duration of the shaft construction operations, provided:

1. the dollar amount estimated by the Contracting Agency has been exceeded, and;

2. the Contractor shows that the obstruction removal work represents a delay to the completion of the project based on the current progress schedule provided in accordance with Section 1-08.3.

Section 6-14, Geosynthetic Retaining Walls
January 2, 2012

6-14.2 Materials
The referenced section for the following item is revised to read:

Grout	9-20.3(4)

In the first paragraph, the following items are inserted after the item “Gravel Borrow For Geosynthetic Retaining Wall”:

Polyurethane Sealant	9-04.2(3)
Closed Cell Foam Backer Rod	9-04.2(3)A

Section 6-15, Soil Nail Walls
January 2, 2012

6-15.2 Materials
The referenced section for the following item is revised to read:

Grout	9-20.3(4)

6-15.3(3) Submittals
Item f beneath item number 3 is revised to read:

f. Mix design and procedures for placing the grout.

6-15.3(6) Soil Nailing
This section is supplemented with the following:

The Contractor shall make and cure grout cubes once per day in accordance with WSDOT Test Method T 813. These samples shall be retained by the Contractor until all associated verification and proof testing of the soil nails has been successfully completed. If the Contractor elects to test the grout cubes for compressive strength, testing shall be conducted by an independent laboratory and shall be in accordance with the WSDOT FOP for AASHTO T106.

Section 6-16, Soldier Pile and Soldier Pile Tieback Walls
January 2, 2012

6-16.3(3) Shaft Excavation
The third sentence in the seventh paragraph is revised to read:

When efforts to advance past the obstruction to the design shaft tip elevation result in the rate of advance of the shaft drilling equipment being significantly reduced relative to the rate of

advance for the rest of the shaft excavation, then the Contractor shall remove the obstruction under the provisions of Section 6-16.5.

6-16.5 Payment

This section is supplemented with the following:

"Removing Soldier Pile Shaft Obstructions", estimated.

Payment for removing obstructions, as defined in Section 6-16.3(3), will be made for the changes in shaft construction methods necessary to remove the obstruction. The Contractor and the Engineer shall evaluate the effort made and reach agreement on the equipment and employees utilized, and the number of hours involved for each. Once these cost items and their duration have been agreed upon, the payment amount will be determined using the rate and markup methods specified in Section 1-09.6. For the purpose of providing a common proposal for all bidders, the Contracting Agency has entered an amount for the item "Removing Soldier Pile Shaft Obstructions" in the bid proposal to become a part of the total bid by the Contractor.

If the shaft construction equipment is idled as a result of the obstruction removal work and cannot be reasonably reassigned within the project, then standby payment for the idled equipment will be added to the payment calculations. If labor is idled as a result of the obstruction removal work and cannot be reasonably reassigned within the project, then all labor costs resulting from Contractor labor agreements and established Contractor policies will be added to the payment calculations.

The Contractor shall perform the amount of obstruction work estimated by the Contracting Agency within the original time of the contract. The Engineer will consider a time adjustment and additional compensation for costs related to the extended duration of the shaft construction operations, provided:

1. the dollar amount estimated by the Contracting Agency has been exceeded, and;

2. the Contractor shows that the obstruction removal work represents a delay to the completion of the project based on the current progress schedule provided in accordance with Section 1-08.3.

Section 6-17, Permanent Ground Anchors
August 6, 2012

6-17.3(3) Submittals

The first sentence in the sixth paragraph is revised to read:

The Contractor shall submit the mix design for the grout conforming to Section 9-20.3(4) and the procedures for placing the grout to the Engineer for approval.

6-17.3(7) Installing Permanent Ground Anchors

The following new paragraph is inserted after the sixth paragraph:

The Contractor shall make and cure grout cubes once per day in accordance with WSDOT Test Method T 813. These samples shall be retained by the Contractor until all associated

verification, performance and proof testing of the permanent ground anchors has been successfully completed. If the Contractor elects to test the grout cubes for compressive strength, testing shall be conducted by an independent laboratory and shall be in accordance with the WSDOT FOP for AASHTO T106.

6-17.3(9) Permanent Ground Anchor Acceptance Criteria

The fourth paragraph is deleted.

Section 6-19, Shafts
August 6, 2012

6-19.3(4)F Slurry Disposal

This section including title is revised to read:

6-19.3(4)F Disposal of Slurry and Slurry Contacted Spoils

The Contractor shall dispose of the slurry and slurry-contacted spoils as specified in the shaft installation narrative in accordance with Section 6-19.3(2)B, item 8, and in accordance with the following requirements:

1. Water slurry with no additives may be infiltrated to an upland area within the confines of the Contracting Agency Right of Way for the project. Infiltration is allowed provided the ground-line at the disposal site is at least 5 feet above the current water table, and that disposal operations conform to the temporary erosion and sedimentation control (TESC) requirements established for this project. For the purposes of water slurry disposal, upland is defined as an area that has no chance of discharging directly to waters of the State, including wetlands or conveyances that indirectly lead to wetlands or waters of the State. Spoils in contact with this slurry may be disposed of as clean fill.

2. Synthetic slurry and water slurry with polymer-based additives shall be contained and disposed of by the Contractor at an approved facility. The Contractor shall acquire all permits or approvals necessary for disposal of the slurry and shall provide copies to the Engineer. Spoils in contact with synthetic slurry or water slurry with polymer-based additives shall be disposed of in accordance with Section 2-03.3(7)C. With approval of the Engineer, the Contractor may re-use these spoils on-site.

3. Mineral slurry may be infiltrated to a temporary sediment trap located in an upland area within the confines of the Contracting Agency Right of Way for the project. Infiltration is allowed provided the ground-line at the disposal site is at least 5 feet above the current water table, and that disposal operations conform to the temporary erosion and sedimentation control (TESC) requirements established for this project. For the purposes of mineral slurry disposal, upland is defined as an area that has no chance of discharging directly to waters of the State, including wetlands or conveyances that indirectly lead to wetlands or waters of the State.

Spoils in contact with mineral slurry shall be disposed of in accordance with Section 2-03.3(7)C. With approval of the Engineer, the Contractor may re-use these spoils on-site.

Section 7-02, Culverts
August 6, 2012

7-02.2 Materials
Note 3 in the table titled, "Culvert Pipe Schedules" is revised to read:

[3]Polypropylene pipe, 12 inch to 30 inch diameters approved for Schedule A and Schedule B, 36 inch to 60 inch diameters approved for Schedule A only.

7-02.5
The bid item "Steel Rib Reinforced Polyethylene Culvert Pipe _____ In. Diam.", per linear foot is revised to read:

"St. Rib Reinf Polyethylene Culv. Pipe _____ In. Diam.", per linear foot

Section 7-03, Structural Plate Pipe, Pipe Arch, Arch, and Underpass
August 6, 2012

7-03.3(1) Foundations, General
This section is supplemented with the following:

When aluminum pipe or pipe arch is in contact with cement concrete, two coats of paint shall be applied in accordance with Section 7-08.3(2)D.

7-03.3(5) Headwalls
This section is supplemented with the following:

When aluminum pipe or pipe arch is in contact with cement concrete, two coats of paint shall be applied in accordance with Section 7-08.3(2)D.

Section 7-04, Storm Sewers
August 6, 2012

7-04.3(1)B Exfiltration Test – Storm Sewers
The fifth column title "PE[4]" is revised to read "PP[4]" from the table titled, "Storm Sewer Pipe Schedules".

Note 4 in the table titled, "Storm Sewer Pipe Schedules" is revised to read:

[4]PP = Polypropylene Pipe, 12 inch to 30 inch approved for Schedule A and Schedule B, 36 inch to 60 inch diameters approved for Schedule A only.

7-04.5
The bid item "Steel Rib Reinforced Polyethylene Storm Sewer Pipe _____ In Diam", per linear foot is revised to read:

"St. Rib Reinf Polyethylene Storm Sewer Pipe _____ In. Diam", per linear foot

Section 7-05, Manholes, Inlets, Catch Basins, and Drywells
April 2, 2012

7-05.3 Construction Requirements
The third paragraph is supplemented with the following:

Leveling and adjustment devices that do not modify the structural integrity of the metal frame, grate or cover, and do not void the originating foundry's compliance to these specifications and warranty is allowed. Approved leveling devices are listed in the Qualified Products List. Leveling and adjusting devices that interfere with the backfilling, backfill density, grouting and asphalt density will not be allowed. The hardware for leveling and adjusting devices shall be completely removed when specified by the Project Engineer.

Section 7-08, General Pipe Installation Requirements
August 6, 2012

7-08.3(2)D Pipe Laying – Steel or Aluminum
The following new sentence is inserted after the first sentence in the second paragraph:

The paint shall cover all the surface in contact with the concrete and extend one inch beyond the point of contact.

Section 7-09, Water Mains
August 6, 2012

7-09.3(19)A Connections to Existing Mains
In the second paragraph, "Special Conditions" is revised to read "Special Provisions".

Section 8-01, Erosion Control and Water Pollution Control
January 7, 2013

8-01.2 Materials
The first paragraph is revised to read:

Materials shall meet the requirements of the following sections:

Corrugated Polyethylene Drain Pipe	9-05.1(6)
Quarry Spalls	9-13
Seed	9-14.2
Fertilizer	9-14.3
Mulch and Amendments	9-14.4
Tackifiers	9-14.4(7)
Erosion Control Devices	9-14.5
High Visibility Fence	9-14.5
Construction Geotextile	9-33

8-01.3(2)D Mulching
The following two new paragraphs are inserted after the fourth paragraph:

Short-Term Mulch shall be hydraulically applied at the rate of 2500 pounds per acre and may be applied in one lift.

Moderate-Term Mulch and Long-Term Mulch shall be hydraulically applied at the rate of 3500 pounds per acre with no more than 2000 pounds applied in any single lift.

8-01.3(2)E Soil Binders and Tacking Agents

This section including title is revised to read:

8-01.3(2)E Tackifiers

Tackifiers applied using a hydroseeder shall have a mulch tracer added to visibly aid uniform application. This tracer shall not be harmful to plant, aquatic, or animal life. A minimum of 125 pounds per acre and a maximum of 250 pounds per acre of Short-Term Mulch shall be used as a tracer. Tackifier shall be mixed and applied in accordance with the manufacturer's recommendations.

Soil Binding Using Polyacrylamide (PAM) – The PAM shall be applied on bare soil completely dissolved and mixed in water or applied as a dry powder. Dissolved PAM shall be applied at a rate of not more than⅔ pound per 1,000 gallons of water per acre. A minimum of 200 pounds per acre of Short-Term Mulch shall be applied with the dissolved PAM. Dry powder applications may be at a rate of 5 pounds per acre using a hand-held fertilizer spreader or a tractor-mounted spreader.

PAM shall be applied only to areas that drain to completed sedimentation control BMPs in accordance with the TESC Plan. PAM may be reapplied on actively worked areas after a 48-hour period.

PAM shall not be applied during rainfall or to saturated soils

8-01.3(2)F Dates for Application of Final Seed, Fertilizer, and Mulch

In the first paragraph, "Engineer" is revised to read "Project Engineer".

Note 1 of the table in the first paragraph is revised to read:

[1] Where Contract timing is appropriate, seeding, fertilizing, and mulching shall be accomplished during the fall period listed above

The third paragraph is deleted.

8-01.3(3) Placing Erosion Control Blanket

This section including title is revised to read:

8-01.3(3) Placing Biodegradable Erosion Control Blanket

Biodegradable Erosion Control Blankets are used as an erosion prevention device and to enhance the establishment of vegetation. Erosion control blankets shall be installed according to the manufacturer's recommendations.

Seeding and fertilizing shall be done prior to blanket installation.

Select erosion control blanket material for an area based on the intended function: slope or ditch stabilization, and site specific factors including soil, slope gradient, rainfall, and flow exposure. Erosion Control Blankets shall not be used on slopes or in ditches that exceed the manufacturer's recommendations.

8-01.3(4) Placing Compost Blanket

The first paragraph is revised to read:

Compost blanket shall be placed to a depth of 3 inches over bare soil. Compost blanket shall be placed prior to seeding or other planting. An organic tackifier shall be placed over the entire composted area when dry or windy conditions are present or expected before the final application of mulch or erosion control blanket. The tackifier shall be applied immediately after the application of compost to prevent compost from leaving the composted area.

8-01.3(5) Placing Plastic Covering

The second and third paragraphs are revised to read:

Clear plastic covering shall be used to promote seed germination when seeding is performed outside of the Dates for Application of Final Seed in Section 8-01.3(2)F. Black plastic covering shall be used for stockpiles or other areas where vegetative growth is unwanted.

The plastic cover shall be installed and maintained in a way that prevents water from cutting under the plastic and prevents the plastic cover from blowing open in the wind.

8-01.3(6) Check Dams

This section is revised to read:

Check dams shall be installed as soon as construction will allow, or when designated by the Engineer. The Contractor may substitute a different check dam, in lieu of what is specified in the contract, with approval of the Engineer. The check dam is a temporary or permanent structure, built across a minor channel. Water shall not flow through the check dam structure. Check dams shall be constructed in a manner that creates a ponding area upstream of the dam to allow pollutants to settle, with water from increased flows channeled over a spillway in the check dam. The check dam shall be constructed to prevent erosion in the area below the spillway. Check dams shall be placed perpendicular to the flow of water and installed in accordance with the Standard Plans. The outer edges shall extend up the sides of the conveyance to prevent water from going around the check dam. Check dams shall be of sufficient height to maximize detention, without causing water to leave the ditch. Check dams shall meet the requirements in Section 9-14.5(4).

8-01.3(6)A Geotextile-Encased Check Dam

This sections content including title is deleted.

8-01.3(6)B Quarry Spall Check Dam

This sections content including title is deleted.

8-01.3(6)C Sandbag Check Dam

This sections content including title is deleted.

8-01.3(6)D Wattle Check Dam
This sections content including title is deleted.

8-01.3(6)E Coir Log
This sections title is revised to read:

8-01.3(6)A Coir Log

8-01.3(7) Stabilized Construction Entrance
The first paragraph is revised to read:

Temporary stabilized construction entrance shall be constructed in accordance with the Standard Plans, prior to beginning any clearing, grubbing, embankment or excavation. All quarry spall material used for stabilized construction entrance shall be free of extraneous materials that may cause or contribute to track out.

8-01.3(9)B Gravel Filter, Wood Chip, or Compost Berm
The first paragraph is revised to read:

Filter berms shall retain sediment and direct flows. The gravel filter berm shall be a minimum of 1 foot in height and shall be maintained at this height for the entire time they are in use. Rock material used for filter berms shall meet the grading requirements in Section 9-03.9(2), but shall not include any recycled materials as outlined in Section 9-03.21.

8-01.3(9)C Straw Bale Barrier
This section including title is revised to read:

8-01.3(9)C Vacant

8-01.3(11) Vacant
This section including title is revised to read:

8-01.3(11) Outlet Protection
Outlet protection shall prevent scour at the outlets of ponds, pipes, ditches or other conveyances. All quarry spall material used for outlet protection shall be free of extraneous material and meet the gradation requirements in Section 9-13.6.

8-01.3(13) Temporary Curb
This section is revised to read:

Temporary curbs shall divert or redirect water around erodible soils.

Temporary curbs shall be installed along pavement edges to prevent runoff from flowing onto erodible slopes. Water shall be directed to areas where erosion can be controlled. The temporary curbs shall be a minimum of 4 inches in height. Ponding shall not be in roadways.

8-01.4 Measurement
The third paragraph is revised to read:

Check dams will be measured per linear foot one time only along the completed check dam. No additional measurement will be made for check dams that are required to be rehabilitated or replaced due to wear.

The ninth paragraph is deleted.

The twelfth paragraph (after the preceding amendment is applied) is revised to read:

Seeding, fertilizing, liming, mulching, mowing, and tackifier will be measured by the acre by ground slope measurement or through the use of design data

This section is supplemented with the following:

Outlet Protection will be measured per each initial installation at an outlet location.

8-01.5 Payment

The bid item “Straw Bale”, per each is deleted.

The bid item “___Erosion Control Blanket”, per square yard is deleted.

The bid item “Soil Binder or Tacking Agent”, per acre is deleted.

This section is supplemented with the following:

“Outlet Protection”, per each.
The unit Contract price per each for “Outlet Protection” shall be full payment for all costs incurred to complete the Work.

“Tackifier”, per acre.
The unit Contract price per acre for “Tackifier” shall be full payment for all costs incurred to complete the Work.

“Biodegradable Erosion Control Blanket”, per square yard.
The unit Contract price per square yard for “Biodegradable Erosion Control Blanket” shall be full pay for all costs to complete the specified Work.

Section 8-02, Roadside Restoration
August 6, 2012

In this section, “psiPE” is revised to read “PSIPE”.

8-02.3(4)C Topsoil Type C

In this section, “9-14.1(2)” is revised to read “9-14.1(3)”.

8-02.3(8) Planting

Item number 1 in the second paragraph is revised to read:

1. Non-Irrigated Plant Material
   West of the summit of the Cascade Range - October 1 to March 1.
   East of the summit of the Cascade Range - October 1 to November 15.

8-02.5 Payment

The paragraph following bid item “Coarse Compost”, per cubic yard” is revised to read:

The unit Contract price per cubic yard for “Fine Compost”, Medium Compost” or “Coarse Compost” shall be full pay for furnishing and spreading the compost onto the existing soil.

Section 8-03, Irrigation Systems
April 2, 2012

8-03.3(7) Flushing and Testing

The fifth paragraph is deleted.

Section 8-04, Curbs, Gutters, and Spillways
April 2, 2012

8-04.3(1) Cement Concrete Curbs, Gutters, and Spillways

This section is supplemented with the following new sub-section:

8-04.3(1)B Roundabout Cement Concrete Curb and Gutter

Roundabout cement concrete curb and gutter and roundabout splitter island nosing curb shall be shaped and finished to match the shape of the adjoining curb as shown in the Plans. All other requirements for cement concrete curb and cement concrete curb and gutter shall apply to roundabout cement concrete curb and gutter.

8-04.4 Measurement

This section is supplemented with the following:

Roundabout splitter island nosing curb will be measured per each.

8-04.5 Payment

The bid item, “Roundabout Truck Apron Cement Concrete Curb”, per linear foot is deleted.

This section is supplemented with the following:

“Roundabout Cement Concrete Curb and Gutter”, per linear foot

The unit Contract price per linear foot for “Roundabout Cement Concrete Curb and Gutter” shall be full payment for all costs for the Work including transitioning the roundabout cement concrete curb and gutter to the adjoining curb shape.

“Roundabout Splitter Island Nosing Curb”, per each.

The unit Contract price per each for “Roundabout Splitter Island Nosing Curb” shall be full payment for all costs for the Work including transitioning the roundabout splitter island nosing curb to the adjoining curb shape.

Section 8-07, Precast Traffic Curb and Block Traffic Curb
January 7, 2013

This section's title is revised to read:

8-07 Precast Traffic Curb

8-07.1 Description
This section is revised to read:

This Work consists of furnishing and installing precast traffic sloped mountable curb or dual faced sloped mountable curb of the design and type specified in the Plans in accordance with these Specifications and the Standard Plans in the locations indicated in the Plans or as staked by the Engineer.

8-07.2 Materials
The material reference "Block Traffic Curb 9-18.3" is deleted from this section.

The referenced section for the following item is revised to read:

Paint 9-34.2

8-07.3(1) Installing Curbs
The fifth and seventh paragraphs are deleted from this section.

8-07.4 Measurement
The first paragraph is deleted from this section.

8-07.5 Payment
The following bid items are deleted from this section:

"Type A Precast Traffic Curb", per linear foot.
"Type C Precast Traffic Curb", per linear foot.
"Type A Block Traffic Curb", per linear foot.
"Type C Block Traffic Curb", per linear foot.

Section 8-11, Guardrail
August 6, 2012

8-11.3(1)D Removing Guardrail and Guardrail Anchor
The first two sentences in the first paragraph are revised to read:

Removal of the various types of guardrail shall include removal of the rail, cable elements, hardware, and posts, including transition sections, expansion sections, terminal sections and the rail element of anchor assemblies. Removal of the various types of guardrail anchors shall include removal of the anchor assembly, including concrete bases, rebar, steel tubes, and any other appurtenances in the anchor assembly.

8-11.4 Measurement

The seventh paragraph is revised to read:

Measurement of removal of guardrail will be by the linear foot measured along the line of guardrail removed including transition sections, expansion sections, guardrail anchor rail elements and terminal sections.

8-11.5 Payment

The paragraph following the bid item “Removing Guardrail Anchor”, per each is revised to read:

The unit Contract price per each for “Removing Guardrail Anchor” shall be full payment for all costs to perform the Work as described in Section 8-11.3(1)D, including rail removal, if there isn’t a Bid Item for Removing Guardrail in the run of guardrail connecting to the anchor.

Section 8-12, Chain Link Fence and Wire Fence
April 2, 2012

In this Section “Engineer” is revised to read “Project Engineer”.

8-12.1 Materials

This section is supplemented with the following:

Paint 9-08.1(2)B

8-12.3(1)A Posts

The words “for Type 3 and Type 4 fences” and “on Type 3 and Type 4 fences” are deleted from this section.

The first sentence of the fifth paragraph is revised to read:

After the post is set and plumbed, the hole shall be filled with Grout Type 4.

The third sentence in the sixth paragraph is replaced with the following two sentences:

After the post is set and plumbed, the hole in the portion of the post in solid rock shall be filled with Grout Type 4. The grout shall be thoroughly worked into the hole so as to leave no voids.

The seventh paragraph is deleted.

The ninth paragraph is revised to read:

Steep slopes or abrupt topography may require changes in various elements of the fence. It shall be the responsibility of the Contractor to provide all posts of sufficient length to accommodate the chain link fabric.

The tenth paragraph is revised to read:

All round posts shall have approved top caps fastened securely to the posts. The base of the top cap fitting for round posts shall feature an apron around the outside of the posts.

8-12.3(1)B Top Rail

This section's content including title is deleted and replaced with:

8-12.3(1)B Vacant

8-12.3(1)C Tension Wire and Tension Cable

This section's content including title is revised to read:

8-12.3(1)C Tension Wire

Tension Wires shall be attached to the posts as detailed in the Plans or as approved by the Engineer.

8-12.3(1)D Chain Link Fabric

The first three paragraphs are revised to read:

Chain link fabric shall be attached after the cables and wires have been properly tensioned.

Chain link fabric shall be placed on the face of the post away from the Highway, except on horizontal curves where it shall be placed on the face on the outside of the curve unless otherwise directed by the Project Engineer.

Chain link fabric shall be placed approximately 1-inch above the ground and on a straight grade between posts by excavating high points of ground. Filling of depressions will be permitted only upon approval of the Project Engineer.

The fourth sentence in the fourth paragraph is revised to read:

The top and bottom edge of the fabric shall be fastened with hog rings to the top and bottom tension wires as may be applicable, spaced at 24-inch intervals.

8-12.3(1)E Chain Link Gates

The third paragraph is deleted.

8-12.3(2)A Posts

In the second paragraph, "commercial" is deleted.

The first sentence of the fifth paragraph is revised to read:

After the post is set and plumbed, the hole shall be filled with Grout Type 4.

The fourth sentence in the sixth paragraph is replaced with the following two sentences:

After the post is set and plumbed, the hole in the portion of the post in solid rock shall be filled with Grout Type 4. The grout shall be thoroughly worked into the hole so as to leave no voids.

The tenth paragraph is revised to read:

Where the new fence joins an existing fence, the 2 shall be attached in a manner satisfactory to the Project Engineer, and end or corner posts shall be set as necessary.

The eleventh paragraph is deleted.

8-12.5 Payment

The paragraph following the item "Chain Link Fence Type ____", per linear foot is revised to read:

> The unit Contract price per linear foot for "Chain Link Fence Type ___" shall be full payment for all costs for the specified Work including brace post installation and all other requirements of Section 8-12 for Chain Link Fence, unless covered in a separate Bid Item in this Section.

The following paragraph is inserted after the item "End, Gate, Corner, and Pull Post for Chain Link Fence", per each:

> The unit Contract price per each for "End, Gate, Corner, and Pull Post for Chain Link Fence" shall be full payment for all costs for the specified Work.

The following paragraph is inserted after the item "Single 6 Ft. Chain Link Gate", per each:

> The unit Contract price per each for "Double 14 Ft. Chain Link Gate", "Double 20 Ft. Chain Link Gate", and "Single 6 Ft. Chain Link Gate", shall be full payment for all costs for the specified Work.

The paragraph following the item "Wire Fence Type ____", per linear foot is revised to read

> The unit Contract price per each for "Wire Fence Type ____" shall be full payment for all costs for the specified Work including payment for clearing of the fence line.

The following paragraph is inserted after the item "Double Wire Gate 20 Ft. Wide", per each:

> The unit contract price per each for "Single Wire Gate 14 Ft. Wide" and "Double Wire Gate 20 Ft. Wide" shall be full payment for all costs for the specified Work.

The paragraph following the item "Access Control Gate", per each is revised to read:

> The unit contract price per each for "Access Control Gate" shall be full payment for all costs to perform the specified Work.

Section 8-15, Riprap
April 2, 2012

8-15.1 Description

The second paragraph is revised to read:

> Riprap will be classified as heavy loose riprap, light loose riprap, and hand placed riprap.

Section 8-20, Illumination, Traffic Signal Systems, And Electrical
August 6, 2012

8-20.3(4) Foundations

The first paragraph is revised to read:

Foundation concrete shall conform to the requirements for the specified class, be cast-in-place concrete and be constructed in accordance with Sections 6-02.2 and 6-02.3. Concrete for Type II, III, IV, V, and CCTV signal standards and light standard foundations shall be Class 4000P. Concrete for pedestals and cabinets, Type PPB, PS, I, FB, and RM signal standards and other foundations shall be Class 3000. Concrete placed into an excavation where water is present shall be placed using an approved tremie. If water is not present, the concrete shall be placed such that the free-fall is vertical down the center of the shaft without hitting the sides, the steel reinforcing bars, or the steel reinforcing bar cage bracing. The Section 6-02.3(6) restriction for 5-feet maximum free-fall shall not apply to placement of Class 4000P concrete into a shaft. Steel reinforcing bars for foundations shall conform to Section 9-07.

8-20.3(9) Bonding, Grounding

The first sentence in the second paragraph is replaced with the following two sentences:

All conduit installed shall have an equipment ground conductor installed in addition to the conductors noted in the Contract. Conduit with innerducts shall have an equipment ground conductor installed in each innerduct that has an electrical conductor.

Section 8-21, Permanent Signing
January 7, 2013

8-21.2 Materials

The third sentence is revised to read:

Materials for sign mounting shall conform to Section 9-28.11.

8-21.3(9)A Fabrication of Steel Structures

The first sentence in the first paragraph is revised to read:

Fabrication shall conform to the applicable requirements of Section 6-03 and 9-06.

This section is supplemented with the following:

All fabrication, including repairs, adjustments or modifications of previously fabricated sign structure members and connection elements, shall be performed in the shop, under an Engineer approved shop drawing prepared and submitted by the Contractor for the original fabrication or the specific repair, adjustment or modification. Sign structure fabrication repair, adjustment or modification of any kind in the field is not permitted. If fabrication repair, adjustment or modification occurs after a sign structure member or connection element has been galvanized, the entire member or element shall be re-galvanized in accordance with AASHTO M 111.

8-21.3(9)B Vacant

This section including title is revised to read:

8-21.3(9)B Erection of Steel Structures

Erection shall conform to the applicable requirements of Sections 6-03 and 8-21.3(9)F. Section 8-21.3(9)F notwithstanding, the Contractor may erect a sign bridge prior to completion of the shaft cap portion of one foundation for one post provided the following conditions are satisfied:

1. The Contractor shall submit design calculations and working drawings of the temporary supports and falsework supporting the sign bridge near the location of the incomplete foundation to the Engineer for approval in accordance with Section 6-01.9. The submittal shall include the method of releasing and removing the temporary supports and falsework without inducing loads and stress into the sign bridge.

2. The Contractor shall submit the method used to secure the anchor bolt array in proper position with the sign bridge while casting the shaft cap concrete to complete the foundation.

3. The Contractor shall erect the sign bridge and temporary supports and falsework, complete the remaining portion of the incomplete foundation, and remove the temporary supports and falsework, in accordance with the working drawing submittals as approved by the Engineer.

8-21.3(9)F Foundations

The following new paragraph is inserted after the second paragraph:

Concrete placed into an excavation where water is present shall be placed using an approved tremie. If water is not present, the concrete shall be placed such that the free-fall is vertical down the center of the shaft without hitting the sides, the steel reinforcing bars, or the steel reinforcing bar cage bracing. The Section 6-02.3(6) restriction for 5-feet maximum free-fall shall not apply to placement of Class 4000P concrete into a shaft.

The ninth paragraph (after implementing the preceding Amendment) is replaced with the following three new paragraphs:

After construction of concrete foundations for sign bridge and cantilever sign structures, the Contractor shall survey the foundation locations and elevations, the anchor bolt array locations and lengths of exposed threads. The Contractor shall confirm that the survey conforms to the sign structure post, beam, span and foundation design geometry shown in the Plans, and shall identify any deviations from the design geometry shown in the Plans. When deviations are identified, the Contractor shall notify the Engineer, and such notice shall be accompanied by the Contractor's proposed method(s) of addressing the deviations, including removal and reconstruction of the shaft cap portion of the affected concrete foundation as outlined in this Section, or fabrication repair, adjustment or modification, with associated shop drawings, in accordance with Section 8-21.3(9)A.

If the Contractor's survey indicates that a concrete foundation has been constructed incorrectly for a sign structure that has already been fabricated, the Contractor may remove and reconstruct the shaft cap portion of the foundation, in accordance with Section 1-07.13, provided the following conditions are satisfied:

1. The Contractor shall submit the method and equipment to be used to remove the portion of the concrete foundation to be removed and reconstructed to the Engineer for approval in accordance with Section 1-05.3. The submittal shall include confirmation that the equipment and the method of operation is appropriate to ensure that the existing anchor bolt array and primary shaft vertical steel reinforcing bars will not be damaged.

2. All steel reinforcing bars, except for steel reinforcing bars extending from the bottom portion of the foundation to remain, shall be removed and disposed of in accordance with Sections 2-02.3 and 2-03.3(7)C, and shall be replaced with new steel reinforcing bars conforming to the size, dimensions and geometry shown in the Plans. All concrete of the removed portion of the foundation shall be removed and disposed of in accordance with Sections 2-02.3 and 2-03.3(7)C.

3. The Contractor shall adjust the primary shaft vertical steel reinforcing bars as necessary in accordance with Section 6-02.3(24)C to provide clearance for the anchor bolt array.

Sign structures shall not be erected on concrete foundations until the Contractor confirms that the foundations and the fabricated sign structures are either compatible with each other and the design geometry shown in the Plans, or have been modified in accordance with this Section and as approved by the Engineer to be compatible with each other, and the foundations have attained a compressive strength of 2,400-psi.

Item number 4 in the twelfth paragraph (after implemented the preceding Amendments) is revised to read:

4. Concrete shall be Class 4000P, except as otherwise specified. The concrete for the shaft cap (the portion containing the anchor bolt array assemblies above the construction joint at the top of the shaft) shall be Class 4000.

Item number 3 in the thirteenth paragraph (after implemented the preceding Amendments) is revised to read:

3. Unless otherwise shown in the Plans, concrete shall be Class 4000P.

8-21.5 Payment

This section is supplemented with the following:

All costs in connection with surveying completed concrete foundations for sign bridges and cantilever sign structures shall be included in the lump sum contract price for “Structure Surveying”, except that when no Bid item is included in the Proposal for “Structure Surveying” then such costs shall be included in the lump sum contract price(s) for “Sign Bridge No. ___” and “Cantilever Sign Structure No. ___”.

Section 8-22, Pavement Marking
January 7, 2013

8-22.3(3)D Line Applications

The last paragraph is supplemented with the following:

Grooved line pavement marking shall not be constructed on bridge decks or on bridge approach slabs.

8-22.3(6) Removal of Pavement Markings

The following two new sentences are inserted after the first sentence:

Grinding to remove painted markings is not allowed. Grinding to remove plastic marking is allowed to a depth just above the pavement surface, then water blasting or shot blasting shall be required to remove the remaining markings.

8-22.4 Measurement

The items “Painted Wide Line” and “Plastic Wide Line” are deleted from the fourth paragraph.

The sixth paragraph is revised to read:

Diagonal lines used to delineate parking stalls that are constructed of painted or plastic 4-inch lines will be measured as “Paint Line” or “Plastic Line” by the linear foot of line installed. Crosswalk line will be measured by the square foot of marking installed.

The following two new paragraphs are inserted after the sixth paragraph:

Crosshatch markings used to delineate median and gore areas will be measured by the completed linear foot as “Painted Crosshatch Marking” or “Plastic Crosshatch Marking”.

The measurement for “Painted Crosshatch Marking” and for “Plastic Crosshatch Marking” will be based on the total length of each 8-inch or 12-inch wide line installed.

8-22.5 Payment

The bid items “Painted Wide Line”, per linear foot and “Plastic Wide Line”, per linear foot are deleted from this section.

This section is supplemented with the following two new bid items:

“Painted Crosshatch Marking”, per linear foot.
“Plastic Crosshatch Marking”, per linear foot.

The following new paragraph is inserted after the last bid item in this section:

The unit Contract price for the aforementioned Bid items shall be full payment for all costs to perform the Work as described in Section 8-22.

Section 8-25, Glare Screen
April 9, 2012

In this section, “tension cable” and “cable” are deleted.

8-25.3(3) Posts

The first sentence in the first paragraph is revised to read:

Posts shall be constructed in accordance with the Standard Plans and applicable provisions of Section 8-12.3(1)A.

The last paragraph is revised to read:

All round posts for Type 1 Design B and Type 2 glare screen shall be fitted with a watertight top securely fastened to the post. Line posts shall have tops designed to carry the top tension wire.

8-25.3(5) Tension Cables
This sections content including title is deleted:

8-25.3(6) Fittings, Attachments, and Hardware
This sections content including title is deleted.

Section 8-29, Wire Mesh Slope Protection
January 7, 2013

This section is deleted in its entirety and replaced with the following:

8-29 Wire Mesh Slope Protection

8-29.1 Description
This Work consists of furnishing and installing the anchors and the wire mesh slope protection in accordance with these Specifications and the details shown in the Plans and in conformity with the lines and dimensions shown in the Plans or established by the Engineer.

8-29.2 Materials
Materials shall meet the requirements of Section 9-16.4.

8-29.3 Construction Requirements
8-29.3(1) Submittals
The Contractor shall submit a wire mesh slope protection plan to the Project Engineer a minimum of seven calendar days prior to beginning the work. The wire mesh slope protection plan shall include the following:

1. Plan sheets for anchor layout and installation, and the equipment and process used to confirm the capacity of the constructed anchors including the calibration data for the stressing devices used to proof test the anchors, as completed by an independent testing laboratory within 60 calendar days of the wire mesh slope work.

2. Working drawings for the temporary yoke or load frame to be used for anchor proof testing in accordance with Section 6-01.9.

3. Plans and details for assembling wire mesh and erecting the assembled mesh on the slope.

All costs for the Work required for Submittals shall be included in the unit Bid price detailed in Section 8-29.5.

8-29.3(2) Anchors
The Contractor shall install anchors of the type shown in the Plans and in conformance with the layout shown in the Wire Mesh Protection Plan as described in Section 8-29.3(1). The spacing and number of the anchors and wire ropes as shown in the Plans are approximate only, and upon review of the wire mesh slope protection plan, the Engineer may arrange the spacing to

better hold the wire mesh against the slope. Backfill material shall be thoroughly compacted with a mechanical compactor.

The Contractor shall proof test up to 25 percent of the anchors in vertical pullout to the minimum allowable anchor capacity specified in the Plans. Proof testing of anchors shall be performed against a temporary yoke or load frame. No part of the temporary yoke or load frame shall bear within three feet of the anchor being tested. For vertical pullout proof testing, an anchor is acceptable if it sustains the specified capacity for 10 minutes with no loss of load. Anchors that fail this criterion shall be replaced and retested. If more than three anchors fail, the Contractor shall proof test all anchors.

8-29.3(3) Wire Rope

All wire rope loops shall include a thimble. No wire rope splicing will be allowed.

8-29.3(4) Wire Mesh

The wire mesh shall be fastened to the completed wire rope assembly as shown in the Plans. High tensile steel fasteners on the vertical seams shall be staggered across width of the seam. Horizontal splices joining 2 rolls of mesh shall be made by overlapping the mesh approximately 3 feet and either weaving 3 rows of lacing wires through every mesh opening or using 4 rows of high tensile steel fasteners placed on approximately 3-inch spacing. All top and bottom laps shall be made by folding the mesh to the outside, away from the slope, to avoid the possibility of falling material hanging up in the folds. The bottom of the mesh shall be located as shown in the Plans. The ends of all lacing wires shall be secured to the mesh with a minimum of 1½-turns.

The wire mesh shall not be tensioned in any direction, but is to remain loose so as to increase its dampening effect on rolling rocks. The Contractor shall use care in the handling and installing of the wire mesh and wire rope. Any mesh or wire rope damaged due to the Contractor's operations shall be replaced by the Contractor at no expense to the Contracting Agency.

8-29.4 Measurement

Measurement of anchors will be per each for the completed anchor. Anchor types will not be differentiated.

Wire mesh slope protection will be measured by the square foot of wire mesh erected on the slope. There will be no deduction made for overlapping the wire mesh material as required for splices or for coverage due to variations in the slope or ground conditions.

8-29.5 Payment

Payment will be made in accordance with Section 1-04.1, for each of the following Bid items that are included in the Proposal:

"Wire Mesh Slope Protection Anchor", per each.
The unit Contract price per each for "Wire Mesh Slope Protection Anchor" shall be full payment for all costs for the Work described in Sections 8-29.3(1) and 8-29.3(2).

"Wire Mesh Slope Protection", per square foot
The unit Contract price per square foot for "Wire Mesh Slope Protection" shall be full payment for all costs for the Work described in Section 8-29.3(3) and 8-29.3(4).

Section 9-02, Bituminous Materials
January 7, 2013

9-02.4 Anti-Stripping Additive
This section is revised to read:

Anti-stripping additive shall be a product listed in the current WSDOT Qualified Products List (QPL).

Section 9-03, Aggregates
January 7, 2012

9-03.1(1) General Requirements
The eighth paragraph is deleted.

9-03.13 Backfill for Sand Drains
This section is supplemented with the following:

That portion of backfill retained on a No. 4 sieve shall not contain more than 0.05 percent by weight of wood waste.

9-03.13(1) Sand Drainage Blanket
The last paragraph is revised to read:

That portion of backfill retained on a No. 4 sieve shall not contain more than 0.05 percent by weight of wood waste.

9-03.14(1) Gravel Borrow
Note [1] is deleted, including the reference in the table.

9-03.14(2) Select Borrow
Note [1] is deleted.

Note [2] is re-numbered Note [1], including the reference in the table.

9-03.14(4) Gravel Borrow for Geosynthetic Retaining Wall
This section is revised to read:

All backfill material for geosynthetic retaining walls shall consist of granular material, either naturally occurring or processed, and shall be free draining, free from organic or otherwise deleterious material. The material shall be substantially free of shale or other soft, poor durability particles, and shall not contain recycled materials, such as glass, shredded tires, portland cement concrete rubble, or asphaltic concrete rubble. The backfill material shall meet the following requirements for grading and quality:

Sieve Size	Percent Passing
1 ¼ " [1]	99-100
1"	90-100
No. 4	50-80

No. 40	30 max.
No. 200	7.0 max.
Sand Equivalent	50 min.

All percentages are by weight

Property	Test Method	Allowable Test Value
Los Angeles Wear 500 rev.	AASHTO T 96	35 percent max.
Degradation Factor	WSDOT Test Method 113	15 min.
pH, permanent walls	AASHTO T 289	4.5-9
pH, temporary walls	AASHTO T 289	3-10

Wall backfill material satisfying these grading and property requirements shall be classified as nonaggressive.

9-03.21(1) General Requirements

The first sentence in the first paragraph is revised to read:

Hot Mix Asphalt, Concrete Rubble, Recycled Glass (glass cullet), and Steel Furnace Slag may be used as, or blended uniformly with naturally occurring materials for aggregates.

9-03.21(1)C Vacant

This section including title is revised to read:

9-03.21(1)C Recycled Glass (Glass Cullet)

Glass Cullet shall meet the requirements of AASHTO M 318 with the additional requirement that the glass cullet is limited to the maximum amounts set in Section 9-03.21(1)E for recycled glass. Prior to use the Contractor shall provide certification to the Project Engineer that the recycled glass meets the physical properties and deleterious substances requirements in AASHTO M-318.

9-03.21(1) E Table on Maximum Allowable Percent (By Weight) of Recycled Material

The column heading “Recycled Glass” is revised to read “Recycled Glass (Glass Cullet) in the table.

In the column “Recycled Glass (Glass Cullet)” all amounts are revised to read “20” beginning with the item “Ballast” and continuing down until the last item in the table.

Section 9-04, Joint And Crack Sealing Materials
January 7, 2013

9-04.2 Joint Sealants

This section is supplemented with the following new sub-sections:

9-04.2(3) Polyurethane Sealant

Polyurethane sealant shall conform to ASTM C 920 Type S Grade NS Class 25 Use M.

Polyurethane sealant shall be compatible with the closed cell foam backer rod. When required, compatibility characteristics of sealants in contact with backer rods shall be determined by Test Method ASTM C 1087.

9-04.2(3)A Closed Cell Foam Backer Rod

Closed cell foam backer rod for use with polyurethane sealant shall conform to ASTM C 1330 Type C.

9-04.10 Crack Sealing – Rubberized Asphalt

This section is deleted.

9-04.11 Butyl Rubber and Nitrile Rubber

This sections number is revised to read:

9-04.10

Section 9-05, Drainage Structures, Culverts, and Conduits
January 7, 2013

9-05.0 Acceptance by Manufacturer's Certification

This section including title is revised to read:

9-05.0 Acceptance and Approval of Drainage Structures, and Culverts

The Drainage Structure or Culvert may be selected from the Qualified Products List, or submitted using a Request for Approval of Materials (RAM) in accordance with Section 1-06.

Certain drainage materials may be accepted by the Engineer based on a modified acceptance criteria when materials are selected from the Qualified Products List (QPL). The modified acceptance criteria are defined in the QPL for each material.

9-05.1(6) Corrugated Polyethylene Drain Pipe, Couplings, and Fittings (Up to 10 Inch)

This section is supplemented with the following:

Corrugated polyethylene drain pipe manufacturers shall participate in the National Transportation Product Evaluation Program (NTPEP) work plan for HDPE (High Density Polyethylene) Thermoplastic Pipe and be listed on the NTPEP audit website displaying they are NTPEP compliant.

9-05.1(7) Corrugated Polyethylene Drain Pipe, Couplings, and Fittings (12 Inch Through 60 Inch)

This section is supplemented with the following:

Corrugated polyethylene drain pipe manufacturers shall participate in the National Transportation Product Evaluation Program (NTPEP) work plan for HDPE (High Density Polyethylene) Thermoplastic Pipe and be listed on the NTPEP audit website displaying they are NTPEP compliant.

9-05.2(7) Perforated Corrugated Polyethylene Underdrain Pipe (Up to 10 Inch)

This section is supplemented with the following:

Perforated corrugated polyethylene underdrain pipe manufacturers shall participate in the National Transportation Product Evaluation Program (NTPEP) work plan for HDPE (High Density Polyethylene) Thermoplastic Pipe and be listed on the NTPEP audit website displaying they are NTPEP compliant.

9-05.2(8) Perforated Corrugated Polyethylene Underdrain Pipe (12-Inch Through 60 Inch Diameter Maximum), Couplings, and Fittings

This section is supplemented with the following:

Perforated corrugated polyethylene underdrain pipe manufacturers shall participate in the National Transportation Product Evaluation Program (NTPEP) work plan for HDPE (High Density Polyethylene) Thermoplastic Pipe and be listed on the NTPEP audit website displaying they are NTPEP compliant.

9-05.19 Corrugated Polyethylene Culvert Pipe, Couplings, and Fittings

The word “producer” is revised to read “manufacturer”.

The second paragraph is revised to read:

Joints for corrugated polyethylene culvert pipe shall be made with either a bell/bell or bell and spigot coupling and shall incorporate the use of a gasket conforming to the requirements of ASTM D 1056 Type 2 Class B Grade 3 or ASTM F 477. All gaskets shall be factory installed on the coupling or on the pipe by the qualified manufacturer.

This section is supplemented with the following:

Corrugated polyethylene culvert pipe manufacturers shall participate in the National Transportation Product Evaluation Program (NTPEP) work plan for HDPE (High Density Polyethylene) Thermoplastic Pipe and be listed on the NTPEP audit website displaying they are NTPEP compliant.

9-05.20 Corrugated Polyethylene Storm Sewer Pipe, Couplings, and Fittings

The word “producer” is revised to read “manufacturer”.

The first paragraph is revised to read:

Corrugated polyethylene storm sewer pipe, couplings, and fittings shall meet the requirements of AASHTO M 294 Type S or D. The maximum pipe diameter for corrugated polyethylene storm sewer pipe shall be the diameter for which a manufacturer has submitted. Fittings shall be blow molded, rotational molded, or factory welded.

This section is supplemented with the following:

Corrugated polyethylene culvert pipe manufacturers shall participate in the National Transportation Product Evaluation Program (NTPEP) work plan for HDPE (High Density Polyethylene) Thermoplastic Pipe and be listed on the NTPEP audit website displaying they are NTPEP compliant.

9-05.24 Polypropylene Culvert Pipe, Polypropylene Storm Sewer Pipe, and Polypropylene Sanitary Sewer Pipe

This sections content is deleted and replaced with the following:

All joints for polypropylene pipe shall be made with a bell/bell or bell and spigot coupling and shall conform to ASTM D 3212 using elastomeric gaskets conforming to ASTM F 477. All gaskets shall be factory installed on the pipe in accordance with the producer's recommendations.

Qualification for each producer of polypropylene storm sewer pipe requires joint system conformance to ASTM D 3212 using elastomeric gaskets conforming to ASTM F 477 and a formal quality control plan for each plant proposed for consideration.

A Manufacturer's Certificate of Compliance shall be required and shall accompany the materials delivered to the project. The certificate shall clearly identify production lots for all materials represented. The Contracting Agency may conduct verification tests of pipe stiffness or other properties it deems appropriate.

This section is supplemented with the following new sub-sections:

9-05.24(1) Polypropylene Culvert Pipe and Storm Sewer Pipe

Polypropylene culvert and storm sewer pipe shall conform to the following requirements:

1. For dual wall pipe sizes up to 30 inches: ASTM F2736. .
2. For triple wall pipe sizes from 30 to 60 inches: ASTM F2764.
3. For dual wall profile pipe sizes 36 to 60 inches: AASHTO MP 21, Type S or Type D.
4. Fittings shall be factory welded, injection molded or PVC.

9-05.24(2) Polypropylene Sanitary Sewer Pipe

Polypropylene sanitary sewer pipe shall conform to the following requirements:

1. For pipe sizes up to 30 inches: ASTM F2736.
2. For pipe sizes from 30 to 60 inches: ASTM F2764.
3. Fittings shall be factory welded, injection molded or PVC.

Section 9-06, Structural Steel and Related Materials
April 2, 2012

9-06.5(3) High Strength Bolts

In this section, "AASHTO M 291" is revised to read "ASTM A 563".

Section 9-07, Reinforcing Steel
August 6, 2012

9-07.7 Wire Mesh
The first sentence in the first paragraph is revised to read:

> Wire mesh for concrete reinforcement shall conform to the requirements of AASHTO M 55, Welded Steel Wire Fabric for Concrete Reinforcement or AASHTO M 221, Steel Welded Wire Reinforcement, Deformed for Concrete.

Section 9-10, Piling
April 2, 2012

9-10.4 Steel Pile Tips and Shoes
In the first paragraph "ASTMA A 148 Grade 60-90" is revised to read "ASTMA A 148 Grade 90-60".

Section 9-14, Erosion Control and Roadside Planting
January 7, 2013

9-14.3 Fertilizer
The second sentence in the first paragraph is revised to read:

> It may be separate or in a mixture containing the percentage of total nitrogen, available phosphoric acid, and water-soluble potash or sulfur in the amounts specified.

9-14.4(2) Hydraulically Applied Erosion Control Products (HECPs)
The first sentence in the third paragraph is revised to read:

> All HECPs shall be furnished premixed by the manufacturer with Organic or Synthetic Tackifier as specified in Section 9-14.4(7).

The third and fourth rows in Table 1 is revised to read:

Heavy Metals	EPA 6020A Total Metals	Antimony – < 4 mg/kg Arsenic – < 6 mg/kg Barium – < 80 mg/kg Boron – < 160 mg/kg Cadmium – < 2 mg/kg Total Chromium – < 4 mg/kg Copper – < 10 mg/kg Lead – < 5 mg/kg Mercury – < 2 mg/kg Nickel – < 2 mg/kg Selenium – < 10 mg/kg Strontium – < 30 mg/kg Zinc – < 30 mg/kg
Water Holding Capacity	ASTM D 7367	800 percent minimum

9-14.4(2)A Long Term Mulch
In the first paragraph, the phrase "within 2 hours of application" is deleted.

9-14.4(4) Wood Strand Mulch

The last sentence in the second paragraph is deleted.

This section is supplemented with the following new paragraph:

The Contractor shall provide Material Safety Data Sheet (MSDS) that demonstrates that the product is not harmful to plant life and a test report performed in accordance with WSDOT Test Method 125 demonstrating compliance to this specification prior to acceptance.

9-14.4(8) Compost

The second paragraph is revised to read:

Compost production and quality shall comply with WAC 173-350 and for biosolids composts, WAC 173-308.

The third paragraph is to read:

Compost products shall meet the following physical criteria:

1. Compost material shall be tested in accordance with U.S. Composting Council Testing Methods for the Examination of Compost and Composting (TMECC) 02.02-B, “Sample Sieving for Aggregate Size Classification”.

Fine compost shall meet the following gradation:

Sieve Size	Percent Passing	
	Minimum	Maximum
1”	100	
5/8”	90	100
¼”	75	100

Note Maximum particle length of 4 inches.

Medium compost shall meet the following gradation:

Sieve Size	Percent Passing	
	Minimum	Maximum
1”	100	
5/8”	85	100
¼”	70	85

Note Maximum particle length of 4 inches. Medium compost shall have a carbon to nitrogen ration (C:N) between 18:1 and 35:1. The carbon to nitrogen ration shall be calculated using dry weight of “Organic Carbon” using TMECC 04.01A divided by the dry weight of “Total N” using TMECC 04.02D.

Coarse compost shall meet the following gradation:

Sieve Size	Percent Passing	
	Minimum	Maximum

2”	100	
1”	90	100
¾”	70	100
¼”	40	60

Note Maximum particle length of 6 inches. Coarse compost shall have a carbon to nitrogen ratio (C:N) between 25:1 and 35:1. The carbon to nitrogen ratio shall be calculated using the dry weight of “Organic Carbon” using TMECC 04.01A divided by the dry weight of “Total N” using TMECC 04.02D.

2. The pH shall be between 6.0 and 8.5 when tested in accordance with U.S. Composting Council TMECC 04.11-A, “1:5 Slurry pH”.

3. Manufactured inert material (plastic, concrete, ceramics, metal, etc.) shall be less than 1 percent by weight as determined by U.S. Composting Council TMECC 03.08-A “Classification of Inerts by Sieve Size”.

4. Minimum organic matter shall be 40 percent by dry weight basis as determined by U.S. Composting Council TMECC 05.07A “Loss-On-Ignition Organic Matter Method (LOI)”.

5. Soluble salt contents shall be less than 4.0 mmhos/cm when tested in accordance with U.S. Composting Council TMECC 04.10 “Electrical Conductivity.”

6. Maturity shall be greater than 80 percent in accordance with U.S. Composting Council TMECC 05.05-A, “Germination and Root Elongation”.

7. Stability shall be 7-mg CO2–C/g OM/day or below in accordance with U.S. Composting Council TMECC 05.08-B “Carbon Dioxide Evolution Rate”.

8. The compost product shall originate from organic waste as defined in WAC 173 350 as “Type 1 Feedstocks”, “Type 2 Feedstocks”, and/or “Type 3 Feedstocks”. The Contractor shall provide a list of feedstock sources by percentage in the final compost product.

9. The Engineer may also evaluate compost for maturity using U.S. Composting Council TMECC 05.08-E “Solvita® Maturity Index”. Fine compost shall score a number 6 or above on the Solvita® Compost Maturity Test. Medium and coarse compost shall score a 5 or above on the Solvita® Compost Maturity Test.

9-14.4(8)A Compost Approval

This section’s title is revised to read:

9-14.4(8)A Compost Submittal Requirements

The first sentence in this section up until the colon is revised to read:

The Contractor shall submit the following information to the Engineer for approval:

Item No. 2 in the first paragraph is revised to read:

2. A copy of the Solid Waste Handling Permit issued to the manufacturer by the Jurisdictional Health Department in accordance with WAC 173-350 (Minimum Functional Standards for Solid Waste Handling) or for biosolid composts a copy of the Coverage Under the General Permit for Biosolids Management issued to the manufacturer by the Department of Ecology in accordance with WAC 173-308 (Biosolids Management).

9-14.5(1) Polyacrylamide (PAM)

The third sentence is replaced with the following two new sentences:

The minimum average molecular weight shall be greater than 5-mg/mole. The charge density shall be no less than 15 percent and no greater than 30 percent.

9-14.5(2) Erosion Control Blanket

This section including title is deleted in its entirety and replaced with the following:

9-14.5(2) Biodegradable Erosion Control Blanket

Biodegradable erosion control blankets shall be made of natural plant fibers, and all netting material, if present, shall biodegrade within a life span not to exceed 2 years.

The Contractor shall provide independent test results from the National Transportation Product Evaluation Program (NTPEP) meeting the requirements of Section 9-14.5(2)B, 9-14.5(2)C and 9-14.5(2)D.

9-14.5(2)A Approval and Acceptance of Biodegradable Erosion Control Blankets

The erosion control blanket may be selected from the Qualified Products List, or submitted using a Request for Approval of Materials (RAM) in accordance with Section 1-06. Erosion control blankets may be accepted by the Engineer based on the modified acceptance criteria when materials are selected from the QPL. The modified acceptance criteria are defined in the QPL for each material.

9-14.5(2)B Biodegradable Erosion Control Blanket for Slopes Steeper than 3:1 (H:V)

Table 6

Properties	ASTM Test Method	Requirements for Slopes Steeper than 3:1
Protecting Slopes from Rainfall-Induced Erosion	ASTM D 6459 Soil tested shall be sandy loam as defined by the NRCS** Soil Texture Triangle	C factor = 0.04 maximum for cumulative R-Factor<231
Mass Per Unit Area	ASTM D 6475	7.6 oz./sq. yd. minimum
Light Penetration	ASTM D 6567	44 % maximum
Tensile Strength	ASTM D 6818	10.0 x 6.0 pounds/inch minimum

MD x XD*		
Tensile Elongation MD x XD*	ASTM D 6818	38% x 33% maximum
*MD is Machine Design and XD is Cross Direction **Natural Resource Conservation Services		

9-14.5(2)C Biodegradable Erosion Control Blanket for Slopes Flatter than 3:1(H:V)

Table 7

Properties	ASTM Test Method	Slope Flatter than 3:1 Requirements
Protecting Slopes from Rainfall-Induced Erosion	ASTM D 6459 Soil tested shall be sandy loam as defined by the NRCS** Soil Texture Triangle	C factor = 0.15 maximum for cumulative R-Factor<231
Mass Per Unit Area	ASTM D 6475	7.6 oz./sq. yd. minimum
Light Penetration	ASTM D 6567	40% maximum
Tensile Strength MD x XD*	ASTM D 6818	6.5 x 2.3 pounds/inch minimum
Tensile Elongation MD x XD*	ASTM D 6818	38% x 33% maximum
*MD is Machine Design and XD is Cross Direction **Natural Resource Conservation Services		

9-14.5(2)D Biodegradable Erosion Control Blanket for Ditches

Table 8

Properties	Test Method	Requirements
Performance in Protecting	ASTM D 6460 Soil tested shall be sandy	Limiting Shear (T_{Limit}) = 2.0 psf minimum.

Earthen Channels from Stormwater-Induced Erosion	loam as defined by the NRCS** Soil Texture Triangle	Limiting Velocity (V_{Limit}) = 7.5 ft/sec flow minimum.
Mass per Unit Area	ASTM D 6475	7.4 oz./ sq. yd. minimum
Light Penetration	ASTM D 6567	65 % maximum
Tensile Strength MD x XD*	ASTM D 6818	9.6 x 3.2 lbs/inch minimum
Tensile Elongation MD x XD*	ASTM D 6818	38% x 33% maximum
*MD is Machine Design and XD is Cross Direction **Natural Resource Conservation Services		

9-14.5(4) Geotextile Encased Check Dam

This section including title is revised to read:

9-14.5(4) Check Dams

All materials used for check dams shall be non-toxic and not pose a threat to wildlife when installed.

This section is supplemented with the following new sub-sections:

9-14.5(4)A Biodegradable Check Dams

Biodegradable check dams shall meet the following requirements:

Biodegradable Check Dams	Materials
Wattle Check Dam	9-14.5(5)
Compost Sock Check Dam	9-14.5(6)
Coir Log Check Dam	9-14.5(7)

The Contractor may substitute a different biodegradable check dam as long as it complies with the following and is approved by the Engineer:

1. Made of natural plant fiber.

2. Netting if present shall be biodegradable.

9-14.5(4)B Non-biodegradable Check Dams

Non-biodegradable check dams shall meet the following requirements:

1. Geotextile materials shall conform to section 9-33 for silt fence.

2. Other such devices that fulfill the requirements of section 9-14.5(4) and shall be approved by the Engineer prior to installation.

9-14.6(1) Description
In item No. C in the fourth paragraph, “22-inch” is revised to read “2-inch”.

Section 9-16, Fence and Guardrail
January 7, 2013

9-16.1(1)A Post Material for Chain Link Fence
The first paragraph is revised to read:

Except as noted otherwise, post material shall conform to the requirements of AASHTO M 181, Type 1 (zinc-coated steel), Grade 1 or 2, and shall include all round and roll-formed material (line posts, brace posts, end posts, corner posts, and pull posts).

The last sentence in the fourth paragraph is deleted.

9-16.1(1)C Tension Wire and Tension Cable
This section including title is revised to read:

9-16.1(1)C Tension Wire
Tension wire shall meet the requirements of AASHTO M 181. Tension wire galvanizing shall be Class 1.

9-16.1(1)D Fittings and Hardware
The second sentence in the first paragraph is deleted.

The last paragraph is deleted.

9-16.1(2) Approval
This section is deleted.

9-16.2(2) Approval
This section is deleted.

9-16.4(2) Wire Mesh
This section is revised to read:

The galvanized wire mesh shall be a Style 1 double-twisted hexagonal mesh conforming to ASTM A 975 with 8 by 10 opening, except when a colorized, polyvinyl chloride coating is required then the Style shall be a Style 3.

The longitudinal edges of the wire mesh fabric shall have knuckled selvedges with continuous selvedge wire as specified in ASTM A 975.

9-16.4(3) Wire Rope
This section is revised to read:

Wire rope shall be ¾- inch-diameter, independent wire rope class (IWRC) 6x19, extra improved plow steel (EIP) wire rope galvanized in accordance with ASTM A1023. Each lot of wire rope shall be accompanied by a Manufacturer's Certificate of Compliance, a mill certificate, and a test report showing the wire rope meets the minimum breaking force requirements of ASTM A 1023.

9-16.4(4) Hardware

This section is revised to read:

Weldless steel rings shall be drop-forged steel and heat treated after forging; have a single pull, working load limit of at least 10,000 lbs; and meet performance requirements of Federal Specification RR-C-271D Type VI.

Thimbles required for all wire rope loops shall be standard weight, galvanized, and meet performance requirements of Federal Specification FF-T-276b Type II.

Wire rope clips shall have drop-forged steel bases, be galvanized, and meet performance requirements of Federal Specification FF-C-450 Type I Class 1.

9-16.4(5) Hog Rings and Tie Wire

This section including title is revised to read:

9-16.4(5) Fasteners and Lacing Wire

Fasteners shall consist of 11 gauge high tensile steel. Lacing wire shall consist of 9 gauge, zinc-coated steel wire conforming to ASTM A 641.

9-16.4(6) Grout

This section include title is deleted.

9-16.4(7) Anchor

This section including title and section number is revised to read:

9-16.4(6) Ground Anchors

Threaded bar ground anchors shall be deformed, continuously threaded, steel reinforcement bars conforming to either Section 9-07.2 or Section 9-07.11. Threaded bar ground anchors shall be either epoxy-coated in accordance with Sections 6-02.3(24)H and 9-07.3 or galvanized after fabrication in accordance with ASTM A 767 Class I.

Hollow-core anchor bars shall have continuous threads/deformations and be fabricated from steel tubing conforming to ASTM A 519. Couplers and nuts shall provide 100% of the guaranteed minimum tensile strength of the hollow core anchor bars.

Bearing plates shall conform to ASTM A 572 Grade 50 and shall be galvanized after fabrication in accordance with AASHTO M 111. Nuts shall conform to either AASHTO M 291 Grade B, hexagonal, or Section 9-07.11. Nuts shall be galvanized after fabrication in accordance with AASHTO M 111 for plate washers and AASHTO M 232 for all other hardware.

Grout for ground anchors shall be Grout Type 2 for Nonshrink Applications, conforming to Section 9-20.3(2).

Concrete for soil anchor deadmen shall be either commercial concrete conforming to 8 Section 6-02.3(2)B or Class 3000 conforming to Section 6-02.

Steel reinforcing bars for soil anchor deadmen shall conform to Section 9-07.2, and shall be epoxy-coated in accordance with Sections 6-02.3(24)H and 9-07.3.

9-16.6(3) Posts

This section is revised to read:

Line posts for Types 1 and 2 glare screens shall be 2 inch inside diameter galvanized steel pipe with a nominal weight of 3.65 pounds per linear foot. End, corner, brace, and pull posts for Type 1 Design A and B and Type 2 shall be 2 ½ inch inside diameter galvanized steel pipe with a nominal weight of 5.79 pounds per linear foot. Intermediate pull posts (braced line posts) shall be as specified for line posts.

The base material for the manufacture of steel pipes used for posts shall conform to the requirements of ASTM A 53, except the weight tolerance on tubular posts shall be applied as provided below.

Posts provided for glare screen will have an acceptance tolerance on the weight per linear foot, as specified, equal to plus or minus 5 percent. This tolerance will apply to each individual post.

All posts shall be galvanized in accordance with AASHTO M 181 Section 32. The minimum average zinc coating is per square foot of surface area. This area is defined as the total area inside and outside. A sample for computing the average of mass of coating is defined as a 12-inch piece cut from each end of the galvanized member.

9-16.6(5) Cable

This section including title is revised to read:

9-16.6(5) Vacant

9-16.6(6) Cable and Tension Wire Attachments

This section including title is revised to read:

9-16.6(6) Tension Wire Attachments

All tension wire attachments shall be galvanized steel conforming to the requirements of AASHTO M 232 unless otherwise specified. Eye bolts shall have either a shoulder or a back-up nut on the eye end and be provided with an eye nut where needed or standard hex nut and lock washer ⅜-inch diameter for tension wire and of sufficient length to fasten to the type of posts used. Turnbuckles shall be of the shackle end type, ½ inch diameter, with standard take-up of 6 inches and provided with ⅜ inch diameter pins.

9-16.6(9) Fabric Bands and Stretcher Bars

The first paragraph is revised to read:

Fabric bands shall be⅛ inch by 1inch nominal. Stretcher bars shall be $^{3}/_{16}$ inch by ¾ inch nominal or $^{5}/_{16}$ inch diameter round bar nominal. A $^{5}/_{16}$ inch diameter round stretcher bar shall be used with Type 1. Nominal shall be construed to be the area of the cross section of the shape obtained by multiplying the specified width by thickness. A variation of minus 5-percent

from this theoretical area shall be construed as “nominal” size. All shall be galvanized to meet the requirements of ASTM F 626.

Section 9-18, Precast Traffic Curb and Block Traffic Curb
August 6, 2012

This section’s title is revised to read:

9-18 Precast Traffic Curb

9-18.3 Block Traffic Curb
This section including title is revised to read:

9-18.3 Vacant

Section 9-20, Concrete Patching Material, Grout, and Mortar
January 2, 2012

9-20.3(3) Grout Type 3 for Unconfined Bearing Pad Applications
This section is revised to read:

Grout Type 3 shall be a prepackaged material meeting the requirements of ASTM C 928 – Table 1, R2 Concrete or Mortar.

9-20.3(4) Grout Type 4 for Multipurpose Applications
In the third sentence of the first paragraph, the reference “0.40” is revised to read “0.45”.

Section 9-23, Concrete Curing Materials and Admixtures
April 2, 2012

9-23.2 Liquid Membrane-Forming Concrete Curing Compounds
In the first paragraph, “moisture loss” is revised to read “water retention”.

Section 9-28, Signing Materials and Fabrication
September 17, 2012

9-28.14(2) Steel Structures and Posts
“AASHTO M 291” is revised to read “ASTM A 563”.

Section 9-29, Illumination, Signal, Electrical
January 7, 2013

9-29.1(4) Non-Metallic Conduit
This section is supplemented with the following new sub-section:

9-29.1(4)D Deflection Fittings
Deflection Fittings for use with rigid PVC conduit shall be as described in 9-29.1(2)A

9-29.2 Junction Boxes, Cable Vaults, and Pull Boxes

The section is supplemented with the following:

The Contractor shall perform quality control inspection. The Contracting Agency intends to perform Quality Assurance Inspection. By its inspection, the Contracting Agency intends only to verify the quality of that Work. This inspection shall not relieve the Contractor of any responsibility for identifying and replacing defective material and workmanship. Prior to the start of production of the precast concrete units, the Contractor shall advise the Engineer of the production schedule. The Contractor shall give the Inspector safe and free access to the Work. If the Inspector observes any nonspecification Work or unacceptable quality control practices, the Inspector will advise the plant manager. If the corrective action is not acceptable to the Engineer, the unit(s) will be rejected.

9-29.2(1) Standard Duty and Heavy-Duty Junction Boxes

The third paragraph is deleted and replaced with the following new paragraphs:

The Contractor shall provide shop drawings for all components, including the concrete box, and Lid and the shop drawings shall show placement of reinforcing steel. The shop drawing shall be prepared by (or under the direct supervision of) a Professional Engineer, licensed under Title 18 RCW, State of Washington, in the branch of Civil or Structural, and each sheet shall carry the following:

1. Professional Engineer's original signature, date of signature, original seal, registration number, and date of expiration.

2. The initials and dates of all participating design professionals

3. Clear notation of all revisions including identification of who authorized the revision, who made the revision, and the date of the revision.

4. Design calculations shall carry on the cover page, the Professional Engineer's original signature, date of signature, original seal, registration number, and date of expiration.

For each type of junction box, or whenever there is a change to the junction box design, a proof test, as defined in this Specification, shall be performed and new shop drawings submitted.

9-29.2(1)A Standard Duty Junction Boxes

The sub-paragraph's titled "**Concrete Junction Boxes"** are revised to read:

Concrete Junction Boxes

The Standard Duty Concrete Junction Box steel frame, lid support, and lid shall be painted with a black paint containing rust inhibiters or painted with a shop applied, inorganic zinc primer in accordance with Section 6-07.3, or hot-dip galvanized in accordance with AASHTO M 111. All Standard Duty Concrete Junction Boxes placed in sidewalks, walkways, and shared-use paths shall have slip-resistant surfaces. Slip-reisistant lids and frames shall be hot dip galvanized.

Concrete used in Standard Duty Junction Boxes shall have a minimum compressive strength of 6,000 psi when reinforced with a welded wire hoop, or 4,000 psi when reinforced with welded wire fabric or fiber reinforcement. The frame shall be anchored to the box by welding headed studs ⅜ by 3 inches long, as specified in Section 9-06.15, to the frame. The wire fabric shall be

attached to the studs and frame with standard tie practices. The box shall contain ten studs located near the centerline of the frame and box wall. The studs shall be placed one anchor in each corner, one at the middle of each width and two equally spaced on each length of the box.

Materials for Type 1, 2, and 8 Concrete Junction Boxes shall conform to the following:

Materials	Requirement
Concrete	Section 6-02
Reinforcing Steel	Section 9-07
Fiber Reinforcing	ASTM C 1116, Type III
Lid	ASTM A 786 diamond plate steel
Slip Resistant Lid	ASTM A 36 steel
Frame	ASTM A 786 diamond plate steel or ASTM A36 steel
Slip Resistant Frame	ASTM A 36 steel
Lid Support	ASTM A 36, or ASTM A1011 Grade SS
Handle & Handle support	ASTM A 36 steel or ASTM A1011 Grade CS or SS
Anchors (studs)	Section 9-06.15
Bolts, Studs, Nuts, Washers	ASTM F 593 or A 193, Type 304 or 316, or Stainless Steel grade 302, 304, or 316 steel in accordance with approved shop drawing
Locking and Latching Mechanism Hardware and Bolts	In accordance with approved shop drawings

9-29.2(1)B Heavy Duty Junction Boxes

The section is revised to read:

Heavy-Duty Junction Boxes shall be concrete and have a minimum vertical load rating of 46,000 pounds without permanent deformation and 60,000 pounds without failure when tested in accordance with Section 9-29.2(1)C.

The Heavy-Duty Junction Box steel frame, lid support and lid shall be painted with a shop applied, inorganic zinc primer in accordance with Section 6-07.3.

Materials for Type 4, 5, and 6 Concrete Junction Boxes shall conform to the following:

Materials	Requirement
Concrete	Section 6-02
Reinforcing Steel	Section 9-07
Lid	ASTM A 786 diamond plate steel, rolled from plate complying with ASTM A 572, grade 50 or ASTM A 588, and having a min. CVN toughness of 20 ft-lb at 40 degrees F.

Frame and stiffener plates	ASTM A 572 grade 50 or ASTM A 588, both with min. CVN toughness of 20 ft-lb at 40 degrees F
Handle	ASTM A 36 steel or ASTM A 1011 Grade CS or SS
Anchors (studs)	Section 9-06.15
Bolts, Studs, Nuts, Washers	ASTM F 593 or A 193, Type 304 or 316, or Stainless steel grade 302, 304, or 316 in accordance with approved shop drawing
Hinges and Locking and Latching Mechanism Hardware and Bolts	In accordance with approved shop drawings

The lid stiffener plates shall bear on the frame, and be milled so that there is full even contact, around the perimeter, between the bearing seat and lid stiffener plates, after fabrication of the frame and lid. The bearing seat and lid perimeter bar shall be free from burrs, dirt, and other foreign debris that would prevent solid seating. Bolts and nuts shall be liberally coated with anti-seize compound. Bolts shall be installed snug tight. The bearing seat and lid perimeter bar shall be machined to allow a minimum of 75 percent of the bearing areas to be seated with a tolerance of 0.0 to 0.005 inches measured with a feeler gage. The bearing area percentage will be measured for each side of the lid as it bears on the frame.

9-29.2(1)C Testing Requirements

The first paragraph is revised to read:

The Contractor shall provide for testing of junction boxes, cable vaults and pull boxes. Junction boxes, cable vaults and pull boxes shall be tested by an independent materials testing facility, and a test report issued documenting the results of the tests performed.

The second paragraph is revised to read:

For concrete junction boxes, vaults and pull boxes, the independent testing laboratory shall meet the requirements of AASHTO R 18 for Qualified Tester and Verified Test Equipment. The test shall be conducted in the presence of a Professional Engineer, licensed under Title 18 RCW, State of Washington, in the branch of Civil or Structural, and each test sheet shall have the Professional Engineer's original signature, date of signature, original seal, registration number, and date of expiration. One copy of the test report shall be furnished to the Contracting Agency certifying that the box and cover meet or exceed the loading requirements for a concrete junction box, and shall include the following information:

1. Product identification.
2. Date of testing.
3. Description of testing apparatus and procedure.
4. All load deflection and failure data.
5. Weight of box and cover tested.

6. Upon completion of the required test(s) the box shall be loaded to failure.

7. A brief description of type and location of failure.

The third paragraph is revised to read:

For non-concrete junction boxes the independent testing laboratory shall meet the requirements of AASHTO R 18 for Qualified Tester and Verified Test Equipment. The test shall be conducted in the presence of a Professional Engineer, licensed under Title 18 RCW, State of Washington, in the branch of Civil or Structural, and each test sheet shall have the Professional Engineer's original signature, date of signature, original seal, registration number, and date of expiration. One copy of the test report shall be furnished to the Contracting Agency certifying that the box and cover meet or exceed the loading requirements for a non-concrete junction box, and shall include the following information:

1. Product identification.

2. Date of testing.

3. Description of testing apparatus and procedure.

4. All load deflection data.

5. Weight of box and cover tested.

The first paragraph following the title "**Testing for the Standard Duty Non-Concrete Junction Boxes"** is revised to read:

Non-concrete Junction Boxes shall be tested as defined in the ANSI/SCTE 77-2007 Tier 15 test method with test load minimum of 22,500 lbs. In addition, the Contractor shall provide a Manufacture Certificate of Compliance for each non-concrete junction box installed.

9-29.2(2) Standard Duty and Heavy-Duty Cable Vaults and Pull Boxes

This section is revised to read:

Standard Duty and Heavy-Duty Cable Vaults and Pull Boxes shall be constructed as a concrete box and as a concrete lid. The lid for the Heavy-Duty and Standard Duty Cable Vaults and Pull Boxes shall be interchangeable and both shall fit the same box as shown in the Standard Plans.

The Contractor shall provide shop drawings for all components, including concrete box, Cast Iron Ring, Ductile Iron Lid, Steel Rings, and Lid. In addition, the shop drawings shall show placement of reinforcing steel, knock outs, and any other appurtenances. The shop drawing shall be prepared by or under the direct supervision of a Professional Engineer, licensed under Title 18 RCW, State of Washington, in the branch of Civil or Structural, and each sheet shall carry the following:

1. Professional Engineer's original signature, date of signature, original seal, registration number, and date of expiration.

2. The initials and dates of all participating design professionals

3. Clear notation of all revisions including identification of who authorized the revision, who made the revision, and the date of the revision.

4. Design calculations shall carry on the cover page, the Professional Engineer's original signature, date of signature, original seal, registration number, and date of expiration.

For each type of box or whenever there is a change to the Cable Vault or Pull box design, a proof test, as defined in this Specification, shall be performed and new shop drawings submitted.

9-29.2(2)A Standard Duty Cable Vaults and Pull Boxes

This section is revised to read:

Standard Duty Cable Vaults and Pull boxes shall be concrete and have a minimum load rating of 22,500 pounds and be tested in accordance with Section 9-29.2(1)C for concrete Standard Duty Junction Boxes.

Concrete for standard duty cable vaults and pull boxes shall have a minimum compressive strength of 4,000 psi. The lid frame shall be anchored to the vault/box concrete lid by welding headed studs ⅜ by 3 inches long, as specified in Section 9-06.15, to the frame. The wire fabric shall be attached to the studs and frame with standard tie practices. The vault/box concrete lid shall contain ten studs located near the centerline of the frame and wall. Studs shall be placed one anchor in each corner, one at the middle of each width and two equally spaced on each length of the vault/box. The steel frame, lid support, and lid shall be painted with a black paint containing rust inhibiters or painted with a shop applied, inorganic zinc primer in accordance with Section 6-07.3 or hot-dip galvanized in accordance with ASTM M 111.

All Standard Duty Cable Vaults and Pull Boxes placed in sidewalks, walkways, and shared-use paths shall have slip-resistant surfaces. The steel frame, lid support, and lid for the Standard Duty Cable Vaults and Pull Boxes shall be hot-dip galvanized.

Materials for Standard Duty Cable Vaults and Pull Boxes shall conform to the following:

Materials	Requirements
Concrete	Section 6-02
Reinforcing Steel	Section 9-07
Lid	ASTM A 786 diamond plate steel
Slip Resistant Lid	ASTM A 36 Steel
Frame	ASTM A 786 diamond plate steel or ASTM A 36
Slip Resistant Frame	ASTM A 36 Steel
Lid Support	ASTM A 36 Steel, or ASTM A 1011 Grade SS
Handle & Handle Support	ASTM A 36 steel or ASTM A 1011 Grade CS or SS
Anchors (studs)	Section 9-06.15
Bolts, Studs, Nuts,	ASTM F593 or A 193, type 304 or 316, or

Washers	Stainless steel grade 302, 304, 316 per approved shop drawing
Hinges and Locking Mechanism Hardware and Bolts	Per approved shop drawings

9-29.2(2)B Heavy-Duty Cable Vaults and Pull Boxes

This section is revised to read:

Heavy-Duty Cable Vaults and Pull Boxes shall be constructed of concrete having a minimum compressive strength of 4,000 psi, and have a minimum vertical load rating of 46,000 pounds without permanent deformation and 60,000 pounds without failure when tested in accordance with Section 9-29.2(1)C for Heavy-Duty Junction Boxes.

Materials for Heavy Duty Cable Vaults and Pull boxes shall conform to the following:

Materials	Requirements
Concrete	Section 6-02
Reinforcing Steel	Section 9-07
Cover	Section 9-05.15(1)
Ring	Section 9-05.15(1)
Anchors (studs)	Section 9-06.15
Bolts, Nuts, Washers	ASTM F 593 or A 193, Type 304 or 316, or Stainless steel grade 302, 304, 316 in accordance with approved shop drawing

9-29.6(2) Slip Base Hardware

"AASHTO M 291" is revised to read "ASTM A 563".

9-29.6(5) Foundation Hardware

"AASHTO M 291" is revised to read "ASTM A 563".

9-29.10 Luminaires

The third paragraph is revised to read:

All luminaires shall be provided with markers for positive identification of light source type and wattage. Markers shall be 3 inches square with Gothic bold, black 2-inch legend on colored background. Background color shall be gold for high-pressure sodium and red for metal halide, and white for induction light sources. Legends shall be sealed with transparent film resistant to dust, weather, and ultraviolet exposure.

9-29.10(2) Decorative Luminaries

The second sentence in the third paragraph is deleted.

9-29.13(7)B Auxiliary Equipment for NEMA Controllers

In the first paragraph, item number's 8-13 are renumbered to read 9-14 respectfully.

Item number 7 in the first paragraph is revised to read:

7. A “Display Panel” when noted in the Contract. The display panel shall depict a generic eight-phase operation. The panel shall be mounted on the inside of the front cabinet door and the mounting shall be of a design that allows positioning of the panel in four orientations 90 degrees from each other. The mounting shall be removable without use of any tools. Incandescent red, yellow, green, walk and don’t walk indicator lights shall be provided for each phase. The indicator lights shall be connected to the associated field terminals. The connecting cable shall be long enough to allow for any mounting orientation. No diodes will be allowed in the display panel. A means of disconnecting all wiring entering the panel shall be provided. Switches shall be provided on the panel with labels and functions as follows:

    a. Display On – Signal indicator lamps will display the operation of the intersection.

    b. Test – All indicator lamps shall be energized.

    c. Display Off – All signal indicator lamps shall be de-energized.

The following new numbered item is inserted after item number 7.:

8. A “Detector Panel”, as specified in Section 9-29.13(7)D, shall be installed. The panel shall be mounted on the inside of the front cabinet door. The detector panel shall be constructed as a single unit. Detector switches with separate operate, test, and off positions shall be provided for each field detector input circuit. A high intensity light emitting diode (LED) shall be provided for each switch. The lamp shall energize upon vehicle, pedestrian or test switch actuation. The test switch shall provide a spring loaded momentary contact that will place a call into the controller. When in the OFF position, respective detector circuits will be disconnected. In the operate position, each respective detector circuit shall operate normally. Switches shall be provided on the panel with labels and functions as follows:

    a. Display On – Detector indicator lights shall operate consistent with their respective switches.

    b. Display Off – detector indicator lights shall be de-energized.

    A means of disconnecting all wiring entering the panel shall be provided. The disconnect shall include a means to jumper detection calls when the display panel is disconnected. All switches on the panel shall be marked with its associated Plan detector number. All markers shall be permanent.

9-29.13(7)E Type 170E, 170E-HC-11, 2070, 2070 Lite, ATC Controller Cabinets

The following new title is inserted after the fifth sentence in the first paragraph:

9-29.13(7)F Ramp Meter, Traffic Data, and Warning Sign Cabinets

9-29.16(1)A1 Conventional Optical System

This section’s title is revised to read:

9-29.16(1)A1 Non-LED Optical System

9-29.16(1)D1 Electrical - Conventional

This section's title is revised to read:

9-29.16(1)D1 Electrical – Non-LED

9-29.20 Pedestrian Signals

This section is revised to read:

Pedestrian signals shall be Light Emitting Diods (LED) type.

The LED pedestrian signal module shall be operationally compatible with controllers and conflict monitors. The LED lamp unit shall contain a disconnect that will show an open switch to the conflict monitor when less than 60 percent of the LEDs in the unit are operational.

The Pedestrian signal heads shall be on the QPL or the Contractor shall submit a Manufacturer's Certificate of Compliance, in accordance with Standard Specification 1-06.3, with each type of signal head. The certificate shall state that the lot of pedestrian signal heads meet the following requirements:

1. All pedestrian signal heads shall be a Walk/Don't Walk module with a countdown display.

2. All pedestrian displays shall comply with the MUTCD and ITE publication ST 011B, VTCSH2 or current ITE Specification and shall have an incandescent appearance. The Contractor shall provide test results from a Nationally Recognized Testing Laboratory documenting that the LED display conforms to the current ITE and the following requirements:

   a. All pedestrian signals supplied to any one project shall be from the same manufacturer and type but need not be from the same manufacturer as the vehicle heads.

   b. Each pedestrian signal face shall be a single unit housing with the signal indication size, a nominal 16 inch x 18 inch with side by side symbol messages with countdown display.

   c Housings shall be green polycarbonate or die-cast aluminum and the aluminum housings shall be painted with two coats of factory applied traffic signal green enamel (Federal Standard 595-14056). All hinges and latches and interior hardware shall be stainless steel.

3. Optical units for traffic signal displays shall conform to the following:

   a. Pedestrian "RAISED HAND" and "WALKING PERSON" modules shall be the countdown display type showing the time remaining in the pedestrian change interval. When the pedestrian change interval is reduced due to a programming change, the display may continue to show the previous pedestrian change interval for one signal cycle. During the following pedestrian change interval the countdown shall show the revised time, or shall be blank. In the event of an emergency vehicle preemption, during the following two cycles, the display shall

show the programmed pedestrian change interval or be blank. In the event the controller is put in stop time during the pedestrian change interval, during the following two cycles the display shall show the programmed clearance or be blank. In the event there is railroad preempt during the pedestrian change interval, during the following two cycles the display shall show the programmed clearance or be blank. Light emitting diode (LED) light sources having the incandescent appearance are required for Portland Orange Raised Hand and the Lunar White Walking Person.

b. Voltage: The operating voltages shall be between 85 VAC and 135 VAC.

c. Temperature: Temperature range shall be -35° F to +165° F.

d. LED pedestrian heads shall be supplied with Z crate visors. Z crate visors shall have 21 members at 45 degrees and 20 horizontal members.

9-29.20(2) Neon Grid Type

This section is deleted.

9-29.24 Service Cabinets

In the first paragraph, the lettered items A-J are re-lettered to read B-K respectfully.

The first paragraph is supplemented with the following new lettered item:

A. Display an arc flash warning label that meets the requirements of ANSI Z535.

9-29.25 Amplifier, Transformer, and Terminal Cabinets

In item No. 2.C., “Transformer 23.1 to 12.5 KVA” is revised to read “Transformer 3.1 to 12.5 KVA” and the height column value of 40” is revised to read “48”.

The first and second sentences in the first paragraph are revised to read:

Amplifier and terminal and transformer cabinets shall be NEMA 3R and the following:

Item number 5 is revised to read:

5. All cabinets shall provide a gasketed door flange

Item number 7 is revised to read:

7. Insulated terminal blocks shall be 600 volt, heavy-duty, barrier type. The terminal blocks shall be provided with a field-side and a control-side connector separated by a marker strip. One spare 12-position insulated terminal block shall be installed in each terminal cabinet and amplifier cabinet.

Item number 8 is revised to read:

8. Each non-pad mounted Terminal, Amplifier and Transformer cabinet shall have 1/4 inch drain holes in back corners. Each pad mounted Terminal, Amplifier and Transformer

cabinet shall drain to a sump and through a 3/8 inch diameter drain pipe to grade as detailed in the Standard Plans.

Item number 10 is revised to read:

10. Transformer cabinets shall have two separate compartments, one for the transformer and one for the power distribution circuit breakers. Each compartment shall be enclosed with a dead front. Each breaker shall be labeled with the device name by means of a screwed or riveted engraved name plate.

Section 9-34, Pavement Marking Material
April 2, 2012

9-34.2 Paint

The second paragraph is revised to read:

Blue and black paint shall comply with the requirements for yellow paint in Section 9-34.2(4) and Section 9-34.2(5), with the exception that blue and black paints do not need to meet the requirements for titanium dioxide, directional reflectance, and contrast ration.

INTRODUCTION TO THE SPECIAL PROVISIONS

(July 31, 2007)

The work on this project shall be accomplished in accordance with the *Standard Specifications for Road, Bridge and Municipal Construction*, 2012 edition, as issued by the Washington State Department of Transportation (WSDOT) and the American Public Works Association (APWA), Washington State Chapter (hereafter "Standard Specifications"). The Standard Specifications, as modified or supplemented by the Amendments to the Standard Specifications and these Special Provisions, all of which are made a part of the Contract Documents, shall govern all of the Work.

These Special Provisions are made up of both General Special Provisions (GSPs) from various sources, which may have project-specific fill-ins; and project-specific Special Provisions. Each Provision either supplements, modifies, or replaces the comparable Standard Specification, or is a new Provision. The deletion, amendment, alteration, or addition to any subsection or portion of the Standard Specifications is meant to pertain only to that particular portion of the section, and in no way should it be interpreted that the balance of the section does not apply.

The project-specific Special Provisions are not labeled as such. The GSPs are labeled under the headers of each GSP, with the date of the GSP and its source, as follows:

(May 18, 2007 APWA GSP)
(August 7, 2006 WSDOT GSP)

Also incorporated into the Contract Documents by reference are:

- *Manual on Uniform Traffic Control Devices for Streets and Highways*, currently adopted edition, with Washington State modifications, if any
- *Standard Plans for Road, Bridge and Municipal Construction*, WSDOT/APWA, current edition

Contractor shall obtain copies of these publications, at Contractor's own expense.

DIVISION 1
GENERAL REQUIREMENTS

DESCRIPTION OF WORK

(March 13, 1995)
This Contract provides for the improvement of Rocky Reach Trail Phase One and other work, all in accordance with the attached Contract Plans, these Contract Provisions, and the Standard Specifications.

1-01.3 Definitions

(March 13, 2012 APWA GSP)

Delete the heading **Completion Dates** and the three paragraphs that follow it, and replace them with the following:

Dates

Bid Opening Date
The date on which the Contracting Agency publicly opens and reads the Bids.

Award Date
The date of the formal decision of the Contracting Agency to accept the lowest responsible and responsive Bidder for the Work.

Contract Execution Date
The date the Contracting Agency officially binds the Agency to the Contract.

Notice to Proceed Date
The date stated in the Notice to Proceed on which the Contract time begins.

Substantial Completion Date
The day the Engineer determines the Contracting Agency has full and unrestricted use and benefit of the facilities, both from the operational and safety standpoint, any remaining traffic disruptions will be rare and brief, and only minor incidental work, replacement of temporary substitute facilities, plant establishment periods, or correction or repair remains for the Physical Completion of the total Contract.

Physical Completion Date
The day all of the Work is physically completed on the project. All documentation required by the Contract and required by law does not necessarily need to be furnished by the Contractor by this date.

Completion Date
The day all the Work specified in the Contract is completed and all the obligations of the Contractor under the contract are fulfilled by the Contractor. All documentation required by the Contract and required by law must be furnished by the Contractor before establishment of this date.

Final Acceptance Date
The date on which the Contracting Agency accepts the Work as complete.

Supplement this Section with the following:

All references in the Standard Specifications, Amendments, or WSDOT General Special Provisions, to the terms “State”, “Department of Transportation”, “Washington State Transportation Commission”, “Commission”, “Secretary of Transportation”, “Secretary”, “Headquarters”, and “State Treasurer” shall be revised to read “Contracting Agency”.

All references to “State Materials Laboratory” shall be revised to read “Contracting Agency designated location”.

All references to “final contract voucher certification” shall be interpreted to mean the final payment form established by the Contracting Agency.

The venue of all causes of action arising from the advertisement, award, execution, and performance of the contract shall be in the Superior Court of the County where the Contracting Agency's headquarters are located.

Additive
A supplemental unit of work or group of bid items, identified separately in the Bid Proposal, which may, at the discretion of the Contracting Agency, be awarded in addition to the base bid.

Alternate
One of two or more units of work or groups of bid items, identified separately in the Bid Proposal, from which the Contracting Agency may make a choice between different methods or material of construction for performing the same work.

Business Day
A business day is any day from Monday through Friday except holidays as listed in Section 1-08.5.

Contract Documents
See definition for "Contract".

Contract Time
The period of time established by the terms and conditions of the Contract within which the Work must be physically completed.

Notice of Award
The written notice from the Contracting Agency to the successful Bidder signifying the Contracting Agency's acceptance of the Bid Proposal.

Notice to Proceed
The written notice from the Contracting Agency or Engineer to the Contractor authorizing and directing the Contractor to proceed with the Work and establishing the date on which the Contract time begins.

Traffic
Both vehicular and non-vehicular traffic, such as pedestrians, bicyclists, wheelchairs, and equestrian traffic.

1-02 BID PROCEDURES AND CONDITIONS

1-02.1 Prequalification of Bidders

Delete this Section and replace it with the following:

1-02.1 Qualifications of Bidder
(January 24, 2011 APWA GSP)

Before award of a public works contract, a bidder must meet at least the minimum qualifications of RCW 39.04.350(1) to be considered a responsible bidder and qualified to be awarded a public works project.

1-02.2 Plans and Specifications

(June 27, 2011 APWA GSP)

Delete this section and replace it with the following:

Information as to where Bid Documents can be obtained or reviewed can be found in the Call for Bids (Advertisement for Bids) for the work.

After award of the contract, plans and specifications will be issued to the Contractor at no cost as detailed below:

To Prime Contractor	No. of Sets	Basis of Distribution
Reduced plans (11" x 17")	5	Furnished automatically upon award.
Contract Provisions	5	Furnished automatically upon award.
Large plans (e.g., 22" x 34")	3	Furnished only upon request.

Additional plans and Contract Provisions may be obtained by the Contractor from the source stated in the Call for Bids, at the Contractor's own expense.

(******)

Examination of Plans, Specifications and Site of Work

Section 1-02.4(2) is supplemented with the following:

(January 2, 2012)
The soils information used for study and design of this project is available for review by the bidder at the following location:

Appendix C

The soils information includes the following:

Boring logs, analysis, and recommendations

1-02.5 Proposal Forms

(June 27, 2011 APWA GSP)

Delete this section and replace it with the following:

The Proposal Form will identify the project and its location and describe the work. It will also list estimated quantities, units of measurement, the items of work, and the materials to be furnished at the unit bid prices. The bidder shall complete spaces on the proposal form that call for, but are not limited to, unit prices; extensions; summations; the total bid amount; signatures; date;

and, where applicable, retail sales taxes and acknowledgment of addenda; the bidder's name, address, telephone number, and signature; the bidder's D/M/WBE commitment, if applicable; a State of Washington Contractor's Registration Number; and a Business License Number, if applicable. Bids shall be completed by typing or shall be printed in ink by hand, preferably in black ink. The required certifications are included as part of the Proposal Form.

The Contracting Agency reserves the right to arrange the proposal forms with alternates and additives, if such be to the advantage of the Contracting Agency. The bidder shall bid on all alternates and additives set forth in the Proposal Form unless otherwise specified.

1-02.6 Preparation of Proposal

(June 27, 2011 APWA GSP)

Supplement the second paragraph with the following:

4. If a minimum bid amount has been established for any item, the unit or lump sum price must equal or exceed the minimum amount stated.

5. Any correction to a bid made by interlineation, alteration, or erasure, shall be initialed by the signer of the bid.

Delete the last paragraph, and replace it with the following:

The Bidder shall make no stipulation on the Bid Form, nor qualify the bid in any manner.

A bid by a corporation shall be executed in the corporate name, by the president or a vice president (or other corporate officer accompanied by evidence of authority to sign).

A bid by a partnership shall be executed in the partnership name, and signed by a partner. A copy of the partnership agreement shall be submitted with the Bid Form if any D/M/WBE requirements are to be satisfied through such an agreement.

A bid by a joint venture shall be executed in the joint venture name and signed by a member of the joint venture. A copy of the joint venture agreement shall be submitted with the Bid Form if any D/W/MBE requirements are to be satisfied through such an agreement.

(August 2, 2004)
The fifth and sixth paragraphs of Section 1-02.6 are deleted.

1-02.7 Bid Deposit

(October 1, 2005 APWA GSP)

Supplement this section with the following:

Bid bonds shall contain the following:

1. Contracting Agency-assigned number for the project;
2. Name of the project;
3. The Contracting Agency named as obligee;

4. The amount of the bid bond stated either as a dollar figure or as a percentage which represents five percent of the maximum bid amount that could be awarded;
5. Signature of the bidder's officer empowered to sign official statements. The signature of the person authorized to submit the bid should agree with the signature on the bond, and the title of the person must accompany the said signature;
6. The signature of the surety's officer empowered to sign the bond and the power of attorney.

If so stated in the Contract Provisions, bidder must use the bond form included in the Contract Provisions.

1-02.9 Delivery of Proposal

(August 15, 2012 APWA GSP, Option A)

Delete this section and replace it with the following:

Each proposal shall be submitted in a sealed envelope, with the Project Name and Project Number as stated in the Call for Bids clearly marked on the outside of the envelope, or as otherwise required in the Bid Documents, to ensure proper handling and delivery.

If the project has FHWA funding and requires DBE Written Confirmation Documents or Good Faith Effort Documentation, then to be considered responsive, the Bidder shall submit with their Bid Proposal, written Confirmation Documentation from each DBE firm listed on the Bidder's completed DBE Utilization Certification, form 272-056A EF, as required by Section 1-02.6.

The Contracting Agency will not open or consider any Bid Proposal that is received after the time specified in the Call for Bids for receipt of Bid Proposals, or received in a location other than that specified in the Call for Bids.

(******)

Public Opening Of Proposal

Section 1-02.12 is supplemented with the following:

Date Of Opening Bids

Sealed bids are to be received at the following location prior to the time Specified:

At the front reception area, located at Washington State Parks and Recreation Commission Office, 1111 Israel Road SW, Tumwater, Washington 98504-2650, until the bid deadline. All bids are not considered as received until stamped by Parks' reception area. Bids delivered in person will be received only at the front reception area.

The bid opening date for this project is 1:00 P.M., (fill in day), (fill in date). Bids received will be publicly opened and read after 1:00 P.M. on this date.

1-02.13 Irregular Proposals

(March 13, 2012 APWA GSP)

Revise item 1 to read:

1. A proposal will be considered irregular and will be rejected if:

a. The Bidder is not prequalified when so required;
b. The authorized proposal form furnished by the Contracting Agency is not used or is altered;
c. The completed proposal form contains any unauthorized additions, deletions, alternate Bids, or conditions;
d. The Bidder adds provisions reserving the right to reject or accept the award, or enter into the Contract;
e. A price per unit cannot be determined from the Bid Proposal;
f. The Proposal form is not properly executed;
g. The Bidder fails to submit or properly complete a Subcontractor list, if applicable, as required in Section 1-02.6;
h. The Bidder fails to submit or properly complete a Disadvantaged Business Enterprise Certification, if applicable, as required in Section 1-02.6;
i. The Bidder fails to submit written confirmation from each DBE firm listed on the Bidder's completed DBE Utilization Certification that they are in agreement with the bidders DBE participation commitment, if applicable, as required in Section 1-02.6, or if the written confirmation that is submitted fails to meet the requirements of the Special Provisions;
j The Bidder fails to submit DBE Good Faith Effort documentation, if applicable, as required in Section 1-02.6, or if the documentation that is submitted fails to demonstrate that a Good Faith Effort to meet the Condition of Award was made;
k. The Bid Proposal does not constitute a definite and unqualified offer to meet the material terms of the Bid invitation; or
l. More than one proposal is submitted for the same project from a Bidder under the same or different names.

AWARD AND EXECUTION OF CONTRACT

1-03.1 Consideration of Bids

(January 23, 2006 APWA GSP)

Revise the first paragraph to read:

After opening and reading proposals, the Contracting Agency will check them for correctness of extensions of the prices per unit and the total price. If a discrepancy exists between the price per unit and the extended amount of any bid item, the price per unit will control. If a minimum bid amount has been established for any item and the bidder's unit or lump sum price is less than the minimum specified amount, the Contracting Agency will unilaterally revise the unit or lump sum price, to the minimum specified amount and recalculate the extension. The total of extensions, corrected where necessary, including sales taxes where applicable and such additives and/or alternates as selected by the Contracting Agency, will be used by the Contracting Agency for award purposes and to fix the Awarded Contract Price amount and the amount of the contract bond.

1-03.3 Execution of Contract

(October 1, 2005 APWA GSP)

Revise this section to read:

Copies of the Contract Provisions, including the unsigned Form of Contract, will be available for signature by the successful bidder on the first business day following award. The number of copies to be executed by the Contractor will be determined by the Contracting Agency.

Within 15 calendar days after the award date, the successful bidder shall return the signed Contracting Agency-prepared contract, an insurance certification as required by Section 1-07.18, and a satisfactory bond as required by law and Section 1-03.4. Before execution of the contract by the Contracting Agency, the successful bidder shall provide any pre-award information the Contracting Agency may require under Section 1-02.15.

Until the Contracting Agency executes a contract, no proposal shall bind the Contracting Agency nor shall any work begin within the project limits or within Contracting Agency-furnished sites. The Contractor shall bear all risks for any work begun outside such areas and for any materials ordered before the contract is executed by the Contracting Agency.

If the bidder experiences circumstances beyond their control that prevents return of the contract documents within the calendar days after the award date stated above, the Contracting Agency may grant up to a maximum of 15 additional calendar days for return of the documents, provided the Contracting Agency deems the circumstances warrant it.

Contract Bond

Section 1-03.4 is supplemented with the following:

(June 27, 2011)
Release of Contract Bond will be 60 days following Contracting Agency Final Acceptance of Contract, provided following conditions are met:

1. Payment to the State with respect to taxes imposed pursuant to Title 82, RCW on Contracts totaling more than $ 35,000, a release has been obtained from the Washington State Department of Revenue.

2. Affidavits of Wages Paid for the Contractor and all Subcontractors are on file with the Contracting Agency (RCW 39.12.040).

3. A certificate of Payment of Contributions Penalties and Interest on Public Works Contract is received from the Washington State Employment Security Department.

4. Washington State Department of Labor and Industries (per Section 1-07.10) shows the Contractor, Subcontractor(s) and any lower tier Subcontractor(s) are current with payments of industrial insurance and medical aid premiums.

5. All claims, as provided by law, filed against the Contract Bond have been resolved.

SCOPE OF WORK

1-04.2 Coordination of Contract Documents, Plans, Special Provisions, Specifications, and Addenda

(March 13, 2012 APWA GSP)

Revise the second paragraph to read:

Any inconsistency in the parts of the contract shall be resolved by following this order of precedence (e.g., 1 presiding over 2, 2 over 3, 3 over 4, and so forth):

1. Addenda,
2. Proposal Form,
3. Special Provisions,
4. Contract Plans,
5. Amendments to the Standard Specifications,
6. Standard Specifications,
7. Contracting Agency's Standard Plans or Details (if any), and
8. WSDOT Standard Plans for Road, Bridge, and Municipal Construction.

CONTROL OF WORK

1-05.4 Conformity With and Deviations from Plans and Stakes

Add the following two new sub-sections:

1-05.4(1) Roadway and Utility Surveys

(October 1, 2005 APWA GSP)

The Contractor shall retail an experienced surveyor to provide all principal lines, grades, and measurements the Engineer deems necessary for completion of the work. These shall generally consist of one initial set of:

1. Slope stakes for establishing grading;
2. Trail grade stakes;
3. Centerline finish grade stakes for pavement sections wider than 25 feet; and
4. Offset points to establish line and grade for underground utilities such as water, sewers, and storm drains.

1-05.4(2) Bridge and Structure Surveys

(October 1, 2005 APWA GSP)

For all structural work such as bridges and retaining walls, the Contractor shall retain as a part of Contractor's organization an experienced team of surveyors.

The Contractor shall provide all surveys required to complete the structure, except the following primary survey control which will be provided by the Engineer:

1. Centerline or offsets to centerline of the structure.
2. Stations of abutments and pier centerlines.

3. A sufficient number of bench marks for levels to enable the Contractor to set grades at reasonably short distances.
4. Monuments and control points as shown in the Plans.

The Contractor shall establish all secondary survey controls, both horizontal and vertical, as necessary to assure proper placement of all project elements based on the primary control points provided by the Engineer. Survey work shall be within the following tolerances:

Stationing	+.01 foot
Alignment	+.01 foot (between successive points)
Superstructure Elevations	+.01 foot (from plan elevations)
Substructure Elevations	+.05 foot (from plan elevations)

During the progress of the work, the Contractor shall make available to the Engineer all field books including survey information, footing elevations, cross sections and quantities.

The Contractor shall be fully responsible for the close coordination of field locations and measurements with appropriate dimensions of structural members being fabricated.

1-05.7 Removal of Defective and Unauthorized Work

(October 1, 2005 APWA GSP)

Supplement this section with the following:

If the Contractor fails to remedy defective or unauthorized work within the time specified in a written notice from the Engineer, or fails to perform any part of the work required by the Contract Documents, the Engineer may correct and remedy such work as may be identified in the written notice, with Contracting Agency forces or by such other means as the Contracting Agency may deem necessary.

If the Contractor fails to comply with a written order to remedy what the Engineer determines to be an emergency situation, the Engineer may have the defective and unauthorized work corrected immediately, have the rejected work removed and replaced, or have work the Contractor refuses to perform completed by using Contracting Agency or other forces. An emergency situation is any situation when, in the opinion of the Engineer, a delay in its remedy could be potentially unsafe, or might cause serious risk of loss or damage to the public.

Direct or indirect costs incurred by the Contracting Agency attributable to correcting and remedying defective or unauthorized work, or work the Contractor failed or refused to perform, shall be paid by the Contractor. Payment will be deducted by the Engineer from monies due, or to become due, the Contractor. Such direct and indirect costs shall include in particular, but without limitation, compensation for additional professional services required, and costs for repair and replacement of work of others destroyed or damaged by correction, removal, or replacement of the Contractor's unauthorized work.

No adjustment in contract time or compensation will be allowed because of the delay in the performance of the work attributable to the exercise of the Contracting Agency's rights provided by this Section.

The rights exercised under the provisions of this section shall not diminish the Contracting Agency's right to pursue any other avenue for additional remedy or damages with respect to the Contractor's failure to perform the work as required.

1-05.11 Final Inspection

Delete this section and replace it with the following:

1-05.11 Final Inspections and Operational Testing

(October 1, 2005 APWA GSP)

1-05.11(1) Substantial Completion Date

When the Contractor considers the work to be substantially complete, the Contractor shall so notify the Engineer and request the Engineer establish the Substantial Completion Date. The Contractor's request shall list the specific items of work that remain to be completed in order to reach physical completion. The Engineer will schedule an inspection of the work with the Contractor to determine the status of completion. The Engineer may also establish the Substantial Completion Date unilaterally.

If, after this inspection, the Engineer concurs with the Contractor that the work is substantially complete and ready for its intended use, the Engineer, by written notice to the Contractor, will set the Substantial Completion Date. If, after this inspection the Engineer does not consider the work substantially complete and ready for its intended use, the Engineer will, by written notice, so notify the Contractor giving the reasons therefor.

Upon receipt of written notice concurring in or denying substantial completion, whichever is applicable, the Contractor shall pursue vigorously, diligently and without unauthorized interruption, the work necessary to reach Substantial and Physical Completion. The Contractor shall provide the Engineer with a revised schedule indicating when the Contractor expects to reach substantial and physical completion of the work.

The above process shall be repeated until the Engineer establishes the Substantial Completion Date and the Contractor considers the work physically complete and ready for final inspection.

1-05.11(2) Final Inspection and Physical Completion Date

When the Contractor considers the work physically complete and ready for final inspection, the Contractor by written notice, shall request the Engineer to schedule a final inspection. The Engineer will set a date for final inspection. The Engineer and the Contractor will then make a final inspection and the Engineer will notify the Contractor in writing of all particulars in which the final inspection reveals the work incomplete or unacceptable. The Contractor shall immediately take such corrective measures as are necessary to remedy the listed deficiencies. Corrective work shall be pursued vigorously, diligently, and without interruption until physical completion of the listed deficiencies. This process will continue until the Engineer is satisfied the listed deficiencies have been corrected.

If action to correct the listed deficiencies is not initiated within 7 days after receipt of the written notice listing the deficiencies, the Engineer may, upon written notice to the Contractor, take whatever steps are necessary to correct those deficiencies pursuant to Section 1-05.7.

The Contractor will not be allowed an extension of contract time because of a delay in the performance of the work attributable to the exercise of the Engineer's right hereunder.

Upon correction of all deficiencies, the Engineer will notify the Contractor and the Contracting Agency, in writing, of the date upon which the work was considered physically complete. That date shall constitute the Physical Completion Date of the contract, but shall not imply acceptance of the work or that all the obligations of the Contractor under the contract have been fulfilled.

1-05.11(3) Operational Testing

It is the intent of the Contracting Agency to have at the Physical Completion Date a complete and operable system. Therefore when the work involves the installation of machinery or other mechanical equipment; street lighting, electrical distribution or signal systems; irrigation systems; buildings; or other similar work it may be desirable for the Engineer to have the Contractor operate and test the work for a period of time after final inspection but prior to the physical completion date. Whenever items of work are listed in the Contract Provisions for operational testing they shall be fully tested under operating conditions for the time period specified to ensure their acceptability prior to the Physical Completion Date. During and following the test period, the Contractor shall correct any items of workmanship, materials, or equipment which prove faulty, or that are not in first class operating condition. Equipment, electrical controls, meters, or other devices and equipment to be tested during this period shall be tested under the observation of the Engineer, so that the Engineer may determine their suitability for the purpose for which they were installed. The Physical Completion Date cannot be established until testing and corrections have been completed to the satisfaction of the Engineer.

The costs for power, gas, labor, material, supplies, and everything else needed to successfully complete operational testing, shall be included in the unit contract prices related to the system being tested, unless specifically set forth otherwise in the proposal.

Operational and test periods, when required by the Engineer, shall not affect a manufacturer's guaranties or warranties furnished under the terms of the contract.

1-05.13 Superintendents, Labor and Equipment of Contractor

(March 25, 2009 APWA GSP)

Revise the seventh paragraph to read:

Whenever the Contracting Agency evaluates the Contractor's qualifications pursuant to Section 1-02.14, it will take these performance reports into account.

1-05.15 Method of Serving Notices

(March 25, 2009 APWA GSP)

Revise the second paragraph to read:

All correspondence from the Contractor shall be directed to the Project Engineer. All correspondence from the Contractor constituting any notification, notice of protest, notice of dispute, or other correspondence constituting notification required to be furnished under the

Contract, must be in paper format, hand delivered or sent via mail delivery service to the Project Engineer's office. Electronic copies such as e-mails or electronically delivered copies of correspondence will not constitute such notice and will not comply with the requirements of the Contract.

Add the following new section:

1-05.16 Water and Power
(October 1, 2005 APWA GSP)

The Contractor shall make necessary arrangements, and shall bear the costs for power and water necessary for the performance of the work, unless the contract includes power and water as a pay item.

Add the following new section:

1-05.17 Oral Agreements
(October 1, 2005 AWPA GSP)

No oral agreement or conversation with any officer, agent, or employee of the Contracting Agency, either before or after execution of the contract, shall affect or modify any of the terms or obligations contained in any of the documents comprising the contract. Such oral agreement or conversation shall be considered as unofficial information and in no way binding upon the Contracting Agency, unless subsequently put in writing and signed by the Contracting Agency.

(****)

CONTROL OF MATERIAL

Section 1-06 is supplemented with the following:

(August 6, 2012)
In accordance with Buy America requirements contained in 23 CFR 635.410, the major quantities of steel and iron construction material that is permanently incorporated into the project shall consist of American-made materials only. Buy America does not apply to temporary steel items, e.g., temporary sheet piling, temporary bridges, steel scaffolding and falsework.

Minor amounts of foreign steel and iron may be utilized in this project provided the cost of the foreign material used does not exceed one-tenth of one percent of the total contract cost or $2,500.00, whichever is greater.

American-made material is defined as material having all manufacturing processes occurring domestically. To further define the coverage, a domestic product is a manufactured steel material that was produced in one of the 50 States, the District of Columbia, Puerto Rico, or in the territories and possessions of the United States.

If domestically produced steel billets or iron ingots are exported outside of the area of coverage, as defined above, for any manufacturing process then the resulting product does not conform to the Buy America requirements. Additionally, products manufactured domestically

from foreign source steel billets or iron ingots do not conform to the Buy America requirements because the initial melting and mixing of alloys to create the material occurred in a foreign country.

Manufacturing begins with the initial melting and mixing, and continues through the coating stage. Any process which modifies the chemical content, the physical size or shape, or the final finish is considered a manufacturing process. The processes include rolling, extruding, machining, bending, grinding, drilling, welding, and coating. The action of applying a coating to steel or iron is deemed a manufacturing process. Coating includes epoxy coating, galvanizing, aluminizing, painting, and any other coating that protects or enhances the value of steel or iron. Any process from the original reduction from ore to the finished product constitutes a manufacturing process for iron.

Due to a nationwide waiver, Buy America does not apply to raw materials (iron ore and alloys), scrap (recycled steel or iron), and pig iron or processed, pelletized, and reduced iron ore.

The following are considered to be steel manufacturing processes:

1. Production of steel by any of the following processes:

    a. Open hearth furnace.
    b. Basic oxygen.
    c. Electric furnace.
    d. Direct reduction.

2. Rolling, heat treating, and any other similar processing.

3. Fabrication of the products.

    a. Spinning wire into cable or strand.

    b. Corrugating and rolling into culverts.
    c. Shop fabrication.

A certification of materials origin will be required for any items comprised of, or containing, steel or iron construction materials prior to such items being incorporated into the permanent work. The certification shall be on DOT Form 350-109EF provided by the Engineer, or such other form the Contractor chooses, provided it contains the same information as DOT Form 350-109EF.

LEGAL RELATIONS AND RESPONSIBILITIES TO THE PUBLIC

Laws to be Observed

1-07.1 Laws to be Observed

(October 1, 2005 APWA GSP)

Supplement this section with the following:

In cases of conflict between different safety regulations, the more stringent regulation shall apply.

The Washington State Department of Labor and Industries shall be the sole and paramount administrative agency responsible for the administration of the provisions of the Washington Industrial Safety and Health Act of 1973 (WISHA).

The Contractor shall maintain at the project site office, or other well known place at the project site, all articles necessary for providing first aid to the injured. The Contractor shall establish, publish, and make known to all employees, procedures for ensuring immediate removal to a hospital, or doctor's care, persons, including employees, who may have been injured on the project site. Employees should not be permitted to work on the project site before the Contractor has established and made known procedures for removal of injured persons to a hospital or a doctor's care.

The Contractor shall have sole responsibility for the safety, efficiency, and adequacy of the Contractor's plant, appliances, and methods, and for any damage or injury resulting from their failure, or improper maintenance, use, or operation. The Contractor shall be solely and completely responsible for the conditions of the project site, including safety for all persons and property in the performance of the work. This requirement shall apply continuously, and not be limited to normal working hours. The required or implied duty of the Engineer to conduct construction review of the Contractor's performance does not, and shall not, be intended to include review and adequacy of the Contractor's safety measures in, on, or near the project site.

1-07.2 State Taxes

Delete this section, including its sub-sections, in its entirety and replace it with the following:

1-07.2 State Sales Tax
(June 27, 2011 APWA GSP)

The Washington State Department of Revenue has issued special rules on the State sales tax. Sections 1-07.2(1) through 1-07.2(3) are meant to clarify those rules. The Contractor should contact the Washington State Department of Revenue for answers to questions in this area. The Contracting Agency will not adjust its payment if the Contractor bases a bid on a misunderstood tax liability.

The Contractor shall include all Contractor-paid taxes in the unit bid prices or other contract amounts. In some cases, however, state retail sales tax will not be included. Section 1-07.2(2) describes this exception.

The Contracting Agency will pay the retained percentage (or release the Contract Bond if a FHWA-funded Project) only if the Contractor has obtained from the Washington State Department of Revenue a certificate showing that all contract-related taxes have been paid (RCW 60.28.051). The Contracting Agency may deduct from its payments to the Contractor any amount the Contractor may owe the Washington State Department of Revenue, whether the amount owed relates to this contract or not. Any amount so deducted will be paid into the proper State fund.

1-07.2(1) State Sales Tax — Rule 171

WAC 458-20-171, and its related rules, apply to building, repairing, or improving streets, roads, etc., which are owned by a municipal corporation, or political subdivision of the state, or by the United States, and which are used primarily for foot or vehicular traffic. This includes storm or combined sewer systems within and included as a part of the street or road drainage system and power lines when such are part of the roadway lighting system. For work performed in such cases, the Contractor shall include Washington State Retail Sales Taxes in the various unit bid item prices, or other contract amounts, including those that the Contractor pays on the purchase of the materials, equipment, or supplies used or consumed in doing the work.

1-07.2(2) State Sales Tax — Rule 170

WAC 458-20-170, and its related rules, apply to the constructing and repairing of new or existing buildings, or other structures, upon real property. This includes, but is not limited to, the construction of streets, roads, highways, etc., owned by the state of Washington; water mains and their appurtenances; sanitary sewers and sewage disposal systems unless such sewers and disposal systems are within, and a part of, a street or road drainage system; telephone, telegraph, electrical power distribution lines, or other conduits or lines in or above streets or roads, unless such power lines become a part of a street or road lighting system; and installing or attaching of any article of tangible personal property in or to real property, whether or not such personal property becomes a part of the realty by virtue of installation.

For work performed in such cases, the Contractor shall collect from the Contracting Agency, retail sales tax on the full contract price. The Contracting Agency will automatically add this sales tax to each payment to the Contractor. For this reason, the Contractor shall not include the retail sales tax in the unit bid item prices, or in any other contract amount subject to Rule 170, with the following exception.

Exception: The Contracting Agency will not add in sales tax for a payment the Contractor or a subcontractor makes on the purchase or rental of tools, machinery, equipment, or consumable supplies not integrated into the project. Such sales taxes shall be included in the unit bid item prices or in any other contract amount.

1-07.2(3) Services

The Contractor shall not collect retail sales tax from the Contracting Agency on any contract wholly for professional or other services (as defined in Washington State Department of Revenue Rules 138 and 244).

State Taxes

The third paragraph of Section 1-07.2 is revised to read:

(June 27, 2011)
The Contracting Agency will release the Contract Bond only if the Contractor has obtained from the State Department of Revenue a certificate showing that all Contract-related taxes have been paid.

Environmental Regulations

Section 1-07.5 is supplemented with the following:

(September 20, 2010)
Environmental Commitments

The following Provisions summarize the requirements, in addition to those required elsewhere in the Contract, imposed upon the Contracting Agency by the various documents referenced in the Special Provision PERMITS AND LICENSES. Throughout the work, the Contractor shall comply with the following requirements:

(January 7, 2013)
Stormwater, dewatering water, or other authorized non-stormwater discharges that has come into contact with pH modifying substances such as concrete rubble, concrete pours or amended soils, need to be maintained between 6.5 – 8.5 standard units (su). If pH exceeds 8.5 su, the Contractor shall immediately discontinue work and initiate treatment to prevent discharges outside the acceptable range from occurring. All neutralization methods used shall be in accordance with the permit. Work may resume once treatment has been implemented and pH of the stormwater or authorized non-stormwater discharge is between 6.5 - 8.5 su or it can be demonstrated that high pH waters will not discharge to surface waters.

Stormwater, dewatering water, and other authorized non-stormwater discharges are monitored weekly for compliance with the turbidity benchmark (25 nephelometric turbidity units (ntu)) and the phone reporting trigger value (250 ntu) by the Contracting Agency. When the turbidity benchmark is breached, the best management practices (BMPs) installed on-site are not working adequately and need to be adapted, maintained or more BMPs shall be installed. When the turbidity phone reporting trigger value breached, immediate action becomes required in order to lower the turbidity to $\leq$25 ntu or to eliminate the discharge. Daily follow-up discharge samples will be collected at all locations where a discharge of 250 ntu or higher was collected unless the discharge was stopped or eliminated.

(August 3, 2009)
The Contractor shall notify the Engineer a minimum of 14 calendar days prior to commencing any work in environmentally sensitive areas, mitigation areas, and wetland buffers. Installation of construction fencing is excluded from this notice requirement. At the time of notification, the Contractor shall submit a work plan for review and approval detailing how the work will be performed. Plan detail must be sufficient to verify that work is in conformance with all contract provisions.

(August 3, 2009)
The intentional bypass of stormwater from all or any portion of a stormwater treatment system is prohibited without the approval of the Engineer.

(August 3, 2009)
No Contractor staging areas will be allowed within 50 feet of any waters of the State including wetlands.

(August 3, 2009)

Payment

All costs to comply with this special provision for the environmental commitments and requirements are incidental to the contract and are the responsibility of the Contractor. The Contractor shall include all related costs in the associated bid prices of the contract.

Permits and Licenses

Section 1-07.6 is supplemented with the following:

(September 20, 2010)

The Contracting Agency has obtained the below-listed permit(s) for this project. A copy of the permit(s) is attached as an appendix for informational purposes. All contacts with the permitting agency concerning the below-listed permit(s) shall be through the Engineer. The Contractor shall obtain additional permits as necessary. All costs to obtain and comply with additional permits shall be included in the applicable bid items for the work involved. Copies of these permits are required to be onsite at all times.

1) Douglas County Recreational Overlay and Site Plan Development Permits.
2) Douglas County Shoreline Substantial Development Permit.
3) WDFW Hydraulic Project Approval (HPA).
4) Construction Stormwater General Permit.

Load Limits

Section 1-07.7 is supplemented with the following:

(March 13, 1995)

If the sources of materials provided by the Contractor necessitates hauling over roads other than State Highways, the Contractor shall, at the Contractor's expense, make all arrangements for the use of the haul routes.

Wages

General

Section 1-07.9(1) is supplemented with the following:

(January 10, 2012)

The State rates incorporated in this contract are applicable to all construction activities associated with this contract.

Requirements for Nondiscrimination

Section 1-07.11 is supplemented with the following:

(January 3, 2011)

Requirement for Affirmative Action to Ensure Equal Employment Opportunity (Executive Order 11246)

1. The Contractor's attention is called to the Equal Opportunity Clause and the Standard Federal Equal Employment Opportunity Construction Contract Specifications set forth herein.

2. The goals and timetables for minority and female participation set by the Office of Federal Contract Compliance Programs, expressed in percentage terms for the Contractor's aggregate work force in each construction craft and in each trade on all construction work in the covered area, are as follows:

Women - Statewide

Timetable	Goal
Until further notice	6.9%

Minorities - by Standard Metropolitan Statistical Area (SMSA)

Spokane, WA:
SMSA Counties:
Spokane, WA — 2.8
WA Spokane.
Non-SMSA Counties — 3.0
WA Adams; WA Asotin; WA Columbia; WA Ferry; WA Garfield; WA Lincoln, WA Pend Oreille; WA Stevens; WA Whitman.

Richland, WA
SMSA Counties:
Richland Kennewick, WA — 5.4
WA Benton; WA Franklin.
Non-SMSA Counties — 3.6
WA Walla Walla.

Yakima, WA:
SMSA Counties:
Yakima, WA — 9.7
WA Yakima.
Non-SMSA Counties — 7.2
WA Chelan; WA Douglas; WA Grant; WA Kittitas; WA Okanogan.

Seattle, WA:
SMSA Counties:
Seattle Everett, WA — 7.2
WA King; WA Snohomish.
Tacoma, WA — 6.2
WA Pierce.
Non-SMSA Counties — 6.1
WA Clallam; WA Grays Harbor; WA Island; WA Jefferson; WA Kitsap; WA Lewis; WA Mason; WA Pacific; WA San Juan; WA Skagit; WA Thurston; WA Whatcom.

Portland, OR:
SMSA Counties:
Portland, OR-WA 4.5
WA Clark.
Non-SMSA Counties 3.8
WA Cowlitz; WA Klickitat; WA Skamania; WA Wahkiakum.

These goals are applicable to each nonexempt Contractor's total on-site construction workforce, regardless of whether or not part of that workforce is performing work on a Federal, or federally assisted project, contract, or subcontract until further notice. Compliance with these goals and time tables is enforced by the Office of Federal Contract compliance Programs.

The Contractor's compliance with the Executive Order and the regulations in 41 CFR Part 60-4 shall be based on its implementation of the Equal Opportunity Clause, specific affirmative action obligations required by the specifications set forth in 41 CFR 60-4.3(a), and its efforts to meet the goals. The hours of minority and female employment and training must be substantially uniform throughout the length of the contract, in each construction craft and in each trade, and the Contractor shall make a good faith effort to employ minorities and women evenly on each of its projects. The transfer of minority or female employees or trainees from Contractor to Contractor or from project to project for the sole purpose of meeting the Contractor's goal shall be a violation of the contract, the Executive Order and the regulations in 41 CFR Part 60-4. Compliance with the goals will be measured against the total work hours performed.

3. The Contractor shall provide written notification to the Office of Federal Contract Compliance Programs (OFCCP) within 10 working days of award of any construction subcontract in excess of $10,000 or more that are Federally funded, at any tier for construction work under the contract resulting from this solicitation. The notification shall list the name, address and telephone number of the Subcontractor; employer identification number of the Subcontractor; estimated dollar amount of the subcontract; estimated starting and completion dates of the subcontract; and the geographical area in which the contract is to be performed. The notification shall be sent to:

   District Director
   U.S. Department of Labor
   Office of Federal Contract Compliance Programs
   Seattle District Office
   1111 Third Avenue, Suite 745
   Seattle, WA 98101-3212

   Additional information may be found at the U.S. Department of Labor website: http://www.dol.gov/ofccp/TAguides/ctaguide.htm

4. As used in this Notice, and in the contract resulting from this solicitation, the Covered Area is as designated herein.

Standard Federal Equal Employment Opportunity Construction Contract Specifications (Executive Order 11246)

1. As used in these specifications:

    a. Covered Area means the geographical area described in the solicitation from which this contract resulted;

    b. Director means Director, Office of Federal Contract Compliance Programs, United States Department of Labor, or any person to whom the Director delegates authority;

    c. Employer Identification Number means the Federal Social Security number used on the Employer's Quarterly Federal Tax Return, U. S. Treasury Department Form 941;

    d. Minority includes:

        (1) Black, a person having origins in any of the Black Racial Groups of Africa.

        (2) Hispanic, a fluent Spanish speaking, Spanish surnamed person of Mexican, Puerto Rican, Cuban, Central American, South American, or other Spanish origin.

        (3) Asian or Pacific Islander, a person having origins in any of the original peoples of the Pacific rim or the Pacific Islands, the Hawaiian Islands and Samoa.

        (4) American Indian or Alaskan Native, a person having origins in any of the original peoples of North America, and who maintain cultural identification through tribal affiliation or community recognition.

2. Whenever the Contractor, or any Subcontractor at any tier, subcontracts a portion of the work involving any construction trade, it shall physically include in each subcontract in excess of $10,000 the provisions of these specifications and the Notice which contains the applicable goals for minority and female participation and which is set forth in the solicitations from which this contract resulted.

3. If the Contractor is participating (pursuant to 41 CFR 60-4.5) in a Hometown Plan approved by the U.S. Department of Labor in the covered area either individually or through an association, its affirmative action obligations on all work in the Plan area (including goals and timetables) shall be in accordance with that Plan for those trades which have unions participating in the Plan. Contractors must be able to demonstrate their participation in and compliance with the provisions of any such Hometown Plan. Each Contractor or Subcontractor participating in an approved Plan is individually required to comply with its obligations under the EEO clause, and to make a good faith effort to achieve each goal under the Plan in each trade in which it has employees. The overall good faith performance by other Contractors or Subcontractors toward a goal in an approved Plan does not excuse any covered Contractor's or Subcontractor's failure to take good faith effort to achieve the Plan goals and timetables.

4. The Contractor shall implement the specific affirmative action standards provided in paragraphs 7a through 7p of this Special Provision. The goals set forth in the solicitation from which this contract resulted are expressed as percentages of the total hours of employment and training of minority and female utilization the Contractor should reasonably be able to achieve in each construction trade in which it has employees in the covered area. Covered construction contractors performing construction work in geographical areas where they do not have a Federal or federally assisted construction contract shall apply the minority and female goals established for the geographical area where the work is being performed. The Contractor is expected to make substantially uniform progress in meeting its goals in each craft during the period specified.

5. Neither the provisions of any collective bargaining agreement, nor the failure by a union with whom the Contractor has a collective bargaining agreement, to refer either minorities or women shall excuse the Contractor's obligations under these specifications, Executive Order 11246, or the regulations promulgated pursuant thereto.

6. In order for the nonworking training hours of apprentices and trainees to be counted in meeting the goals, such apprentices and trainees must be employed by the Contractor during the training period, and the Contractor must have made a commitment to employ the apprentices and trainees at the completion of their training, subject to the availability of employment opportunities. Trainees must be trained pursuant to training programs approved by the U.S. Department of Labor.

7. The Contractor shall take specific affirmative actions to ensure equal employment opportunity. The evaluation of the Contractor's compliance with these specifications shall be based upon its effort to achieve maximum results from its action. The Contractor shall document these efforts fully, and shall implement affirmative action steps at least as extensive as the following:

    a. Ensure and maintain a working environment free of harassment, intimidation, and coercion at all sites, and in all facilities at which the Contractor's employees are assigned to work. The Contractor, where possible, will assign two or more women to each construction project. The Contractor shall specifically ensure that all foremen, superintendents, and other on-site supervisory personnel are aware of and carry out the Contractor's obligation to maintain such a working environment, with specific attention to minority or female individuals working at such sites or in such facilities.

    b. Establish and maintain a current list of minority and female recruitment sources, provide written notification to minority and female recruitment sources and to community organizations when the Contractor or its unions have employment opportunities available, and maintain a record of the organizations' responses.

    c. Maintain a current file of the names, addresses and telephone numbers of each minority and female off-the-street applicant and minority or female referral from a union, a recruitment source or community organization and of what action was taken with respect to each such individual. If such individual was sent to the union hiring hall for referral and was not referred back to the Contractor by the union or, if referred, not employed by the Contractor, this shall be documented in

the file with the reason therefor, along with whatever additional actions the Contractor may have taken.

d. Provide immediate written notification to the Director when the union or unions with which the Contractor has a collective bargaining agreement has not referred to the Contractor a minority person or woman sent by the Contractor, or when the Contractor has other information that the union referral process has impeded the Contractor's efforts to meet its obligations.

e. Develop on-the-job training opportunity and/or participate in training programs for the area which expressly include minorities and women, including upgrading programs and apprenticeship and trainee programs relevant to the Contractor's employment needs, especially those programs funded or approved by the U.S. Department of Labor. The Contractor shall provide notice of these programs to the sources compiled under 7b above.

f. Disseminate the Contractor's EEO policy by providing notice of the policy to unions and training programs and requesting their cooperation in assisting the Contractor in meeting its EEO obligations; by including it in any policy manual and collective bargaining agreement; by publicizing it in the company newspaper, annual report, etc.; by specific review of the policy with all management personnel and with all minority and female employees at least once a year; and by posting the company EEO policy on bulletin boards accessible to all employees at each location where construction work is performed.

g. Review, at least annually, the company's EEO policy and affirmative action obligations under these specifications with all employees having any responsibility for hiring, assignment, layoff, termination or other employment decisions including specific review of these items with on-site supervisory personnel such as Superintendents, General Foremen, etc., prior to the initiation of construction work at any job site. A written record shall be made and maintained identifying the time and place of these meetings, persons attending, subject matter discussed, and disposition of the subject matter.

h. Disseminate the Contractor's EEO policy externally by including it in any advertising in the news media, specifically including minority and female news media, and providing written notification to and discussing the Contractor's EEO policy with other Contractors and Subcontractors with whom the Contractor does or anticipates doing business.

i. Direct its recruitment efforts, both oral and written to minority, female and community organizations, to schools with minority and female students and to minority and female recruitment and training organizations serving the Contractor's recruitment area and employment needs. Not later than one month prior to the date for the acceptance of applications for apprenticeship or other training by any recruitment source, the Contractor shall send written notification to organizations such as the above, describing the openings, screening procedures, and tests to be used in the selection process.

j. Encourage present minority and female employees to recruit other minority persons and women and where reasonable, provide after school, summer and vacation employment to minority and female youth both on the site and in other areas of a Contractor's work force.

k. Validate all tests and other selection requirements where there is an obligation to do so under 41 CFR Part 60-3.

l. Conduct, at least annually, an inventory and evaluation of all minority and female personnel for promotional opportunities and encourage these employees to seek or to prepare for, through appropriate training, etc., such opportunities.

m. Ensure that seniority practices, job classifications, work assignments and other personnel practices, do not have a discriminatory effect by continually monitoring all personnel and employment related activities to ensure that the EEO policy and the Contractor's obligations under these specifications are being carried out.

n. Ensure that all facilities and company activities are nonsegregated except that separate or single-user toilet and necessary changing facilities shall be provided to assure privacy between the sexes.

o. Document and maintain a record of all solicitations of offers for subcontracts from minority and female construction contractors and suppliers, including circulation of solicitations to minority and female contractor associations and other business associations.

p. Conduct a review, at least annually, of all supervisors' adherence to and performance under the Contractor's EEO policies and affirmative action obligations.

8. Contractors are encouraged to participate in voluntary associations which assist in fulfilling one or more of their affirmative action obligations (7a through 7p). The efforts of a contractor association, joint contractor-union, contractor-community, or other similar group of which the Contractor is a member and participant, may be asserted as fulfilling any one or more of the obligations under 7a through 7p of this Special Provision provided that the Contractor actively participates in the group, makes every effort to assure that the group has a positive impact on the employment of minorities and women in the industry, ensure that the concrete benefits of the program are reflected in the Contractor's minority and female work-force participation, makes a good faith effort to meet its individual goals and timetables, and can provide access to documentation which demonstrate the effectiveness of actions taken on behalf of the Contractor. The obligation to comply, however, is the Contractor's and failure of such a group to fulfill an obligation shall not be a defense for the Contractor's noncompliance.

9. A single goal for minorities and a separate single goal for women have been established. The Contractor, however, is required to provide equal employment opportunity and to take affirmative action for all minority groups, both male and female, and all women, both minority and non-minority. Consequently, the Contractor may be in violation of the Executive Order if a particular group is employed in substantially disparate manner (for example, even though the Contractor has achieved its goals for women generally, the

Contractor may be in violation of the Executive Order if a specific minority group of women is underutilized).

10. The Contractor shall not use the goals and timetables or affirmative action standards to discriminate against any person because of race, color, religion, sex, or national origin.

11. The Contractor shall not enter into any subcontract with any person or firm debarred from Government contracts pursuant to Executive Order 11246.

12. The Contractor shall carry out such sanctions and penalties for violation of these specifications and of the Equal Opportunity Clause, including suspensions, terminations and cancellations of existing subcontracts as may be imposed or ordered pursuant to Executive Order 11246, as amended, and its implementing regulations by the Office of Federal Contract Compliance Programs. Any Contractor who fails to carry out such sanctions and penalties shall be in violation of these specifications and Executive Order 11246, as amended.

13. The Contractor, in fulfilling its obligations under these specifications, shall implement specific affirmative action steps, at least as extensive as those standards prescribed in paragraph 7 of this Special Provision, so as to achieve maximum results from its efforts to ensure equal employment opportunity. If the Contractor fails to comply with the requirements of the Executive Order, the implementing regulations, or these specifications, the Director shall proceed in accordance with 41 CFR 60-4.8.

14. The Contractor shall designate a responsible official to monitor all employment related activity to ensure that the company EEO policy is being carried out, to submit reports relating to the provisions hereof as may be required by the government and to keep records. Records shall at least include, for each employee, their name, address, telephone numbers, construction trade, union affiliation if any, employee identification number when assigned, social security number, race, sex, status (e.g., mechanic, apprentice, trainee, helper, or laborer), dates of changes in status, hours worked per week in the indicated trade, rate of pay, and locations at which the work was performed. Records shall be maintained in an easily understandable and retrievable form; however, to the degree that existing records satisfy this requirement, the Contractors will not be required to maintain separate records.

15. Nothing herein provided shall be construed as a limitation upon the application of other laws which establish different standards of compliance or upon the application of requirements for the hiring of local or other area residents (e.g., those under the Public Works Employment Act of 1977 and the Community Development Block Grant Program).

16. Additional assistance for Federal Construction Contractors on contracts administered by Washington State Department of Transportation or by Local Agencies may be found at:

    Washington State Dept. of Transportation
    Office of Equal Opportunity
    PO Box 47314
    310 Maple Park Ave. SE
    Olympia WA
    98504-7314

Ph: 360-705-7090
Fax: 360-705-6801
http://www.wsdot.wa.gov/equalopportunity/default.htm

(May 7, 2012)

Disadvantaged Business Enterprise Participation

The Disadvantaged Business Enterprise (DBE) requirements of 49 CFR Part 26 apply to this Contract. As such, the requirements of this Contract are to make affirmative efforts to solicit DBEs, provide information on who submitted a Bid or quote and to report DBE participation quarterly as described elsewhere in these Contract Provisions. No preference will be included in the evaluation of Bids/Proposals, no minimum level of DBE participation shall be required as a Condition of Award and Bids/Proposals may not be rejected or considered non-responsive on that basis.

DBE Goals

No DBE goals have been assigned as part of this Contract.

Affirmative Efforts to Solicit DBE Participation

DBE firms shall have an equal opportunity to compete for subcontracts in which the Contractor enters into pursuant to this Contract.

Contractors are encouraged to:

1. Advertise opportunities for Subcontractors or suppliers in a timely and reasonably designed manner to provide notice of the opportunity to DBEs capable of performing the Work. All advertisements should include a Contract Provision encouraging participation by DBE firms. This may be accomplished through general advertisements (e.g. newspapers, journals, etc.) or by soliciting Bids/Proposals directly from DBEs.

   Note: A Directory of Certified DBE Firms denoting the Description of Work the DBE Contractors are certified to perform is available at:

   www.omwbe.wa.gov/certification/index.shtml.

   The directory provides a plain language on the Description of Work that the listed DBE's have been certified by the Office of Minority and Women's Business Enterprises (OMWBE) to perform.

2. Establish delivery schedules that encourage participation by DBEs and other small businesses.

3. Participate with a DBE as a joint venture.

DBE Eligibility/Selection of DBEs for Reporting Purposes Only

Contractors may take credit for DBEs utilized on this Contract only if the firm is certified for the Work being performed.

Absent a mandatory goal, all DBE participation that is attained on this project will be considered as "race neutral" participation and shall be reported as such.

Crediting DBE Participation for Reporting Purposes

Joint Venture

When a DBE performs as a participant in a joint venture, only that portion of the total dollar value of the Contract equal to the distinct, clearly defined portion of the Work that the DBE performs with its own forces shall be credited.

DBE Prime Contractor

A DBE Prime Contractor may only take credit for that portion of the total dollar value of the Contract equal to the distinct, clearly defined portion of the Work that the DBE Prime Contractor performs with its own forces.

DBE Subcontractor

When a DBE firm participates as a Subcontractor, only that portion of the total dollar value of the Contract equal to the distinct, clearly defined portion of the Work that the DBE performs with its own forces shall be credited.

- Include the cost of supplies and materials obtained by the DBE for the Work in the Contract including supplies purchased or equipment leased by the DBE. However, you may not take credit supplies, materials, and equipment the DBE Subcontractor purchases or leases from the Prime Contractor or its affiliate. In addition, Work performed by a DBE, utilizing resources of the Prime Contractor or its affiliates shall not be credited.

- In very rare situations, a DBE firm may utilize equipment and/or personnel from a non-DBE firm other than the Prime Contractor or its affiliates. Should this situation arise the arrangement must be short-term and have prior written approval from the Office of Equal Opportunity (OEO). The arrangement must not impact a DBE firm's ability to perform a Commercially Useful Function (CUF).

- Count the entire value of fees or commissions charged by a DBE firm for providing a bona fide service, such as professional, technical, consultant, managerial services, or for providing bonds or insurance.

- When a DBE subcontracts to another firm, the value of the subcontracted Work may be counted as participation only if the DBE's lower tier Subcontractor is also a DBE.

- When non-DBE Subcontractor further subcontracts to a lower-tier Subcontractor or supplier who is a certified DBE, then that portion of the Work further subcontracted may be credited as DBE participation, so long as it is a distinct clearly defined portion of the Work that the DBE is performing with its own forces.
- If a firm is not certified as a DBE at the time of the execution of the contract, their participation cannot be counted toward any DBE goals.

Trucking

Use the following factors in determining DBE credit and whether a DBE trucking company is performing a Commercially Useful Function (CUF):

1. The DBE must be responsible for the management and supervision of the entire trucking operation for which credit is being claimed.

2. The DBE must itself own and, with its own workforce, operate at least one fully licensed, insured, and operational truck used on the Contract.

3. The DBE receives credit only for the value of the transportation services it provides on the Contract using trucks it owns or leases, licenses, insures, and operates with drivers it employs. For purposes of this requirement a lease must indicate that the DBE has exclusive use of and control over the truck. This does not preclude the leased truck from working for others provided it is with the consent of the DBE and the lease provides the DBE first priority for use of the leased truck. Leased trucks must display the name and identification number of the DBE.

4. The DBE may lease trucks from another DBE firm including an owner-operator provided they are certified as a DBE for trucking. The DBE who leases trucks from another DBE may claim participation for the total value of the transportation services the lessee DBE provides on the Contract.

5. The DBE may also lease trucks from a non-DBE firm and may enter into an agreement with an owner-operator who is a non-DBE. The DBE shall only receive credit for the number of additional non-DBE trucks equal or less than the number of DBE trucks the firms owns or has leased/subcontracted through another DBE trucking company. The DBE must control the work of the non-DBE trucks. If the non-DBE is performing the Work without supervision of that Work by the DBE, the DBE is not performing a Commercially Useful Function (CUF).

6. In any lease or owner-operator situation, as described in requirement #4 and #5 above, the following rules shall apply:

    a. A written lease/rental agreement is required for all trucks leased or rented; documenting the ownership and the terms of the agreement. The agreements must be submitted and approved by the Contracting Agency prior to the beginning of the Work. The agreement must show the leaser's name, truck description and agreed upon amount and method of payment (hour, ton, or per load). All lease agreements shall be for a long-term relationship, rather than for the individual project. (This requirement does not apply to owner-operator arrangements.)

    b. Only the vehicle, (not the operator) may be leased or rented. (This requirement does not apply to owner-operator arrangements.)

7. Credit may only be claimed for DBE trucking firms operating under a subcontract or a written agreement approved by the Contracting Agency prior to performing Work.

Expenditures paid to other DBEs

Expenditures paid to other DBEs for materials or supplies may be counted toward DBE goals as provided in the following:

Manufacturer

You may claim DBE credit for 100 percent of value of the materials or supplies obtained from a DBE manufacturer.

A manufacturer is a firm that operates or maintains a factory or establishment that produces, on the premises, the materials, supplies, articles, or equipment required under the contract. A manufacturer shall include firms that produce finished goods or products from raw or unfinished material or that purchases and substantially alters goods and materials to make them suitable for construction use before reselling them.

In order to receive credit as a DBE Manufacturer, the firm must be certified by OMWBE as a manufacturer in a NAICS code that falls within the 31XXXX to 33XXXX classification.

Regular Dealer

You may claim credit for 60 percent of the value of the materials or supplies purchased from a DBE regular dealer. Rules applicable to regular dealer status are contained in 49 CFR Part 26.55.e.2.

To be considered a regular dealer you must meet the following criteria:

1. WSDOT considers and recognizes a regular dealer, as a firm that owns, operates, or maintains a store, warehouse, or other establishment in which the materials or supplies required for the performance of the Contract and described by the specifications of the Contract are bought, kept in stock and regularly sold or leased to the public in the usual course of business.

2. Sixty percent (60%) of the cost of materials or supplies purchased from an approved regular dealer may be credited as DBE participation.

Regular dealer status is granted on a contract-by-contract basis. A firm wishing to be approved as a regular dealer for WSDOT contracted projects or Highways & Local Program administered projects must submit a request in writing to the OEO no later than seven days prior to bid opening.

Once the OEO has received the request, an onsite review will be set up with the firm and a review conducted to determine the firm's qualifications. If it is determined that the firm qualifies as a regular dealer the OEO will list the firm on an approved regular dealers List. The list may be accessed through the OEO Home website is at:

www.wsdot.wa.gov/equalopportunity

Note: Requests to be listed as a regular dealer will only be processed if the requesting firm is certified by the Office of Minority and Women's

Business Enterprises in a NAICS code that fall within the 42XXXX NAICS Wholesale code section.

Materials or Supplies Purchased from a DBE

With regard to materials or supplies purchased from a DBE who is neither a manufacturer nor a regular dealer you may claim credit for the following:

1. Fees or commissions charged for assistance in the procurement of the materials and supplies.

2. Fees or transportation charges for the delivery of materials or supplies.

In either case, you may not take credit for any part of the cost of the materials and supplies.

Joint Checking Allowance

Prime Contractors and DBEs must receive pre-approval by the OEO before using a joint check. Joint check requests shall be submitted, by the Prime Contractor to the Contracting Agency for approval.

When requesting approval for use of a joint checking allowance, the Contractor must distribute a written joint check agreement among the parties (including the suppliers involved) providing full and prompt disclosure of the expected use of the joint checks. The agreement shall contain all the information concerning the parties' obligations and consequences or remedies if the agreement is not fulfilled or a breach occurs. The joint check request shall be submitted to the Contracting Agency for approval prior to signing the Contract agreement.

The following are some general conditions that must be met by all parties regarding joint check use:

a. It is understood the Prime Contractor acts as the guarantor of a joint check.

b. The DBE's own funds are used to pay supplier of materials. The Prime Contractor does not make direct payment to supplier. In order to be performing a Commercially Useful Function (CUF), the DBE must release the check to the supplier (paying for the materials it-self and not be an extra participant in a transaction).

c. If the Prime Contractor makes joint checks available to one DBE Subcontractor, the service must be made available to all Subcontractors (DBE and non-DBE).

d. The relationship between the DBE and its suppliers should be established independently of and without interference by the Prime Contractor. The DBE has final decision-making responsibility concerning the procurement of materials and supplies, including which supplier to use.

e. The Prime Contractor and DBE shall be able to provide receipts, invoices, cancelled checks and/or certification statements of payment if requested by the Contracting Agency.

f. The DBE remains responsible for all other elements of 49 CFR 26.55(c) (1).

Failure by the Prime Contractor to request and to receive prior approval of a joint check arrangement will result in the joint check amount not counting towards the Prime Contractor's DBE goal.

Commercially Useful Function (CUF)
In any case, you may only take credit when the associated DBE that is determined to be performing a Commercially Useful Function (CUF).

- A DBE performs a CUF when it is responsible for execution of a distinct element of Work and is carrying out its responsibilities by performing, managing and supervising the Work involved. The DBE must also be responsible with respect to materials and supplies used on the Contract. For example; negotiating price, determining quality, determining quantities, ordering, installing (if applicable) and paying for the material itself.

- A DBE does not perform a CUF if its role is limited to that of an extra participant in a transaction, Contract, or project through which funds are passed.

Procedures Between Award and Execution
After award and prior to Execution of the Contract, the successful Bidder shall provide additional information as described below. Failure to comply may result in the forfeiture of the Bidder's Proposal bond or deposit.

A list of all firms who submitted a Bid or quote in an attempt to participate in this project whether they were successful or not.

Include the correct business name, federal employer identification number (optional) and a mailing address.

The firms identified by the Prime Contractor may be contacted by Contracting Agency to solicit general information as follows:

1. Age of the firm.

2. Average of its gross annual receipts over the past three years.

Procedures after Execution

Reporting

Quarterly Report of Amounts Credited as DBE Participation
Form #422-102

The Prime Contractor shall submit a Quarterly Report of Amounts Credited as DBE Participation form (422-102 EF) on a quarterly basis for any calendar quarter in which DBE Work is accomplished or upon completion of the

project, as appropriate. This is a record of payments to the DBE that the Prime Contractor is taking credit for as DBE participation. The dollars reported as specified in Crediting DBE Participation for Reporting Purposes section of this contract provision.

In the event that the payments to a DBE have been made by an entity other than the Prime Contractor (as in the case of a lower-tier Subcontractor or supplier), then the Prime Contractor shall obtain the quarterly report, including the signed affidavit, from the paying entity and submit the report to the Contracting Agency.

Payment

Compensation for all costs associated with complying with the conditions of this specification shall be included in payment for the associated Contract items of Work.

Prompt Payment

Prompt payment to all Subcontractors shall be in accordance with Section 1-08.1(1) of the Contract Provisions.

Damages for Noncompliance

The Prime Contractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this Contract. The Prime Contractor shall carry out applicable requirements of 49 CFR Part 26 in the award and administration of Contracts, which contain funding assistance from the United States Department of Transportation. Failure by the Prime Contractor to carry out these requirements is a material breach of this Contract, which may result in the termination of this Contract or such other remedy as the Contracting Agency deems appropriate.

Federal Agency Inspection

Section 1-07.12 is supplemented with the following:

(July 30, 2012)

Required Federal Aid Provisions

The Required Contract Provisions Federal Aid Construction Contracts (FHWA 1273) Revised May 1, 2012 supersede any conflicting provisions of the Standard Specifications and are made a part of this Contract; provided, however, that if any of the provisions of FHWA 1273 are less restrictive than Washington State Law, then the Washington State Law shall prevail.

The provisions of FHWA 1273 included in this Contract require that the Contractor insert the FHWA 1273 in each Subcontract, together with the wage rates which are part of the FHWA 1273. Also, a clause shall be included in each Subcontract requiring the Subcontractors to insert the FHWA 1273 thereto in any lower tier Subcontracts, together with the wage rates. The Contractor shall also ensure that this section, REQUIRED FEDERAL AID PROVISIONS, is inserted in each Subcontract for Subcontractors and lower tier Subcontractors. For this purpose, upon request to the Project Engineer, the Contractor will be provided with extra copies of the FHWA 1273, the applicable wage rates, and this Special Provision.

Contractor's Responsibility for Work

Repair of Damage

Section 1-07.13(4) is revised to read:

(August 6, 2001)
The Contractor shall promptly repair all damage to either temporary or permanent work as directed by the Engineer. For damage qualifying for relief under Sections 1-07.13(1), 1-07.13(2) or 1-07.13(3), payment will be made in accordance with Section 1-04.4. Payment will be limited to repair of damaged work only. No payment will be made for delay or disruption of work.

Protection and Restoration of Property

Vegetation Protection and Restoration

Section 1-07.16(2) is supplemented with the following:

(August 2, 2010)
Vegetation and soil protection zones for trees shall extend out from the trunk to a distance of 1 foot radius for each inch of trunk diameter at breast height.

Vegetation and soil protection zones for shrubs shall extend out from the stems at ground level to twice the radius of the shrub.

Vegetation and soil protection zones for herbaceous vegetation shall extend to encompass the diameter of the plant as measured from the outer edge of the plant.

Utilities and Similar Facilities

Section 1-07.17 is supplemented with the following:

(April 2, 2007)
Locations and dimensions shown in the Plans for existing facilities are in accordance with available information obtained without uncovering, measuring, or other verification.

The following addresses and telephone numbers of utility companies known or suspected of having facilities within the project limits are supplied for the Contractor's convenience:

*** $$1$$ ***

1-07.18 Public Liability and Property Damage Insurance

Delete this section in its entirety, and replace it with the following:

1-07.18 Insurance

(January 24, 2011 APWA GSP)

1-07.18(1) General Requirements

A. The Contractor shall obtain the insurance described in this section from insurers approved by the State Insurance Commissioner pursuant to RCW Title 48. The insurance must be provided by an insurer with a rating of A-: VII or higher in the A.M. Best's Key Rating Guide, which is licensed to do business in the state of Washington (or issued as a surplus line by a Washington Surplus lines broker). The Contracting Agency reserves the right to approve or reject the insurance provided, based on the insurer (including financial condition), terms and coverage, the Certificate of Insurance, and/or endorsements.

B. The Contractor shall keep this insurance in force during the term of the Contract and for thirty (30) days after the Physical Completion date, unless otherwise indicated (see C. below).

C. If any insurance policy is written on a claims made form, its retroactive date, and that of all subsequent renewals, shall be no later than the effective date of this Contract. The policy shall state that coverage is claims made, and state the retroactive date. Claims-made form coverage shall be maintained by the Contractor for a minimum of 36 months following the Final Completion or earlier termination of this Contract, and the Contractor shall annually provide the Contracting Agency with proof of renewal. If renewal of the claims made form of coverage becomes unavailable, or economically prohibitive, the Contractor shall purchase an extended reporting period ("tail") or execute another form of guarantee acceptable to the Contracting Agency to assure financial responsibility for liability for services performed.

D. The insurance policies shall contain a "cross liability" provision.

E. The Contractor's and all subContractors' insurance coverage shall be primary and non-contributory insurance as respects the Contracting Agency's insurance, self-insurance, or insurance pool coverage.

F. The Contractor shall provide the Contracting Agency and all Additional Insureds with written notice of any policy cancellation, within two business days of their receipt of such notice.

G. Upon request, the Contractor shall forward to the Contracting Agency a full and certified copy of the insurance policy(s).

H. The Contractor shall not begin work under the Contract until the required insurance has been obtained and approved by the Contracting Agency.

I. Failure on the part of the Contractor to maintain the insurance as required shall constitute a material breach of contract, upon which the Contracting Agency may, after giving five business days notice to the Contractor to correct the breach, immediately terminate the Contract or, at its discretion, procure or renew such insurance and pay any and all premiums in connection therewith, with any sums so expended to be repaid to the Contracting Agency on demand, or at

the sole discretion of the Contracting Agency, offset against funds due the Contractor from the Contracting Agency.

J. All costs for insurance shall be incidental to and included in the unit or lump sum prices of the contract and no additional payment will be made.

1-07.18(2) Additional Insured

All insurance policies, with the exception of Professional Liability and Workers Compensation, shall name the following listed entities as additional insured(s):

- the Contracting Agency and its officers, elected officials, employees, agents, and volunteers
- The State of Washington
- Public Utility District No. 1 of Chelan County

The above-listed entities shall be additional insured(s) for the full available limits of liability maintained by the Contractor, whether primary, excess, contingent or otherwise, irrespective of whether such limits maintained by the Contractor are greater than those required by this Contract, and irrespective of whether the Certificate of Insurance provided by the Contractor pursuant to 1-07.18(3) describes limits lower than those maintained by the Contractor.

1-07.18(3) Subcontractors

Contractor shall ensure that each subcontractor of every tier obtains and maintains at a minimum the insurance coverages listed in 1-07.18(5)A and 1-07.18(5)B. Upon request of the Contracting Agency, the Contractor shall provide evidence of such insurance.

1-07.18(4) Evidence of Insurance

The Contractor shall deliver to the Contracting Agency a Certificate(s) of Insurance and endorsements for each policy of insurance meeting the requirements set forth herein when the Contractor delivers the signed Contract for the work. The certificate and endorsements must conform to the following requirements:

1. An ACORD certificate or a form determined by the Contracting Agency to be equivalent.
2. Copies of all endorsements naming Contracting Agency and all other entities listed in 1-07.18(2) as Additional Insured(s), showing the policy number. The Contractor may submit a copy of any blanket additional insured clause from its policies instead of a separate endorsement. A statement of additional insured status on an ACORD Certificate of Insurance shall not satisfy this requirement.
3. Any other amendatory endorsements to show the coverage required herein.

1-07.18(5) Coverages and Limits

The insurance shall provide the minimum coverages and limits set forth below. Providing coverage in these stated minimum limits shall not be construed to relieve the Contractor from liability in excess of such limits. All deductibles and self-insured retentions must be disclosed and are subject to approval by the Contracting Agency. The cost of any claim payments falling within the deductible shall be the responsibility of the Contractor.

1-07.18(5)A Commercial General Liability

A policy of Commercial General Liability Insurance, including:

Per project aggregate
Premises/Operations Liability
Products/Completed Operations – for a period of one year following final acceptance of the work.
Personal/Advertising Injury
Contractual Liability
Independent Contractors Liability
Stop Gap / Employers' Liability
Explosion, Collapse, or Underground Property Damage (XCU)
Blasting (only required when the Contractor's work under this Contract includes exposures to which this specified coverage responds)

Such policy must provide the following minimum limits:

$1,000,000	Each Occurrence
$2,000,000	General Aggregate
$1,000,000	Products & Completed Operations Aggregate
$1,000,000	Personal & Advertising Injury, each offence

Stop Gap / Employers' Liability

$1,000,000	Each Accident
$1,000,000	Disease - Policy Limit
$1,000,000	Disease - Each Employee

1-07.18(5)B Automobile Liability

Automobile Liability for owned, non-owned, hired, and leased vehicles, with an MCS 90 endorsement and a CA 9948 endorsement attached if "pollutants" are to be transported. Such policy(ies) must provide the following minimum limit:

$1,000,000 combined single limit

1-07.18(5)C Workers' Compensation

The Contractor shall comply with Workers' Compensation coverage as required by the Industrial Insurance laws of the state of Washington.

1-07.18(5)E All Risk Builder's Risk

(May 10, 2006 APWA GSP)

Contractor shall purchase and maintain Builders Risk insurance covering interests of the Contracting Agency, the Contractor, Subcontractors, and Sub-subcontractors in the work. Builders Risk insurance shall be on a all-risk policy form and shall insure against the perils of fire and extended coverage and physical loss or damage including flood, earthquake, theft, vandalism, malicious mischief and collapse. The Builders Risk insurance shall include coverage for temporary buildings, debris removal, and damage to materials in transit or stored off-site. Such insurance shall cover "soft costs" including but not limited to design costs, licensing fees, and architect's and engineer's fees. Builders Risk insurance shall be written in the amount of the completed value of the project, with no coinsurance provisions.

The Builders Risk insurance covering the work shall have a deductible of $5,000 for each occurrence, which will be the responsibility of the Contractor. Higher deductibles for flood, earthquake and all other perils may be accepted by the Contracting Agency upon written request by the Contractor and written acceptance by the Contracting Agency. Any increased deductibles accepted by the Contracting Agency will remain the responsibility of the Contractor.

The Builders Risk insurance shall be maintained until final acceptance of the work by the Contracting Agency.

The Contractor and the Contracting Agency waive all rights against each other any of their Subcontractors, Sub-subcontractors, agents and employees, each of the other, for damages caused by fire or other perils to the extent covered by Builders Risk insurance or other property insurance applicable to the work. The policies shall provide such waivers by endorsement or otherwise.

Public Convenience and Safety

Construction Under Traffic

Section 1-07.23(1) is supplemented with the following:

(January 2, 2012)
Work Zone Clear Zone

The Work Zone Clear Zone (WZCZ) applies during working and nonworking hours. The WZCZ applies only to temporary roadside objects introduced by the Contractor's operations and does not apply to preexisting conditions or permanent Work. Those work operations that are actively in progress shall be in accordance with adopted and approved Traffic Control Plans, and other contract requirements.

During nonworking hours equipment or materials shall not be within the WZCZ unless they are protected by permanent guardrail or temporary concrete barrier. The use of temporary concrete barrier shall be permitted only if the Engineer approves the installation and location.

During actual hours of work, unless protected as described above, only materials absolutely necessary to construction shall be within the WZCZ and only construction vehicles absolutely necessary to construction shall be allowed within the WZCZ or allowed to stop or park on the shoulder of the roadway.

The Contractor's nonessential vehicles and employees private vehicles shall not be permitted to park within the WZCZ at any time unless protected as described above.

Deviation from the above requirements shall not occur unless the Contractor has requested the deviation in writing and the Engineer has provided written approval.

Minimum WZCZ distances are measured from the edge of traveled way and will be determined as follows:

Regulatory Posted Speed	Distance From Traveled Way (Feet)
35 mph or less	10 *
40 mph	15
45 to 55 mph	20
60 mph or greater	30

* or 2-feet beyond the outside edge of sidewalk

Minimum Work Zone Clear Zone Distance

1-07.24 Rights of Way

(October 1, 2005 APWA GSP)

Delete this section in its entirety, and replace it with the following:

Street right of way lines, limits of easements, and limits of construction permits are indicated in the Plans. The Contractor's construction activities shall be confined within these limits, unless arrangements for use of private property are made.

Generally, the Contracting Agency will have obtained, prior to bid opening, all rights of way and easements, both permanent and temporary, necessary for carrying out the work. Exceptions to this are noted in the Bid Documents or will be brought to the Contractor's attention by a duly issued Addendum.

Whenever any of the work is accomplished on or through property other than public right of way, the Contractor shall meet and fulfill all covenants and stipulations of any easement agreement obtained by the Contracting Agency from the owner of the private property. Copies of the easement agreements may be included in the Contract Provisions or made available to the Contractor as soon as practical after they have been obtained by the Engineer.

Whenever easements or rights of entry have not been acquired prior to advertising, these areas are so noted in the Plans. The Contractor shall not proceed with any portion of the work in areas where right of way, easements or rights of entry have not been acquired until the Engineer certifies to the Contractor that the right of way or easement is available or that the right of entry has been received. If the Contractor is delayed due to acts of omission on the part of the Contracting Agency in obtaining easements, rights of entry or right of way, the Contractor will be entitled to an extension of time. The Contractor agrees that such delay shall not be a breach of contract.

Each property owner shall be given 48 hours notice prior to entry by the Contractor. This includes entry onto easements and private property where private improvements must be adjusted.

The Contractor shall be responsible for providing, without expense or liability to the Contracting Agency, any additional land and access thereto that the Contractor may desire for temporary construction facilities, storage of materials, or other Contractor needs. However, before using any private property, whether adjoining the work or not, the Contractor shall file with the Engineer a written permission of the private property owner, and, upon vacating the premises, a written release from the property owner of each property disturbed or otherwise interfered with

by reasons of construction pursued under this contract. The statement shall be signed by the private property owner, or proper authority acting for the owner of the private property affected, stating that permission has been granted to use the property and all necessary permits have been obtained or, in the case of a release, that the restoration of the property has been satisfactorily accomplished. The statement shall include the parcel number, address, and date of signature. Written releases must be filed with the Engineer before the Completion Date will be established.

1-08 PROSECUTION AND PROGRESS

Add the following new section:

1-08.0 Preliminary Matters

(May 25, 2006 APWA GSP)

Add the following new section:

1-08.0(1) Preconstruction Conference

(October 10, 2008 APWA GSP)

Prior to the Contractor beginning the work, a preconstruction conference will be held between the Contractor, the Engineer and such other interested parties as may be invited. The purpose of the preconstruction conference will be:

1. To review the initial progress schedule;
2. To establish a working understanding among the various parties associated or affected by the work;
3. To establish and review procedures for progress payment, notifications, approvals, submittals, etc.;
4. To establish normal working hours for the work;
5. To review safety standards and traffic control; and
6. To discuss such other related items as may be pertinent to the work.

The Contractor shall prepare and submit at the preconstruction conference the following:

1. A breakdown of all lump sum items;
2. A preliminary schedule of working drawing submittals; and
3. A list of material sources for approval if applicable.

Add the following new section:

1-08.0(2) Hours of Work

(June 27, 2011 APWA GSP)

Except in the case of emergency or unless otherwise approved by the Contracting Agency, the normal straight time working hours for the Contract shall be any consecutive 8-hour period between 7:00 a.m. and 6:00 p.m. of a working day with a maximum 1-hour lunch break and a 5-day work week. The normal straight time 8-hour working period for the Contract shall be established at the preconstruction conference or prior to the Contractor commencing the work.

Written permission from the Engineer is required, if a Contractor desires to perform work on holidays, Saturdays, or Sundays; before 7:00 a.m. or after 6:00 p.m. on any day; or longer than an 8-hour period on any day. The Contractor shall apply in writing to the Engineer for such permission, no later than noon on the working day prior to the day for which the Contractor is requesting permission to work.

Permission to work between the hours of 10:00 p.m. and 7:00 a.m. during weekdays and between the hours of 10:00 p.m. and 9:00 a.m. on weekends or holidays may also be subject to noise control requirements. Approval to continue work during these hours may be revoked at any time the Contractor exceeds the Contracting Agency's noise control regulations or complaints are received from the public or adjoining property owners regarding the noise from the Contractor's operations. The Contractor shall have no claim for damages or delays should such permission be revoked for these reasons.

Permission to work Saturdays, Sundays, holidays, or other than the agreed upon normal straight time working hours Monday through Friday may be given subject to certain other conditions set forth by the Contracting Agency or Engineer. These conditions may include but are not limited to:

- The Engineer may require designated representatives to be present during the work. Representatives who may be deemed necessary by the Engineer include, but are not limited to: survey crews; personnel from the Contracting Agency's material testing lab; inspectors; and other Contracting Agency employees when in the opinion of the Engineer, such work necessitates their presence.
- On non-Federal aid projects, requiring the Contractor to reimburse the Contracting Agency for the costs in excess of straight-time costs for Contracting Agency representatives who worked during such times.
- Considering the work performed on Saturdays, Sundays, and holidays as working days with regard to the contract time.
- Considering multiple work shifts as multiple working days with respect to contract time, even though the multiple shifts occur in a single 24-hour period.

Subcontracting

Section 1-08.1 is supplemented with the following:

(October 12, 1998)

Prior to any subcontractor or lower tier subcontractor beginning work, the Contractor shall submit to the Engineer a certification (WSDOT Form 420-004 EF) that a written agreement between the Contractor and the subcontractor or between the subcontractor and any lower tier subcontractor has been executed. This certification shall also guarantee that these subcontract agreements include all the documents required by the Special Provision **Federal Agency Inspection**.

A Subcontractor or lower tier Subcontractor will not be permitted to perform any work under the contract until the following documents have been completed and submitted to the Engineer:

1. Request to Sublet Work (Form 421-012 EF), and
2. Contractor and Subcontractor or Lower Tier Subcontractor Certification for Federal-aid Projects (Form 420-004 EF).

The Contractor's records pertaining to the requirements of this Special Provision shall be open to inspection or audit by representatives of the Contracting Agency during the life of the contract and for a period of not less than three years after the date of acceptance of the contract. The Contractor shall retain these records for that period. The Contractor shall also guarantee that these records of all Subcontractors and lower tier Subcontractors shall be available and open to similar inspection or audit for the same time period.

Subcontract Completion and Return of Retainage Withheld

Section 1-08.1(1) is revised to read:

(June 27, 2011)
The following procedures shall apply to all subcontracts entered into as a part of this Contract:

Requirements

1. The Prime Contractor or Subcontractor shall make payment to the Subcontractor not later than ten (10) days after receipt of payment from the Contracting Agency for work satisfactorily completed by the Subcontractor, to the extent of each Subcontractor's interest therein.

2. Prompt and full payment of retainage from the Prime Contractor to the Subcontractor shall be made within 30 days after Subcontractor's Work is satisfactorily completed.

3. For purposes of this Section, a Subcontractor's work is satisfactorily completed when all task and requirements of the Subcontract have been accomplished and including any required documentation and material testing .

4. Failure by a Prime Contractor or Subcontractor to comply with these requirements may result in one or more of the following:

    a. Withholding of payments until the Prime Contractor or Subcontractor complies

    b. Failure to comply shall be reflected in the Prime Contractor's Performance Evaluation

    c. Cancellation, Termination, or Suspension of the Contract, in whole or in part

    d. Other sanctions as provided by the subcontractor or by law under applicable prompt pay statutes.

Conditions

This clause does not create a contractual relationship between the Contracting Agency and any Subcontractor as stated in Section 1-08.1. Also, it is not intended to bestow upon any Subcontractor, the status of a third-party beneficiary to the Contract between the Contracting Agency and the Contractor.

Payment
The Contractor will be solely responsible for any additional costs involved in paying retainage to the Subcontractors. Those costs shall be incidental to the respective Bid Items.

1-08.4 Prosecution of Work

Delete this section in its entirety, and replace it with the following:

1-08.4 Notice to Proceed and Prosecution of Work
(June 27, 2011 APWA GSP)

Notice to Proceed will be given after the Contract has been executed and the contract bond and evidence of insurance have been approved and filed by the Contracting Agency. The Contractor shall not commence with the work until the Notice to Proceed has been given by the Engineer. The Contractor shall commence construction activities on the project site within ten days of the Notice to Proceed Date, unless otherwise approved in writing. The Contractor shall diligently pursue the work to the physical completion date within the time specified in the Contract. Voluntary shutdown or slowing of operations by the Contractor shall not relieve the Contractor of the responsibility to complete the work within the time(s) specified in the Contract.

When shown in the Plans, the first order of work shall be the installation of high visibility fencing to delineate all areas for protection or restoration, as described in the Contract. Installation of high visibility fencing adjacent to the roadway shall occur after the placement of all necessary signs and traffic control devices in accordance with 1-10.1(2). Upon construction of the fencing, the Contractor shall request the Engineer to inspect the fence. No other work shall be performed on the site until the Contracting Agency has accepted the installation of high visibility fencing, as described in the Contract.

Time for Completion

Section 1-08.5 is supplemented with the following:

(March 13, 1995)
This project shall be physically completed within 90 working days.

Measurement and Payment

1-09.6 Force Account
(October 10, 2008 APWA GSP)

Supplement this section with the following:

The Contracting Agency has estimated and included in the Proposal, dollar amounts for all items to be paid per force account, only to provide a common proposal for Bidders. All such dollar amounts are to become a part of Contractor's total bid. However, the Contracting Agency does not warrant expressly or by implication, that the actual amount of work will correspond with

those estimates. Payment will be made on the basis of the amount of work actually authorized by Engineer.

1-09.9 Payments

(March 13, 2012 APWA GSP)

Delete the first four paragraphs and replace them with the following:

The basis of payment will be the actual quantities of Work performed according to the Contract and as specified for payment.

The Contractor shall submit a breakdown of the cost of lump sum bid items at the Preconstruction Conference, to enable the Project Engineer to determine the Work performed on a monthly basis. A breakdown is not required for lump sum items that include a basis for incremental payments as part of the respective Specification. Absent a lump sum breakdown, the Project Engineer will make a determination based on information available. The Project Engineer's determination of the cost of work shall be final.

Progress payments for completed work and material on hand will be based upon progress estimates prepared by the Engineer. A progress estimate cutoff date will be established at the preconstruction conference.

The initial progress estimate will be made not later than 30 days after the Contractor commences the work, and successive progress estimates will be made every month thereafter until the Completion Date. Progress estimates made during progress of the work are tentative, and made only for the purpose of determining progress payments. The progress estimates are subject to change at any time prior to the calculation of the final payment.

The value of the progress estimate will be the sum of the following:

1. Unit Price Items in the Bid Form — the approximate quantity of acceptable units of work completed multiplied by the unit price.
2. Lump Sum Items in the Bid Form — based on the approved Contractor's lump sum breakdown for that item, or absent such a breakdown, based on the Engineer's determination.
3. Materials on Hand — 100 percent of invoiced cost of material delivered to Job site or other storage area approved by the Engineer.
4. Change Orders — entitlement for approved extra cost or completed extra work as determined by the Engineer.

Progress payments will be made in accordance with the progress estimate less:

1. Retainage per Section 1-09.9(1), on non FHWA-funded projects;
2. The amount of progress payments previously made; and
3. Funds withheld by the Contracting Agency for disbursement in accordance with the Contract Documents.

Progress payments for work performed shall not be evidence of acceptable performance or an admission by the Contracting Agency that any work has been satisfactorily completed. The determination of payments under the contract will be final in accordance with Section 1-05.1.

(March 13, 1995)

Payments

Section 1-09.9 is supplemented with the following:

The quantity of the following items to be paid for on this project shall be the quantity shown in the Proposal, unless changes are made in accordance with Section 1-04.4 which affect this quantity. The quantity shown in the Proposal will be adjusted by the amount of the change and will be paid for as specified in Section 1-04.4.

Structure Excavation Class A Incl. Haul	750 C.Y.
Underdrain Pipe 6 in Diam.	190 L.F.
Gravel Backfill for Drain	6 C.Y.
Gravel Backfill for Wall	161 C.Y.
St. Reinf Bar for Bridge	25,000 L.B.
Conc. Class 4000 for Bridge	165 C.Y.
18in Diam. Augercast Piles	784 L.F.
Metal Fabric Railing	120 L.F.

The quantities in the Proposal are listed only for the convenience of the Contractor in determining the volume of work involved and are not guaranteed to be accurate. The prospective bidders shall verify these quantities before submitting a bid. No adjustments other than for approved changes will be made in the quantity even though the actual quantities required may deviate from those listed.

The unit contract price for these items shall be full pay to construct and complete this portion of the work.

Retainage

Section 1-09.9(1) content and title is deleted and replaced with the following:

(June 27, 2011)
Vacant

1-09.13(3) Claims $250,000 or Less

(October 1, 2005 APWA GSP)

Delete this Section and replace it with the following:

The Contractor and the Contracting Agency mutually agree that those claims that total $250,000 or less, submitted in accordance with Section 1-09.11 and not resolved by nonbinding ADR processes, shall be resolved through litigation unless the parties mutually agree in writing to resolve the claim through binding arbitration.

1-09.13(3)A Administration of Arbitration

(October 1, 2005 APWA GSP)

Revise the third paragraph to read:

> The Contracting Agency and the Contractor mutually agree to be bound by the decision of the arbitrator, and judgment upon the award rendered by the arbitrator may be entered in the Superior Court of the county in which the Contracting Agency's headquarters are located. The decision of the arbitrator and the specific basis for the decision shall be in writing. The arbitrator shall use the contract as a basis for decisions.

DIVISION 2
EARTHWORK

2-01 CLEARING, GRUBBING, AND ROADSIDE CLEANUP

2-01.1 Description

Section 2-01.1 is supplemented with the following:

(March 13, 1995)

Clearing and grubbing on this project shall be performed within the following limits:

The limits established by the Plans, staked by the Contractor, and approved by the Engineer.

Do not remove trees, shrubs, and other vegetation indicated to remain.

2-01.3 Construction Requirements

Section 2-01.3(1) is supplemented with the following:

Prune minor roots and branches of trees indicated to remain in a manner that will not compromise the survivability of the trees, where such roots and branches obstruct installation of new construction, under the field direction of the Engineer.

Tree removal shall include the removal and disposal of the entire tree including roots, stump and all associated debris. In areas where it is determined by the Engineer that the removal of the entire trunk is NOT feasible, the Contractor shall cut the trunk flush with ground level and provide stump treatment. The tree stump shall be treated to prevent resprouting with an approved herbicide according to label instructions.

Section 2-01.3(2) is supplemented with the following:

Use only hand methods for grubbing within drip line of remaining trees.

Fill depressions caused by clearing and grubbing operations with satisfactory native soil material, unless further excavation or earthwork is indicated. Place fill material in horizontal layers not exceeding eight (8) inches in loosed depth, and compact each layer to a density equal to adjacent original ground.

The Contractor shall remove and legally dispose of all tree trunks, branches and debris from the site.

2-01.3(3) Archeological Sites

Section 2-01.3(3) is supplemented with the following:

Do no excavation or surface disturbance in those areas designated as Archeological Sites except as approved by the Engineer.

2-01.4 Measurement

Section 2-01.4 is supplemented with the following:

No unit of measurement will apply to the lump sum price for “Orchard Tree Removal.”

2-01.5 Payment

Section 2-01.5 is supplemented with the following:

“Orchard Tree Removal”, per acre.

The contract bid price per lump sum for “Orchard Tree Removal”, including all **incidental** work, shall be full pay for all labor, material, tools and equipment necessary to satisfactorily complete the work as defined in the Standard Specifications and these Special Provisions.

2.02 REMOVAL OF STRUCTURES AND OBSTRUCTIONS

2-02.1 Description

Section 2-02.1 is supplemented with the following:

For the purpose of bid preparation, the removal of structure and obstruction work is described herein. Whether included herein, or shown on the plans, any removal of structure and obstruction work required to complete the project work shall be included in this bid item in accordance with Section 1-04.1. The work shall include, but limited to:

- Removal of asphalt concrete pavement
- Removal of cement concrete curb and curb and gutter
- Removal of base course and gravel material in seeded areas
- Removal of large (4-man) rocks
- Removal of signs
- Removal of storm drainage pipe and drainage structures as designated.

2-02.3 Construction Requirements

Add the following two new subsections:

Sawcutting

Where sawcutting is required, the sawcut shall be a minimum of three (3) inches deep. When the existing pavement is more than three (3) inches thick, the portion below three (3) inches may be broken after the sawcut is made. Care shall be taken to prevent damage to the remaining pavement. Any pavement damaged beyond the sawcut line shall be removed by sawcut and replaced at no cost to the Agency.

Depth of Removal

Remove pavement and curb and gutter to a depth of at least 2-feet below finished ground elevation. Remove footings for all fencing, gates, barricades, and signs to a depth of at least 3-feet below finished ground elevation. Remove 4-man rocks and conflicting storm drainage structures in their entirety, unless otherwise noted on plans.

2-03 ROADWAY EXCAVATION AND EMBANKMENT

2-03.3 Construction Requirements

2-03.3(7)Disposal of Surplus Materials

2-03.3(7)B Haul

Section 2-03.3(7) B is deleted and replaced with the following:

All costs in connection with hauling and disposal of surplus materials will be considered **incidental** to the various bid items of the project and no additional compensation will be made.

2-03.4 Measurement

Section 2-03.4 is supplemented with the following:

(March 13, 1995)
The embankment widening for guardrail will be measured by the cubic yard, between the original roadway slope and the neat lines of the widened embankment.

(March 13, 1995)
Only one determination of the original ground elevation will be made on this project. Measurement for roadway excavation and embankment will be based on the original ground elevations recorded previous to the award of this contract.

If discrepancies are discovered in the ground elevations which will materially affect the quantities of earthwork, the original computations of earthwork quantities will be adjusted accordingly.

Earthwork quantities will be computed, either manually or by means of electronic data processing equipment, by use of the average end area method or by the finite element analysis method utilizing digital terrain modeling techniques.

Copies of the ground cross-section notes will be available for the bidder's inspection, before the opening of bids, at the Project Engineer's office and at the Region office.

Upon award of the contract, copies of the original ground cross-sections will be furnished to the successful bidder on request to the Project Engineer.

2-06 SUBGRADE PREPARATION

2-06.5 Measurement and Payment

Subgrade Constructed under the Same Contract

Section 2-06.5(1) is deleted and replaced with the following:

The subgrade shall be shaped and maintained to drain at all times during construction, including temporary ditches, and modifications to drainage structures necessary to eliminate standing water on the subgrade.

All costs of protection of the subgrade, including replacement of damaged or contaminated suitable material, shall be considered **incidental** to and included in the unit contract prices for other items in the contract.

2-09 STRUCTURE EXCAVATION

2-09.3 Construction Requirements

2-09.3(1)C Removal of Unstable Base Material

Section 2-09.3(1)C is supplemented with the following:

(January 3, 2006)

If unsatisfactory foundation material, as determined by the Engineer, is encountered for placing bridge footings, the foundation material shall be excavated below the footing, and the unsatisfactory material replaced with gravel backfill for foundation Class A, or lean concrete, except, when the maximum design soil pressure is greater than five tons per square foot, lean concrete only shall be used for replacing the unsatisfactory material.

Lean concrete shall meet the requirements of Section 6-02.

The unsatisfactory material shall be removed to a maximum of 3 feet below the bottom of the footing elevation, unless the Engineer directs the Contractor to excavate deeper. Excavations greater than 3 feet below the bottom of the footing may require redesign of the footings and columns, for which the Engineer will furnish revised plans.

2-09.4 Measurement

The subsection **Lower Limits** of Section 2-09.4 is supplemented with the following:

(January 4, 2010)

Under girders, at end pier embankments, the lower limit will follow a line parallel to the bottom of the girders and three feet below them.

2-09.5 Payment

Section 2-09.5 is deleted and replaced with the following:

Structure Excavation, Class A, shall be **Lump Sum**, shall include haul, and shall be included in the various bid items where Class A excavation is required; no additional payment will be made.

Structure Excavation, Class B, shall be **incidental** to and included in various related drain pipe, storm sewer pipe, culvert, catch basin, manholes, vaults, and concrete inlet bid items included in the proposal, and no additional payment will be made.

Shoring or Extra Excavation, Class A, shall be **incidental** to and included in the various bid items where Class A excavation is required; no additional payment will be made.

Shoring or Extra Excavation, Class B, shall be **incidental** to and included in the unit contract prices for other items in the contract.

(March 13, 1995)

When lean concrete is used to backfill voids left by the removal of unsatisfactory foundation material, as determined by the Engineer, payment for this work shall be by force account as provided in Section 1-09.6.

To provide a common basis for all bidders, the Contracting Agency has estimated the amount of force account for "Force Account Lean Concrete" and has entered the amount in the Proposal to become a part of the total bid by the Contractor.

2-11 TRIMMING AND CLEANUP

2-11.5 Payment

Section 2-11.5 is deleted and replaced with the following:

Trimming and cleanup shall be considered **incidental** to and included in the various bid items included in the proposal, and no further compensation will be made.

END OF DIVISION 2

DIVISION 5
SURFACE TREATMENTS AND PAVEMENTS

5-04 HOT MIX ASPHALT

Description

Section 5-04.1 is supplemented with the following:

This work consists of furnishing and installing commercial HMA Class ½" to a compacted depth of 3" as shown on the Plans.

5-04.2 Materials

Section 5-04.2 is revised to read:

(January 7, 2013)

Materials shall meet the requirements of the following sections:

Asphalt Binder	9-02.1(4)
Cationic Emulsified Asphalt	9-02.1(6)
Anti-Stripping Additive	9-02.4
Warm Mix Asphalt Additive	9-02.5
Aggregates	9-03.8
Recycled Asphalt Pavement	9-03.8(3)B
Blending Sand	9-03.8(4)
Mineral Filler	9-03.8(5)
Recycled Material	9-03.21

The Contract documents may establish that the various mineral materials required for the manufacture of HMA will be furnished in whole or in part by the Contracting Agency. If the documents do not establish the furnishing of any of these mineral materials by the Contracting Agency, the Contractor shall be required to furnish such materials in the amounts required for the designated mix. Mineral materials include coarse and fine aggregates, blending sand, and mineral filler.

The Contractor may choose to utilize recycled asphalt pavement (RAP) or reclaimed asphalt shingles (RAS) in the production of HMA. The RAP may be from pavements removed under the Contract, if any, or pavement material from an existing stockpile. The RAS may be from reclaimed shingles.

If greater than 20 percent of the total weight of HMA is RAP or any amount of RAS is utilized in the production of HMA, the Contractor shall sample and test the RAP and RAS during stockpile construction in accordance with WSDOT FOP for AASHTO T 308 for the determination of the asphalt binder content and WSDOT FOP for WAQTC/AASHTO T 27/T 11 for the gradation of the aggregates. The RAP shall be sampled and tested at a frequency of one sample for every 1,000 tons produced and not less than ten samples per project. The RAS shall be sampled and tested at a frequency of one sample for every 100 tons produced and not less than ten samples per project. The asphalt content and gradation test data shall be reported to the Contracting Agency prior to or when submitting the mix design for verification testing. If utilized, the amount of RAS shall not exceed 5-percent of the total weight of the HMA. The Contractor shall include the RAP and RAS as part of the mix design as defined in these Specifications.

The grade of asphalt binder shall be as required by the Contract. Blending of asphalt binder from different sources is not permitted. For HMA with either a RAP percentage greater than 20 percent of the total weight or any amount of RAS the final blended asphalt binder (after inclusion of RAP, RAS, new asphalt binder and recycling agent) shall be the grade as required by the Contract and comply with the requirements of Section 9-02.1(4).

The Contractor may use warm mix asphalt (WMA) processes in the production of HMA with a RAP percentage of 20 percent of the total weight or less. WMA processes shall not be used in the production of HMA with a RAP percentage greater than 20 percent of the total weight or any amount of RAS. The Contractor shall submit to the Engineer for approval the process that is proposed and how it will be used in the manufacture of HMA.

When the Contracting Agency provides aggregates or provides a source for the production of aggregates, the Contract Provisions will establish the approximate percentage of asphalt binder required in the mixture for each class of HMA.

Production of aggregates shall comply with the requirements of Section 3-01.

Preparation of stockpile site, the stockpiling of aggregates, and the removal of aggregates from stockpiles shall comply with the requirements of Section 3-02.

Section 5-04.2 is supplemented with the following:

(January 3, 2011)
ESAL's
The number of ESAL's for the design and acceptance of the HMA shall be ***
$$1$$ *** million.

Construction Requirements

Section 5-04.3 is supplemented with the following:

(August 1, 2011)
Bridge transverse joint seals shall be constructed at the locations specified in the Plans and in accordance with the Standard Plans.

Hot poured joint sealant shall be installed in accordance with the manufacturer's written recommendations. The Contractor shall submit the manufacturer's written installation procedure to the Engineer prior to installation.

HMA Mixing Plant

Section 5-04.3(1) is supplemented with the following:

(November 12, 2012)

6. **Equipment for Processing RAP and RAS.** When producing HMA for mix designs with greater than 20 percent of the total weight RAP or any amount of RAS the HMA

plant shall be equipped with screens or a lump breaker to eliminate oversize RAP/RAS particles from entering the pug mill or drum mixer.

Hot Mix Asphalt Pavers

Preparation of Aggregates

Section 5-04.3(7) is revised to read:

(August 6, 2012)
The aggregates, RAP and RAS shall be stockpiled according to the requirements of Section 3-02. Sufficient storage space shall be provided for each size of aggregate, RAP and RAS. The Contractor may uniformly blend fine aggregate or RAP with the RAS as a method of preventing the agglomeration of RAS particles. The aggregates, RAP and RAS shall be removed from stockpile(s) in a manner to ensure a minimum of segregation when being moved to the HMA plant for processing into the final mixture. Different aggregate sizes shall be kept separated until they have been delivered to the HMA plant.

Mixing

Section 5-04.3(8) is supplemented with the following:

(August 6, 2012)
The following requirements shall apply to mix designs with greater than 20 percent of the total weight RAP or any amount of RAS:

After the required amounts of mineral materials, RAP, RAS, new asphalt binder and asphalt rejuvenator have been introduced into the mixer the HMA shall be mixed until a complete and uniform coating of the particles and a thorough distribution of the asphalt binder throughout the mineral materials, RAP and RAS is ensured.

When discharged, the temperature of the HMA shall not exceed the optimum mixing temperature by more than 25°F as shown on the mix design/anti-strip evaluation report or as approved by the Engineer. Storing or holding of the HMA in approved storage facilities will be permitted during the daily operation but in no event shall the HMA be held for more than 24 hours. HMA held for more than 24 hours after mixing shall be rejected. Rejected HMA shall be disposed of by the Contractor at no expense to the Contracting Agency. The storage facility shall have an accessible device located at the top of the cone or about the third point. The device shall indicate the amount of material in storage. No HMA shall be accepted from the storage facility when the HMA in storage is below the top of the cone of the storage facility, except as the storage facility is being emptied at the end of the working shift.

Recycled asphalt pavement (RAP) and reclaimed asphalt shingles (RAS) utilized in the production of HMA shall be sized prior to entering the mixer so that a uniform and thoroughly mixed HMA is produced. If there is evidence of the RAP or RAS not breaking down during the heating and mixing of the HMA, the Contractor shall immediately suspend production of HMA until changes have been approved by the Project Engineer.

Acceptance Sampling and Testing - HMA Mixture

Section 5-04.3(8)A is supplemented with the following:

(August 2, 2010)
Commercial Evaluation
The following HMA will be accepted by commercial evaluation:

"Trail Paving"

Test Section – HMA Mixtures

Section 5-04.3(8)A7 is supplemented with the following:

(August 6, 2012)
The following requirements shall apply to mix designs with greater than 20 percent RAP by weight or RAS:

For each class of HMA accepted by statistical evaluation, the Contractor shall construct a test section to determine whether the mixture meets the requirements of Sections 9-03.8(2) and 9-03.8(6). The test section shall be constructed at the beginning of paving and will be at least 600 tons and a maximum of 1,000 tons or as approved by the Project Engineer. No further wearing or leveling HMA will be paved the day of or the day following the construction of the test section. The mixture in the test section will be evaluated as a lot with a minimum of three sublots required. If more than one test section is required, each test section shall be a separate lot.

For a test section to be acceptable the pay factor (PF) for gradation, asphalt binder and Va shall be 0.95 or greater for each constituent and the remaining test requirements in Section 9-03.8(2) (dust/asphalt ration, sand equivalent, uncompacted void and fracture) shall conform to the requirements of that section.

For all HMA of the same class and PG asphalt binder grade, payment for the HMA test section(s) will be in accordance with the provisions of 5-04.5(1) Quality Assurance Price Adjustments.

Joints

Section 5-04.3(12) is supplemented with the following:

(January 5, 2004)
The HMA overlay shall be feathered to produce a smooth riding connection to the existing pavement.

HMA utilized in the construction of the feathered connections shall be modified by eliminating the coarse aggregate from the mix at the Contractor's plant or the commercial source or by raking the joint on the roadway, to the satisfaction of the Engineer.

END OF DIVISION 5

DIVISION 6
STRUCTURES

GENERAL REQUIREMENTS FOR STRUCTURES

Foundation Data

Section 6-01.2 is supplemented with the following:

(******)

The Contractor should review the geotechnical recommendations report prepared for this project. The geotechnical recommendations and boring log reports are available in Appendix C.

CONCRETE STRUCTURES

(BSP October 27, 2008)

Rapid Cure Silicone Sealant

Rapid cure silicone sealant shall be the following product conforming to the following specifications:

Dow Corning 902 RCS Joint Sealant

The joint sealant shall be a rapid cure, 100 percent silicone, low modulus, self-leveling, cold applied, two part formulation, which is compatible with the surfaces to which it is applied. Rapid cure is defined as developing sufficient integrity within eight hours to accommodate both horizontal thermal movements and vertical movements at the joint.

The joint sealant shall not be an acid cure sealant.

The joint sealant shall conform to the following properties:

<u>As Applied</u>

Extrusion rate	MIL S 8802	7 to 19.4 ounces/minute
Specific gravity	ASTM D 1475	1.25 to 1.35
Nonvolatile content	93 percent minimum	

<u>As Installed</u>

(at 77F, 50 percent relative humidity, and 48 hours cure)

Skin-over time	20 minutes maximum	
Joint elongation	ASTM D 5329*	600 percent minimum
Joint modulus	ASTM D 5329*	3 to 12 psi at 100% elongation

*Section 14 modified as follows:

Pull Rate = two inches/minute

Specimen joint size = 0.5 inches by 0.5 inches by 2 inches

The primer shall be as recommended by the sealant manufacturer.

The Contractor shall deliver the joint sealant to the job site in the sealant manufacturer's original sealed container. Each container shall be marked with the sealant manufacturer's name and lot or batch number. Each lot or batch shall be accompanied by the manufacturer's Materials Safety Data Sheet (MSDS), and Certificate of Compliance, identifying the sealant manufacturer and the lot or batch number, and certifying that the materials conform to the specified requirements.

The backer rod shall be closed cell expanded polyethylene foam as recommended by the sealant manufacturer and approved by the Engineer. The diameter of the backer rod shall be as recommended by the sealant manufacturer for the expansion joint opening at the time of installation.

(BSP June 26, 2000)
Joint Preparation and Installation Procedure

The Contractor shall submit the sealant manufacturer's recommended joint preparation and installation procedure to the Engineer for approval. The Contractor shall not begin preparing the bridge expansion joints for installing the sealant until receiving the Engineer's approval of the joint preparation and installation procedure.

(BSP August 3, 2009)
Placing Expansion Joint Sealant

The Contractor shall have the services of a qualified sealant manufacturer's technical representative physically present at the job site to assist in assuring the proper installation of the rapid cure silicone sealant, provide technical assistance for the use of the joint sealant, train the Contractor's personnel installing the joint sealant, and to observe and inspect the installation of at least the first complete joint.

The joint sealant shall not be placed against fresh concrete (excluding polymer concrete, polyester and elastomeric concrete) until at least seven days after concrete placement.

The Contractor shall clean the bridge expansion joints of all forms, dirt, form oil, grease, and other deleterious material. The Contractor shall clean and prepare the entire joint surface receiving the joint sealant in accordance with the joint preparation procedure as approved by the Engineer, and as recommended by the sealant manufacturer's technical representative, including two stage abrasive blasting surface preparation and compressed air cleaning. All steel surfaces to be in contact with the joint sealant shall be cleaned to an SSPC-SP10 condition. The joint receiving the sealant shall be sound, clean, dry, and frost free.

The Contractor shall apply the primer, as recommended by the sealant manufacturer, to all surfaces to be in contact with the joint sealant. On steel surfaces, the primer shall be dry to the touch prior to applying the joint sealant. On concrete surfaces, the primer shall cure at least 60 minutes prior to applying the joint sealant.

After the cleaned and prepared joint has received the Engineer's approval for joint dimensions, alignment, and preparation, the Contractor shall prime the bridge expansion joint surfaces, place the backer rod, and place the rapid cure silicone sealant in accordance with the joint installation procedure as approved by the

Engineer, and as recommended by the sealant manufacturer's technical representative.

If the joint width at the time of installation is less than 3/8 inch or greater than three inches, the Contractor shall not proceed with the expansion joint modification until the installation procedure is revised as recommended by the sealant manufacturer's technical representative and as approved by the Engineer.

After installing the rapid cure silicone sealant, the Contractor shall flood the joint area with water and test the joint for leakage. If leakage is detected, the bridge expansion joint system shall be repaired by the Contractor, as recommended by the sealant manufacturer and approved by the Engineer, at no additional expense to the Contracting Agency.

STEEL STRUCTURES

Shop Assembly

Check of Shop Assembly

Section 6-03.3(28)B is supplemented with the following:

(June 26, 2000)

If an assembly or stage of assembly is not approved by the Engineer, deficiencies shall be corrected and the assembly or stage of assembly shall be resubmitted to the Engineer for approval.

PILING

(******)

Description

Augercast grout piles are formed by the rotation of a continuous flight hollow-shaft auger into the ground to the tip elevation established by the requirements specified elsewhere in this section. Grout is then injected through the auger shaft as the auger is being withdrawn in such a way as to exert removing pressure on the withdrawing earth-filled auger as well as lateral pressure on the soil surrounding the grout-filled pile hole.

Equipment

The minimum inside diameter of the hollow shaft of the augerflight shall be 1-1/4 inches. Provide grout injection equipment with a grout pressure gauge in clear view of the equipment operator. Rate of grout injection and rate of auger withdrawal from the soil shall be so coordinated as to maintain at all times a positive pressure on this gauge which will, in turn, indicate the existence of a "removing pressure" on the bottom of the augerflight. Magnitude of this pressure and performance of other augering and grouting procedures, such as rate of augering, rate of grout injection, and control of grout return around the augerflight, are dependent on soil conditions and equipment capability and shall be at the option of the Contractor, subject to review by the Engineer. The auger hoisting equipment shall be capable of withdrawing the auger smoothly and at a constant rate.

Subsurface Data
Subsurface soil data logs are shown on geotechnical recommendations report in Appendix C.

Grout Pump
Provide a positive displacement grout pump of an approved design. The pump discharge capacity shall be calibrated in strokes per cubic meter foot or revolutions per cubic meter foot by a method approved by the Engineer. Remove oil or other rust inhibitors from mixing drums and pressure grout pumps prior to mixing and pumping.

Submittals

Shop Drawings
Detail drawings to demonstrate compliance of augering, mixing, and pumping equipment, installation, and installed piles with contract documents. Include with the drawings erection details and reinforcement as specified.

Product Data
A complete and accurate record of all augercast grout piles, indicating the pile location, diameter, length, elevation of tip and top of pile, and quantity and strength of grout material actually pumped in each pile hole.

Grout Pump
Materials
Grout Specimens for Laboratory Tests
Grout specimens for Contractor Tests

A description of the materials to be used and the proposed methods of operations.

Test Reports
Flow Cone Test

Certificates
Auger cast Grout Piles
Evidence to the Engineer that the Contractor has been engaged in the successful installation of auger cast grout piles for at least 5 years.

Closeout Submittals
Records
Specified records upon completion of work.

(******)
Materials
Grout
Provide grout consisting of a mixture of portland cement, a pozzolanic material when approved, fluidifier, sand, and water proportioned and mixed to produce a grout capable of being pumped with an ultimate compressive strength of 4000 psi at 28 days. Consistency shall not be less than 11 seconds when tested in accordance with paragraph Flow Cone Test. Other admixtures shall not be used.

Portland Cement
Portland cement shall conform to ASTM C 150

Pozzolan
Pozzolan shall be a fly ash or other approved pozzolanic material conforming to ASTM C 618, Class F.

Grout Fluidifier
Grout fluidifier shall conform to ASTM C 937, except that expansion shall not exceed 4 percent. The fluidifier shall be a compound possessing characteristics which will increase the flowability of the mixture, assist in the dispersal of cement grains, and neutralize the setting shrinkage of the high-strength cement mortar.

Water
Water shall be fresh, clean, and free from sewage, oil, acid, alkali, salts, or organic matter.

Aggregate
Aggregate shall meet the requirements of ASTM C 33, for fine aggregate, except as to grading. The sand shall consist of hard, dense, durable, uncoated rock fragments and shall be free from injurious amounts of silt, lumps, loam, soft, or flaky particles, shale, alkali, organic matter, mica, and other deleterious substances. If washed, the method shall not remove other desirable fines, and the sand shall be permitted to drain until the residual free moisture is reasonably uniform and stable. Sand grading shall be reasonably consistent and shall conform to the following requirements as delivered to the grout mixer:

U.S. Standard Sieve Number	Cumulative Percent by Weight Passing	Cumulative Percent by Weight Retained
8	100	0
16	95-100	0-5
30	55-80	20-45
50	30-55	45-70
100	10-30	70-90
200	0-10	90-100

The sand shall have a fineness modulus of not less than 1.30 nor more than 2.10. Sand grading shown above may be modified with the approval of the Engineer. Mortar test specimens made with the modified sand shall exhibit compressive strength equal to or greater than that exhibited by similar specimens made with sand meeting grading and other requirements shown above.

Reinforcement
Materials, assembly, and placement of reinforcement shall conform to the requirements of Section 6-02.3(24)

(******)

Construction Requirements

Installation

The ground surface at each pile location at the time of augering and grouting shall be at least 12 inches higher than the required pile cutoff elevation. All materials shall be fed to the mixer accurately measured by weight, except water that may be measured by volume. The order of placing the materials shall be as follows: (1) water, (2) fluidifier, and (3) other solids in order of increasing particle size. Time of mixing shall not be less than 1 minute. Do not proceed with the installation of contract piles within any area of substantially different subsoil conditions until a satisfactory load test has been performed in that area.

Drilling

Except where auger withdrawal is required or directed by the Engineer, each pile hole shall be drilled and filled with grout in an uninterrupted operation. Drill each pile hole to the required tip elevation. Should the required tip elevation shown on the drawings differ from the calculated tip elevation, an adjustment in the contract requirements will be made. Advance the auger at a continuous rate which prevents removal of excess soil. Stop rotation of auger after reaching the required pile tip elevation.

Grouting and Auger Removal

At the start of pumping grout, raise the auger from 6 to 12 inches and after grout pressure builds up, indicating discharge of grout, redrill auger to the required tip elevation, and fill pile hole with grout without interruption. When the auger is withdrawn to check the soil profile, it shall be reinserted in the pile hole to the required tip elevation and the pile hole then filled with grout without interruption. Coordinate rate of grout injection and rate of auger removal from the soil in such a manner as to maintain a positive pressure on the grout pressure gauge. The gauge indicates the existence of a removing pressure on the bottom of the augerflight. If the auger jumps upward during withdrawal, or if the grouting process is interrupted, or if there is decreased grouting pressure, reinsert it to the original tip elevation and decrease the rate of withdrawal to prevent further jumping. The auger may rotate very slowly during withdrawal. However, counterclockwise rotation is not permitted.

Pile Butts

Place a steel sleeve at top of pile to form the pile butt. For pile cutoff above ground surface, the steel sleeve shall extend from the pile cutoff elevation to a point not less than one foot below the ground surface. For pile cutoff at or below ground surface, the steel sleeve shall extend from the ground surface to a point not less than one foot below the pile cutoff elevation. Pump excess grout to displace as much potential laitance as possible. Remove pile butt to required cutoff elevation or to sound grout, whichever is lower.

Placement Tolerances

Locate piles where indicated. The maximum permissible variation of the center of each pile from the required location is 2 inches at the ground surface. No pile shall be out of required axial alignment by more than 2 percent. Periodically check the required axial alignment of each pile during the drilling operation and after reaching required tip elevation with not less than 5 feet of the augerflight extending above ground surface. Abandon piles which are damaged, mislocated, or out of alignment beyond the maximum tolerance and provide additional piles where directed.

Cutoff

Removal of pile butts above the indicated cutoff elevation may be accomplished by dipping the grout from the pile, while grout is fluid, but not less than one hour after installation. At the option of the Contractor, and as approved prior to pile installation, grout may be allowed to harden at its initial top elevation and then carefully trimmed off to the indicated cutoff elevation with hand operated chipping guns.

Disposal of Excavated Material

Do not leave any piles partially completed overnight. Completely grout and protect piles at the termination of each day's operation. Dispose of excavated material, resulting from augering.

Flow Cone Test

The quantity of water used shall produce a grout having a consistency of not less than 21 seconds when tested with a flow cone in accordance with ASTM C 939. The flow cone shall be modified by removal of the 1/2 inch orifice allowing grout to pass through the 3/4 inch hole in bottom of cone. Conduct tests at the beginning of grout injection and at subsequent intervals to ensure specification requirements are met.

Grout Specimens for Laboratory Tests

Conduct grout tests in accordance with ASTM C 109, ASTM C 942 in a laboratory, approved by the Engineer. Prepare test specimens by pouring grout into 2 by 2 by 2 inch cube molds. Not less than 9 cubes shall be cast during each 8-hour shift. Three cubes shall be tested at 7 days; 3 at 28 days; and 3 at 90 days.

Grout Specimens for Contractor Tests

Conduct grout tests in accordance with ASTM C 31 and ASTM C 39. Prepare test specimens of grout by pouring grout into 6 by 12 inch cylinder molds. Provide molds with a top cover plate so designed as to restrain grout expansion and to permit escape of air and water. Not less than one set of cylinders shall be collected during the placing of each group of 15 piles or fraction thereof. One set shall consist of six cylinders of which three cylinders shall be tested in 7 days and three cylinders at 28 days. Any set of cylinders of which one or more cylinders test at 10 percent or more below the required strength shall be cause for rejection of the pile group.

Protection of Piles

The sequence of pile installation shall be such that adjacent piles show no evidence of disturbance. This evidence would actually appear as a drop in the grout surface. The load applied to the soil by the drilling equipment shall be far enough away from the pile being drilled to avoid compressing or shearing of the soil which may in turn displace or squeeze-off the grout column. No piles shall be placed within 5 feet of adjacent piles until the grout in the piles has set for 3 days, unless otherwise directed by the Engineer.

Records

Keep complete and accurate records of all auger cast grout piles. Indicate the pile location, diameter, length, elevation of tip and top of pile, quantity of grout material actually pumped in each pile hole, and the rated load capacity of the pile. Determine grout quantity by recording grout pump displacement or by other approved means. Record and report immediately any unusual conditions encountered during pile installation.

(******)

Measurement

Unit Price Basis of Payment

The Engineer reserves the right to increase or decrease the length of piles to be furnished and installed by changing the foundation pile locations or elevations, by requiring the installation of additional piles, or by requiring omission of piles from the requirements shown and specified. Whether or not such changes are made, the Contractor will be paid at the contract unit price per linear foot, multiplied by the total linear feet of acceptable piles actually installed.

(******)

Payment

Full Compensation

Payment in accordance with paragraph Unit Price Basis of Payment will constitute full compensation for furnishing, delivering, handling, and/or installing (as applicable) all material, labor and equipment necessary to meet contract requirements applicable to the foundation piles. The Contractor will not be allowed payment for withdrawn, broken, or rejected piles nor for a portion of any pile remaining above the cut-off point.

END OF DIVISION 6

DIVISION 7
DRAINAGE STRUCTURES, STORM SEWERS, SANITARY SEWERS, WATER MAINS, AND CONDUITS

7-06 WATER IRRIGATION LINE CARRIER AND CASING PIPE (NEW SECTION)

7-06.1 Description

This work consists of furnishing and installing water irrigation line carrier pipe and steel casing pipe as shown on the Plans. Field verify existing location, size, and material type of all existing water irrigation lines crossing the trail project. Relocate and reconnect existing irrigation lines into a new carrier and casing pipe and adjust replacement extent as required to provide dimensions and depths shown on the Plans.

7-06.2 Materials

Material requirements are as indicated on the Plans.

7-06.3 Construction Requirements

Construction requirements are as indicated on the Plans.

7-06.4 Measurement

No specific unit of measurement will apply to the Water Irrigation Line Carrier and Casing Pipe.

7-06.5 Payment

“”Water Irrigation Line Carrier and Casing Pipe”, per force account, shall be full pay for all work to furnish and install the number of existing irrigation lines encountered in the field and as shown indicatively on the Plans including excavation, installation, placement, backfill, and compaction.

END OF DIVISION 7

DIVISION 8
MISCELLANEOUS CONSTRUCTION

8-01 EROSION CONTROL AND WATER POLLUTION CONTROL

8-01.1 Description

Section 8-01.1 is supplemented with the following:

This work shall include, but not be limited to, inlet protection, silt fence, straw bale, and other measures needed to protect disturbed areas, and control and prevent pollution, erosion, run-off, and related damage, as shown on plans or as directed by the Engineer.

8-01.3(1) General

The tenth paragraph of Section 8-01.3(1) is revised to read:

(January 25, 2010)
Erodible Soil Eastern Washington
Erodible soil not being worked whether at final grade or not, shall be covered within the following time period using an approved soil cover practice:

July 1 through September 30	30 days
October 1 through June 30	15 days

8-01.3(2)B Seeding, Fertilizing, and Mulching

Section 8-01.3(2)B is supplemented with the following:

Grass seed, of the following composition, proportion, and quality shall be applied at the rates shown below on all areas requiring roadside seeding within the project:

Kind and Variety of Seed in Mixture by Common Name and (Botanical name)	Pounds Pure Live Seed (PLS) Per Acre
Thickspike Wheatgrass "Schwindemar" (*Agropyron trachycaulum)*	6.00
*Bluebunch Wheatgrass "Goldar" (*Agropyron spicatum)* *(Pseudoroegneria spicata)*	8.25
Sand Dropseed *(Sporobolus cryptandrus)*	0.14
*Sandberg Bluegrass "Canby" *(Poa sandbergii)*	2.25
Common Yarrow (*Achillea millefolium)*	0.01

Idaho Fescue (*Festuca idahoensis)*	5.00
Total Pound PLS Per Acre	21.65

*Source identified seed shall be fourth generation or earlier.

Non-Source Identified deed shall meet or exceed Washington State Department of Agriculture Certified Seed Standards and be from within the Columbia Plateau Ecoregion, as defined by the US Environmental Protection Agency (EPA) and shown at:

http://www.wsdot.wa.gov/publications/fulltext/Roadside/eco_regions_v9.jpg

Seeds shall be certified "Weed Free," indicating there are no noxious or nuisance weeds in the seed.

Mulching

Section 8-01.3(2)D is supplemented with the following:

Wood cellulose fiber mulch shall be applied at a rate of 2,000 pounds per acre.

8-01.3(8) Street Cleaning

Section 8-01.3(8) is supplemented with the following:

Contractor shall be responsible for controlling dust and mud within the project limits. Contractor shall clean up on a daily basis all refuse, rubbish, scrap material and debris caused by the work, to the end that, at all times, the site of the work shall present a neat, orderly and workmanlike appearance.

8-02 ROADSIDE RESTORATION

8-02.1 Description

Section 8-02.1 is supplemented with the following:

The work described in this section includes providing all materials, tools, equipment, and labor for soil preparation, finish grading, sodding, fertilizing, and maintenance.

8-02.2 Materials

Section 8-02.2 is supplemented with the following:

Sod shall be locally grown fescue/rye mix from a source approved by the Project Engineer.

8-02.3(1) Responsibility During Construction

Section 8-02.3(1) is supplemented with the following:

Throughout planting operations, the Contractor shall keep the premises clean, free of excess soils, and other materials, including refuse and debris, resulting from the Contractor's work. At the end of each work day, and as each area is completed, surrounding

walks and paved areas shall be cleaned to the satisfaction of the Project Engineer. At the conclusion of work, the Contractor shall remove surplus soils, materials, and debris from the site and shall leave the project in a condition acceptable to the Project Engineer.

The Contractor shall locate all underground utilities (both new and existing) prior to starting work and shall not disturb or damage them. The Contractor shall promptly notify the Project Engineer of any conflict between the proposed work and the obstructions. The Contractor shall be responsible for making any and all repairs for damage, at his own expense.

Lawn installation is anticipated to begin after other related work is complete. Lawn materials shall not be installed until weather permits and installation has been authorized by the Project Engineer.

8-02.3(2)A Chemical Herbicides

Section 8-02.3(2)A is supplemented as follows:

No chemical herbicides will be allowed in the seeded or sodded areas.

8-02.3(5) Planting Area Preparation

Section 8-02.3(5) is revised as follows:

Lawn Area Preparation

Upon approval of the subgrade, place Topsoil Type A as indicated and shown on the Plans.

8-02.4 Measurement

Section 8-02.4 is supplemented as follows:

Preparation for “Sodding” will be measured along the ground slope and computed in square feet of actual area completed, and accepted.

8-02.5 Payment

Section 8-02.5 is supplemented as follows:

The costs of removing all excess material and debris shall be considered **incidental** to and included in the unit prices of other items in this contract.

The contract bid prices above, including all **incidental** work, shall be full pay for all labor, material, tools and equipment necessary to satisfactorily complete the work as defined in the Standard Specifications, these Special Provisions and the Plans.

“Preparation for Sodding”, per square foot.

The unit contract price for “Preparation for Planting” shall be full pay for all labor, tools, and equipment for scarifying, fine grading, and raking at the locations shown in the Plans.

“Topsoil, Type A”, per cubic yard.

The unit contract price per cubic yard for “Topsoil, Type A”, including all **incidental** work, shall be full pay for all labor, material, tools, and equipment necessary to satisfactorily complete the work as defined in the Standard Specifications, these Special Provisions and the Plans.

Soil amendments shall be considered **incidental** to and included in the unit price of "Topsoil, Type A" and no additional compensation will be made.

8-04 CURBS, GUTTERS AND SPILLWAYS

8-04.4 Measurement

Section 8-04.4 is supplemented with the following:

No specific unit of measurement will apply to the curbs and gutters.

8-04.5 Payment

Section 8-04.5 is supplemented with the following:

All costs associated with installing curb for ADA ramp shall be considered **incidental** to and included in the unit contract for "ADA Curb Ramp".

The contract bid prices above, including all **incidental** work, shall be full pay for all labor, material, tools, and equipment necessary to satisfactorily complete the work as defined in the Standard Specifications and these Special Provisions

8-12 CHAIN LINK FENCE AND WIRE FENCE

This Section is revised to read: **CHAIN LINK FENCE, WIRE FENCE, AND SPLIT RAIL FENCE**

8-12.1 Description

The first paragraph of Section 8-12.1 is revised as follows:

This Work consists of furnishing and construction chain link fence, wire fence, and split rail fence of the types specified in accordance with the Plans, these Specifications, and the *Standard Plans* at the locations shown in the Plans and in conformity with the lines as staked.

The third paragraph is revised as follows:

Wire fence shall be of smooth wire or smooth wire combined with wire mesh fastened to posts. Steel posts and steel braces are to be used. White PVC markers as approved by the Project Engineer.

The section is supplemented with the following:

Split rail fence shall be 2-rail split rail fence with wood posts as shown on the Plans.

8-12.2 Materials

Section 8-12.2 is supplemented with the following:

Split Rail Fence:
All 6" x 6" posts and 4" x 4" rails are split Western Red Cedar as shown.

All hardware shall be galvanized, and of the types and sizes shown on the Plans.

8-12.3(1)D Chain Link Fabric

Section 8-12.3(1)D is supplemented with the following:

Chain link fabric shall be vinyl-coated dark brown as approved by the Project Engineer.

8-12.4 Measurement

Section 8-12.4 is supplemented with the following:

"Split Rail Fence" will be measured by the linear foot.

8-12.5 Payment

Section 8-12.5 is supplemented with the following:

"Smooth Wire Fence", per linear foot.
"Split Rail Fence", per linear foot.

8-14 CEMENT CONCRETE SIDEWALKS

8-14.3(4) Curing

Section 8-12.3(4) is supplemented as follows:

It shall be the Contractor's responsibility to watch over curing concrete until it is set to prevent vandalism. Any repairs needed to correct vandalism during the initial set period, including full replacement of the damaged panel, shall be at the expense of the Contractor and subject to approval of the Engineer.

8-14.4 Measurement

Section 8-14.4 is supplemented as follows:

No specific unit of measurement will apply to "ADA Curb Ramp"

No specific unit of measurement will apply to "View Point Concrete Sidewalk"

8-14.5 Payment

Section 8-14.5 is revised as follows:

"ADA Curb Ramp" will be paid on a lump sum basis.

"View Point Concrete Sidewalk" will be paid on a lump sum basis.

8-22 PAVEMENT MARKING

8-22.4 Measurement

Section 8-22.4 is revised as follows:

"Pavement Marking" will not be measured.

8-22.5 Payment

Section 8-22.5 is revised as follows:

"Pavement Marking" will be paid on a lump sum basis.

8-26 SITE FURNISHINGS (NEW SECTION)

8-26.1 Description

This work shall consist of furnishing and placing of site furnishings in accordance with the Plans and these Special Provisions.

8-26.2 Materials

8-26.2(1) Benches

The benches shall be 6' long anodized aluminum with backrest as detailed on the Plans and these Special Provisions.

8-26.3 Construction Requirements

Concrete pads shall be constructed and embedded in the concrete pads per the plans.

8-26.4 Measurement

"Bench", per each.

8-26.5 Payment

Payment will be made in accordance with Section 1-04.1 of the Standard Specifications, for each of the following bid items that are included in the proposal:

"Bench", per each.

The contract unit price per each "Bench" shall be full pay for furnishing and installing the benches and concrete pads as shown on the Plans, including, but not limited to, labor, materials, equipment, excavation, and backfill.

(******)

8.27 BOLLARDS (NEW SECTION)

Description

This work shall consist of constructing steel bollards.

Materials

Steel Assembly

All hardware shall be steel, conforming to the size and thickness shown.

All steel parts shall be hot-dip galvanized after fabrication.

Reflective Tape

Reflective tape, if required by the Engineer, shall be one of the following or an approved equal:

1. 3M 3810 Flexible High Intensity
2. Reflexite PC 100
3. 3M Diamond Grade
4. Stimeonite High Performance Grade

Concrete

Footings shall be constructed using concrete Class 3000.

Construction Requirements

Bollards shall be constructed as shown in Bollard Details as shown on the Plans.

Bollards shall not vary more than 1/2 inch in 30 inches from a vertical plane.

Bollard posts, and the exposed parts of the base assembly shall be painted with one coat of paint formula K-2-83 or as specified by the Engineer.

The Contractor shall make all necessary retrofits to the removable bollard to accommodate up to three (3) padlocks. Concrete footings shall be constructed for the bollards per the plans.

Measurement

Measurement for bollards will be by the unit for each bollard furnished and installed.

Payment

Payment will be made in accordance with Section 1-04.1, for the following bid items:

"Fixed Steel Bollard", per each
"Removable Steel Bollard", per each

8-28 STEEL CONTROL GATE (NEW SECTION)

Description

This work shall consist of constructing and installing one single-arm steel swing control gate.

Materials

Steel Assembly

All hardware shall be steel, conforming to the size and thickness shown.

All steel parts shall be painted after fabrication.

Reflective Tape

Reflective tape, if required by the Engineer, shall be one of the following or an approved equal:

1. 3M 3810 Flexible High Intensity
2. Reflexite PC 100
3. 3M Diamond Grade
4. Stimeonite High Performance Grade

Concrete
Footings shall be constructed using concrete Class 3000.

Paint
Paint shall be white marine-grade polyurethane enamel as approved by the Project Engineer.

Construction Requirements
Gate shall be constructed as shown on the Plans. Entire assembly shall be painted as specified.

Measurement
“Control Gate”, per each.

Payment
Payment will be made in accordance with Section 1-04.1, for the following bid items:

"Control Gate", per each

END OF DIVISION 8

DIVISION 9
MATERIALS

Asphalt Material, General

Section 9-02.1 is supplemented with the following:

(August 2, 2012)

The recycling agent used to rejuvenate the recovered asphalt from recycled asphalt pavement (RAP) and reclaimed asphalt shingles (RAS) shall meet the specifications in Table 1:

Table 1		**RA 1**		**RA 5**		**RA 25**	
Test	**ASTM Test Method**	**Min.**	**Max.**	**Min.**	**Max.**	**Min.**	**Max.**
Viscosity @ 140˚F cSt	D2170 or D2171	50	150	200	800	1000	4000
Flashpoint COC, ˚F	D92	400		400		400	
Saturates, Wt. %	D2007		30		30		30
Specific Gravity	D70 or D2198	Report		Report		Report	
Tests on Residue from RTFC	D2872						
Viscosity Ratio[1]			3		3		3
Mass Change ± %			4		4		4
[1]Viscosity Ratio = RTFC Viscosity @ 140˚F, cSt / Original Viscosity @ 140˚F, cSt							

HMA Test Requirements

The second paragraph in Section 9-03.8(2) is revised to read:

(November 12, 2012)

The mix design shall produce HMA mixtures when combined with RAP, RAS, coarse and fine aggregate within the limits set forth in Section 9-03.8(6) and mixed in the laboratory with the designated grade of asphalt binder, using the Superpave gyratory compactor in accordance with WSDOT FOP for AASHTO T 312, and at the required gyrations for N initial, N design, and N maximum with the following properties:

	HMA Class							
	⅜ inch		**½ inch**		**¾ inch**		**1 inch**	
Mix Criteria	**Min.**	**Max.**	**Min.**	**Max.**	**Min.**	**Max.**	**Min.**	**Max.**
Voids in Mineral Aggregate (VMA), %	15.0		14.0		13.0		12.0	
Voids Filled With Asphalt (VFA), %								
ESAL's (millions)	VFA							
< 0.3	70	80	70	80	70	80	67	80
0.3 to < 3	65	78	65	78	65	78	65	78

3 to < 10	73	76	65	75	65	75	65	75
10 to < 30	73	76	65	75	65	75	65	75
≥ 30	73	76	65	75	65	75	65	75
Dust/Asphalt Ratio	0.6	1.6	0.6	1.6	0.6	1.6	0.6	1.6
Stripping Evaluation, WSDOT Test Method T 718	Pass		Pass		Pass		Pass	
[1]Hamburg Wheel-Track Testing, AASHTO T 324 Rut Depth (mm) @ 20,000 Passes		12.5		12.5		12.5		12.5
[1]Texas Department of Transportation Indirect Tensile Strength Test, Tex-226 -F(psi)		150		150		150		150

	ESAL's (millions)	N initial	N design	N Max.
% Gmm	< 0.3	≤ 91.5	96.0	≤ 98.0
	0.3 to < 3	≤ 90.5	96.0	≤ 98.0
	≥ 3	≤ 89.0	96.0	≤ 98.0
Gyratory Compaction(number of gyrations)	< 0.3	6	50	75
	0.3 to < 3	7	75	115
	3 to < 30	8	100	160
	≥ 30	9	125	205

[1]This test applies only to mix designs with greater than 20 percent of the total weight RAP or any amount of RAS.

Gradation – Recycled Asphalt Pavement and Mineral Aggregate

Section 9-03.8(3)B is supplemented with the following:

(August 6, 2012)

For HMA with a RAP percentage greater than 20 percent of the total weight the RAP shall be processed to ensure that 100 percent of the material passes a sieve twice the size of the maximum aggregate size for the class of mix to be produced.

When RAS is used in the production of HMA the RAS shall be milled, crushed or processed to ensure that 100 percent of the material passes the ½ inch sieve. Extraneous materials in RAS such as metals, glass, rubber, soil, brick, tars, paper, wood and plastic shall not exceed 2.0 percent by mass as determined on material retained on the No. 4 sieve.

General Requirements

Section 9-03.21(1) is supplemented with the following:

(August 2, 2012)

Reclaimed asphalt shingles samples shall contain less than the maximum percentage of asbestos fibers based on testing procedures and frequencies established in conjunction with the specifying jurisdiction and state or federal environmental regulatory agencies.

9-28 SIGNING MATERIALS AND FABRICATION

9-28.1 General

Section 9-28.1 is modified as follows:

The second sentence of the first paragraph is deleted.

Appendices
(January 2, 2012)

The following appendices are attached and made a part of this contract:

APPENDIX A:
TESC Plan

APPENDIX B:
Permits and Regulations

APPENDIX C:
Summary of Geotechnical Conditions and Logs of Test Borings

APPENDIX D:
Federal Aid Provisions

APPENDIX E:
Federal Wage Rates
Washington State Wage Rates, Douglas County
Supplement to Wage Rates
Benefit Code Key

APPENDIX F:
Proposal for Bidding Purposes

(January 7, 2013)
Standard Plans

The State of Washington Standard Plans for Road, Bridge and Municipal Construction M21-01 transmitted under Publications Transmittal No. PT 11-036, effective January 7, 2013 is made a part of this contract.

The Standard Plans are revised as follows:

B-10.20 and B10.40
Substitute “step” in lieu of “handhold” on plan

B-90.40
Offset & Bend details, add the subtitle, “Plan View” above titles

C-5
Deleted

C-13
Deleted

C-13a
Deleted

C-13b
Deleted

C-13c
Deleted

C-14a
Deleted

C-14b
Deleted

C-14c
Deleted

C-14d
Deleted

C-14e
Deleted

C-15a
Deleted

C-15b
Deleted

C-28.40
Deleted

C-70.10-00
Elevation, and Barrier Connection Detail, callout for premolded joint filler, revise ¼” to 3/8”
Note 1, revise ¼” to 3/8”.

The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-75.10-00

Elevation, callout for premolded joint filler, revise ¼" to 3/8", Note 1, revise ¼" to 3/8".
The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-75.20-00

Elevation, callout for premolded joint filler, revise ¼" to 3/8", Note 1, revise ¼" to 3/8".
The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-75.30-00

Elevation, and Plan views, callout for premolded joint filler, revise ¼" to 3/8" ", Note 1, revise ¼" to 3/8".
The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-80.10-00

The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-80.20-00

The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-80.30-00

The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed,

for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

C-80.40-00
The Welded Wire Reinforcing Substitution Option Table is deleted. The note, "*Optional Substitutions to Welded Wire Reinforcements shall conform to Standard Specification Sections 6-10 and 9-07" is revised to read: "Steel Welded Wire Reinforcement Deformed, for Concrete may be substituted for reinforcing steel in accordance with Standard Specification 6-10.3."

D-3
Deleted

D-3.10
Key Note 7, reference to 1130.04(5).06 is revised to 730.05(5)

G-24.50
Detail B, callout, "Nylon Washer ~ When sign face has Type 3 or 4 sheeting" is revised to read; Nylon Washer ~ When sign face has Type 3, 4, 8 or 9 sheeting"

G-30.10
Sheet 2, "Sign Installation on Signal or Light Standard" detail, "7'-0" Min."(2x) dimension(s) revised to read 7'-0"

G-50.10
Sheet 2, Diamond-Shaped Sign detail, dimension, "More than 36" is revised to read; More than 30"

G-60.20
Side View, callout, "Anchor Rod ~ 1-3/4" Diam. x 4'-4" Threaded 8" Min. Each End; W/ 2 Washers & 4 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1'-0" Min." is revised to read; "Anchor Rod ~ 1-3/4" Diam. x 4'-4" Threaded 8" Min. Each End; W/ 2 Washers & 6 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1'-0" Min."

G-60.30
End View, callout, "Anchor Rod ~ 1-3/4" Diam. x 4'-4" Threaded 8" Min. Each End; W/ 2 Washers & 4 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1'-0" Min." is revised to read; "Anchor Rod ~ 1-3/4" Diam. x 4'-4" Threaded 8" Min. Each End; W/ 2 Washers & 6 Heavy Hex Nuts ~ Galvanize Exposed Anchor Rod End for 1'-0" Min."

I-60.10
Deleted

I-60.20
Deleted

J-1f
Deleted

J-3b
Sheet 2 of 2, Plan View of Service Cabinet, Boxed Note, “SEE STANDARD PLAN J-6C…” is revised to read: “SEE STANDARD PLAN J-10.10…”
Sheet 2 of 2, Plan View of Service Cabinet Notes, references to Std. Plan J-9a are revised to J-60.05 (3 instances).

J-7c
Deleted

J-12
Deleted

J-15.10
Elevation View (3x), Depth dimension, reads; ”Depth ~ See Std. Spec. 9-20.3(14)E and Contract”, revised to read; ”Depth ~ See Std. Spec. 8-20.3(13)A and Contract”

J-16b
Key Note 1, reference to J-16a is revised to J-40.36

J-16c
Key Note 1, reference to J-16a is revised to J-40.36

J-20.10
Detail A, add callout, ¾” Thick Grout (Four sides)

J-20.11
Section B, add callout, ¾” Thick Grout (Four sides)

J-40.30
Section A, dimension, “18” Min. from top of soil surface” is revised to read; 24” Min. from top of soil surface. Callout, “Gravel Pad” is revised to read; Crushed Surfacing ~ Per Standard Spec. Section 9-03.9(3)

J-50.16
Deleted

J-75.40
Monotube Sign Structure, elevation, callout – EQUIPMENT GROUNDING CONDUCTOR ~ SIZE PER NEC. MINIMUM SIZE # 8
Is revised to read; EQUIPMENT GROUNDING CONDUCTOR ~ SIZE PER NEC minimum size # 4 AWG
Detail C, callout– EQUIPMENT GROUNDING CONDUCTOR ~ CLAMP TO STEEL REINFORCING BAR, SIZE PER NEC MIN. SIZE # 8
Is revised to read; EQUIPMENT GROUNDING CONDUCTOR ~ CLAMP TO STEEL REINFORCING BAR, SIZE PER NEC minimum size # 4 AWG

J-75.45
elevation, callout – EQUIPMENT GROUNDING CONDUCTOR ~ SIZE PER NEC. MINIMUM SIZE # 8

Is revised to read:

EQUIPMENT GROUNDING CONDUCTOR ~ SIZE PER NEC minimum size # 4 AWG

Detail D, callout– EQUIPMENT GROUNDING CONDUCTOR ~ CLAMP TO STEEL REINFORCING BAR, SIZE PER NEC. MIN. SIZE # 8

Is revised to read:

J-90.10
Section B, callout, “Hardware Mounting Rack ~ S. S. 1-5/8” Slotted Channel” is revised to read: “Hardware Mounting Rack (Typ.) ~ Type 304 S. S. 1-5/8” Slotted Channel”

J-90.20
Section B, callout, “Hardware Mounting Rack (Typ.) ~ S. S. 1-5/8” Slotted Channel” is revised to read: “Hardware Mounting Rack (Typ.) ~ Type 304 S. S. 1-5/8” Slotted Channel”

EQUIPMENT GROUNDING CONDUCTOR ~ CLAMP TO STEEL REINFORCING BAR, SIZE PER NEC minimum size # 4 AWG

K-80.30
In the NARROW BASE, END view, the reference to Std. Plan C-8e is revised to Std. Plan K-80.35

The following are the Standard Plan numbers applicable at the time this project was advertised. The date shown with each plan number is the publication approval date shown in the lower right-hand corner of that plan. Standard Plans showing different dates shall not be used in this contract.

A-10.10-00.....8/7/07
A-10.20-00.....10/5/07
A-10.30-00.....10/5/07
A-20.10-00.....8/31/07
A-30.10-00.....11/8/07
A-30.15-00.....11/8/07
A-30.30-01.....6/16/11
A-30.35-00.....10/12/07
A-40.00-00.....8/11/09
A-40.10-02.....6/2/11
A-40.15-00.....8/11/09
A-40.20-01.....2/7/12
A-40.50-01.....6/2/11
A-50.10-00…....11/17/08
A-50.20-01…....9/22/09
A-50.30-00…....11/17/08
A-50.40-00…....11/17/08
A-60.10-01.....10/14/09
A-60.20-02.....6/2/11
A-60.30-00.....11/8/07
A-60.40-00.....8/31/07

B-5.20-01.....6/16/11
B-5.40-01.....6/16/11
B-5.60-01.....6/16/11
B-10.20-01.....2/7/12
B-10.40-00.....6/1/06
B-10.60-00.....6/8/06
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7 **END OF DIVISION 9**

WASHINGTON STATE PARKS & RECREATION COMMISSION

APPROVED FOR CONSTRUCTION

FIELD OPERATIONS MANAGER date

CAPITAL PROGRAM MANAGER date

Park Manager: GEORGE EIDSON

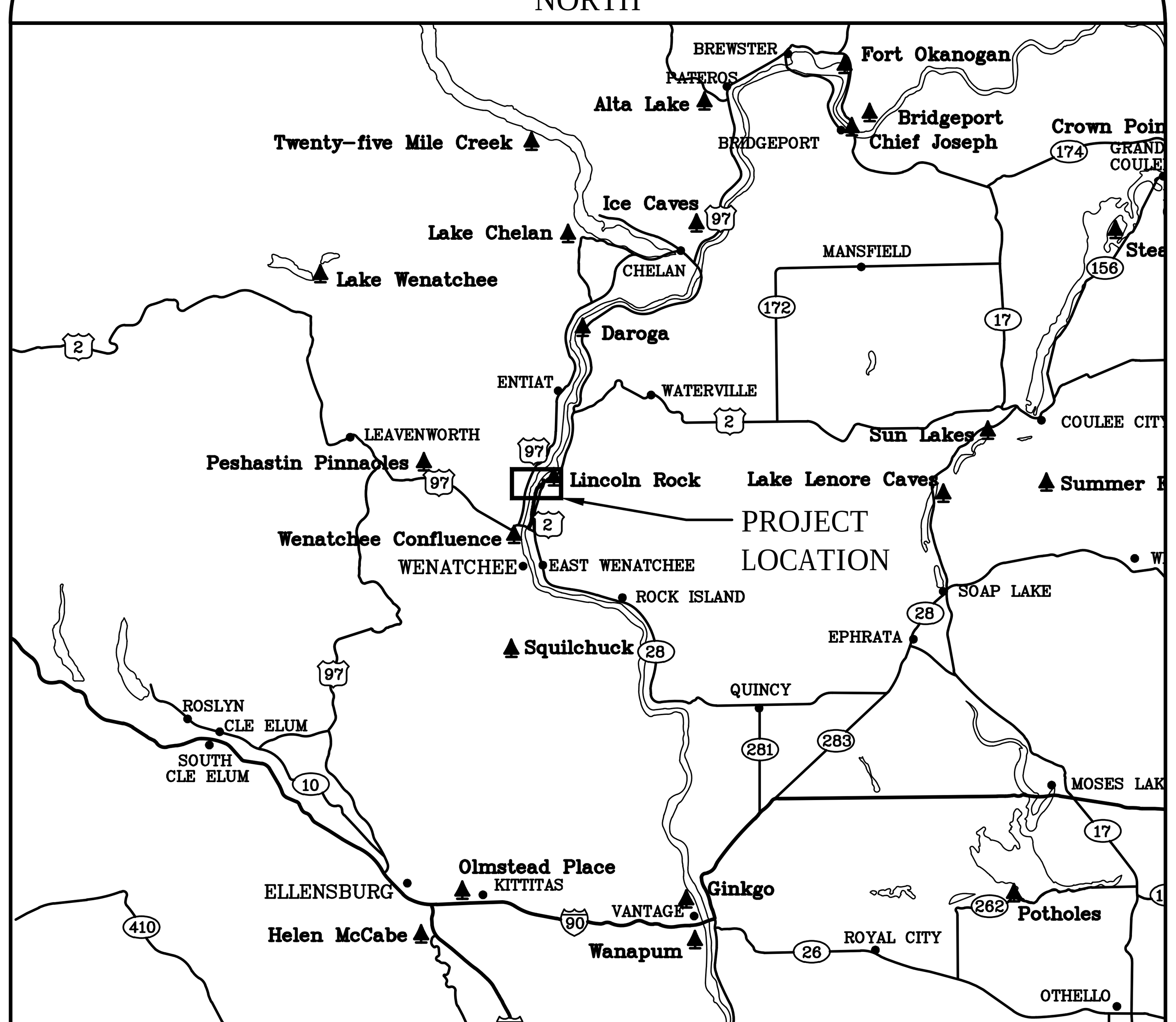
ROCKY REACH TRAIL PHASE ONE

JANUARY 2013

FEDERAL AID NO. STPE - 2009 (020) CONTRACT NO. TA-????

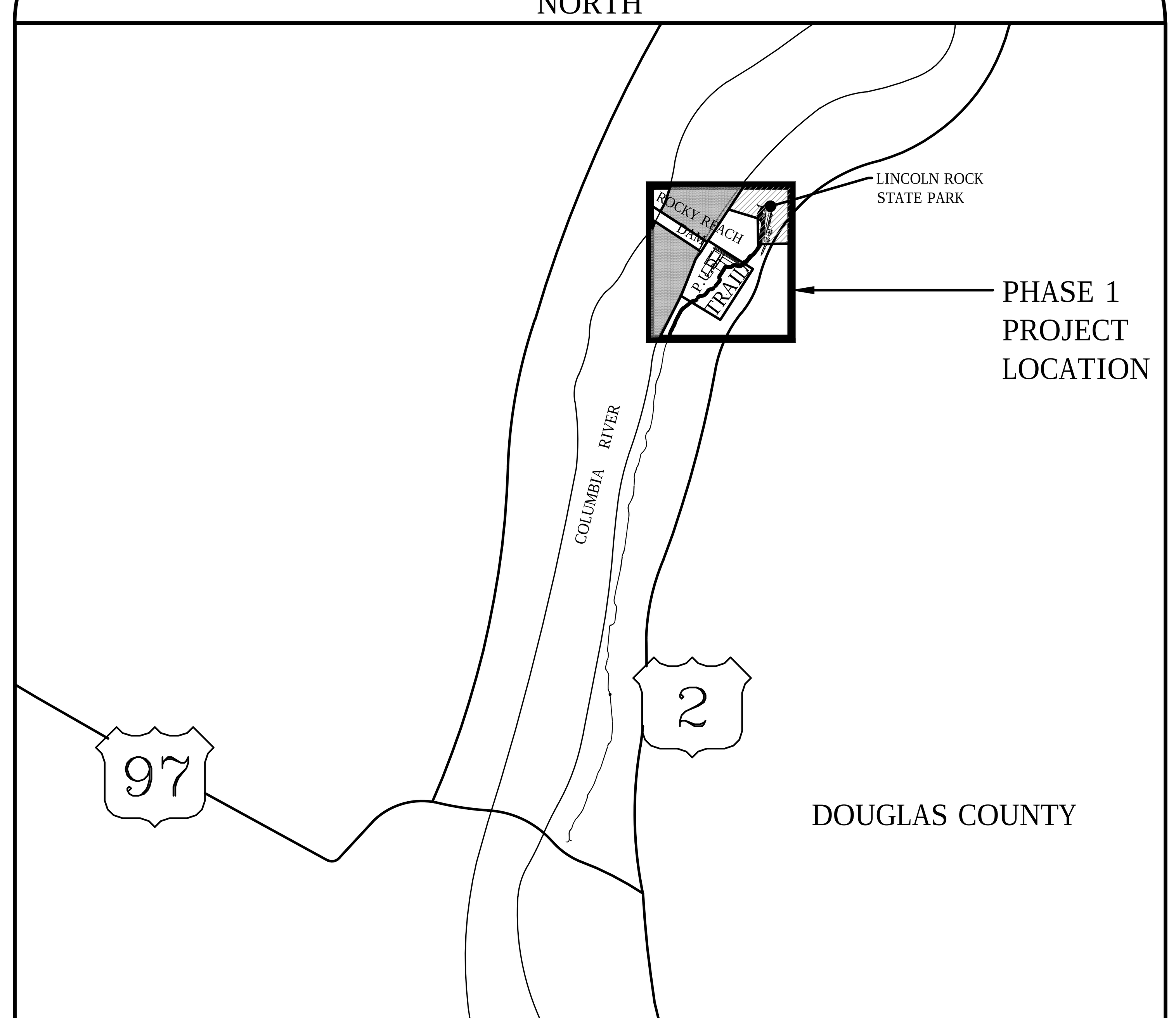
VICINITY MAP

NORTH



PROJECT LOCATION

NORTH



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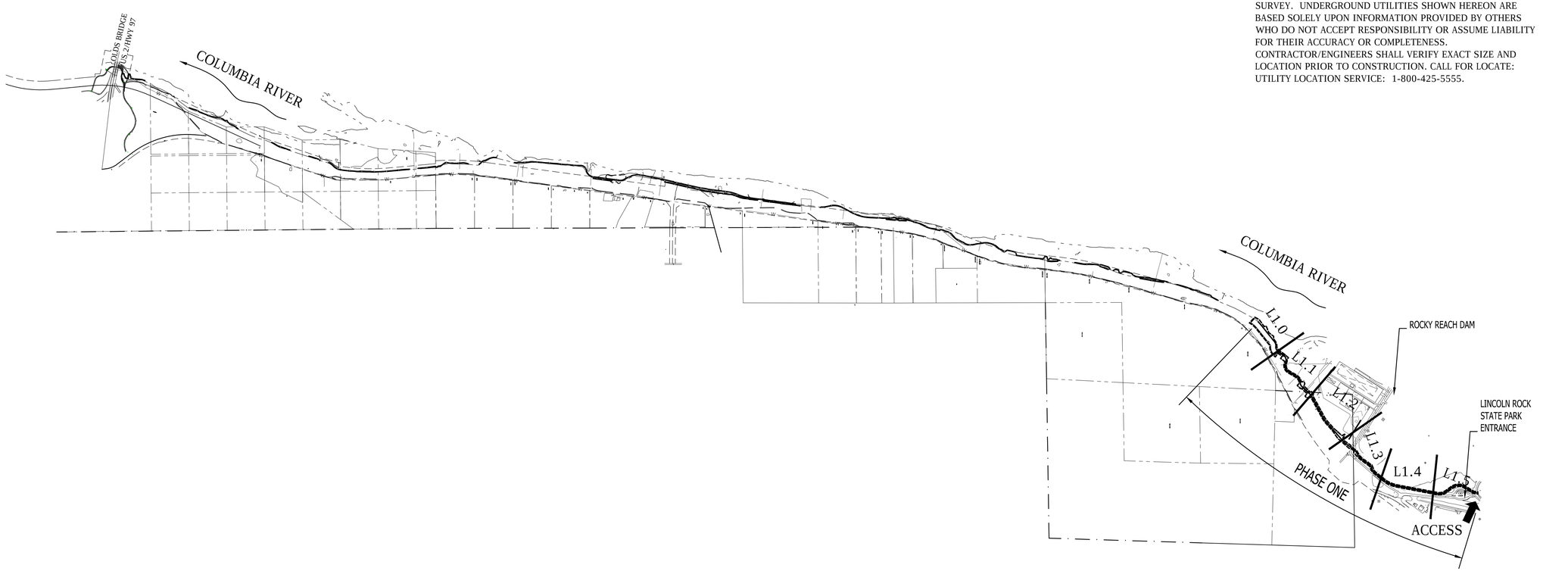
SURVEY CONTROL POINTS

PT NO.	NORTHING	EASTING	ELEVATION	DESCRIPTION
5	1775522.0500	193670.2428	736.2000	DC 614
11	1770564.6620	172122.2349	749.9300	DC625
33	1768976.7814	172357.1052	636.4300	HV101
34	1769741.2948	174136.1713	640.0900	HV102
35	1770226.4770	174423.6515	665.6700	237+64RT
36	1770484.3365	175192.4764	658.1800	245+75RT
37	1770618.4102	175593.1526	657.9700	249+98RT
38	1770673.6638	175757.8003	655.3400	251+72RT
39	1770724.8678	176112.4079	647.7200	255+11RT
40	1770521.0726	176111.2077	638.2800	254+69LT
41	1770460.3879	176110.9524	634.4000	254+55LT
48	1771286.1412	183013.8464	644.6100	REB.330+43.49.RT
50	1771167.0628	182118.0213	647.5500	REB321+39.58.RT
51	1771092.6698	181558.4655	649.4900	REB315+75.24RT
52	1771021.3675	181022.2854	648.3100	REB310+91.82RT
53	1776555.5543	195811.3415	750.4100	PK.HV301
57	1774448.3592	194387.0449	741.8000	PK.HV109
59	1769975.9382	177783.8439	627.5600	1IN.REBAR-HV.355
70	1770752.0326	176888.0867	647.7700	REB.262+69.35RT
71	1770721.9470	177234.0853	644.8800	REB.EQUA..STA
72	1770687.2015	177607.2843	645.5100	REB.275+69.69.RT
73	1770668.4696	177829.1021	647.6900	REB.277+94.69RT
74	1770421.4557	177777.9936	641.6500	REB.277+61.67LT
75	1770471.6977	177778.1605	641.0200	REB.277+58.16LT
76	1770544.8091	176983.8533	640.5400	REB.263+75LT
77	1770522.8583	177215.5542	640.0300	REB.EQUA.STA
78	1770487.9770	177588.4797	642.2000	REB.275+69.69LT
79	1770733.1786	177117.6694	646.1400	REB.264+90.87.RT
80	1770552.6009	176875.3320	642.0800	REB.262+66.21LT
81	1770707.9611	178668.3220	647.9500	REB.EQUA..STA
82	1770680.6394	178447.0043	649.2300	REB.284+31.91.RT
83	1770679.7824	178439.5878	649.2500	RC.H.DENNIS
84	1770419.2381	177810.9173	640.9600	REB.277+94.69LT
85	1771399.4678	183865.1044	645.4900	REB.339+02.15.RT
86	1771511.8301	184709.1494	649.3800	REB.347+53.6.RT
87	1771678.9357	185948.2620	653.7800	REB.360+06.96RT
88	1771749.1105	186281.7128	654.9400	REB.363+56.96RT
89	1771928.8030	186814.7575	659.3000	FND.REB.
90	1772145.1678	187379.7938	648.1100	REB.SC375+31.9RT
91	1772213.3221	187615.5937	648.4900	FND.REB
92	1775274.9403	193762.9805	731.8800	X.PIPE.MON.2
93	1775278.2276	193546.7539	724.6100	4IN.BC.MON.3
94	1775405.4351	193158.2169	725.7500	RC.ERLANDSEN.RW
95	1775266.8218	193061.0886	723.3400	REB..RW
96	1774936.3968	193077.6669	728.0200	4IN.BC.MON4
97	1774871.6569	192808.7785	717.6600	REB.RW
98	1775640.3128	193344.1787	732.7100	REB.RW
501	1774384.6607	192908.3331	703.7700	REB.MON.8
502	1774257.7545	192908.6240	710.0800	BC.MON.9
503	1774010.6660	192659.7096	709.8600	R*.FND
504	1774305.3056	192517.3015	717.0000	REB.435+00.RT
505	1774003.3588	192336.9799	715.0700	REB.431+50.RT
506	1773998.5587	192334.7339	715.0500	REB.EQUA.STA
507	1773629.1810	192435.8829	708.0100	R*.FND
508	1773362.8597	191879.9414	705.1900	REB.423+00.RT
511	1771584.7296	183117.7113	647.6000	RC*.FND
512	1769307.6149	171157.2378	691.4300	AC.MON.20
516	1771647.5142	185728.4789	652.2300	REB.357+81.96.RT
517	1771621.6006	185533.1028	651.8200	RC.FORSGREN
519	1771858.8645	186604.1776	662.8700	REB.367+06.96.RT
523	1769867.2130	174237.3251	637.6100	AC.MON.21.PUD
525	1771624.9588	186313.7175	645.5400	R*.FND
526	1770535.6084	178149.1628	644.0600	R*.FND
527	1770622.4295	177224.8878	641.2900	R*.FND
528	1770531.1844	175805.2569	645.6500	R*.FND
529	1769951.5755	174076.4554	658.0200	R*.FND
530	1772287.7099	189013.8851	645.0400	R*.FND
531	1770706.5768	176327.1555	644.6400	R*.FND
532	1773674.0322	192707.5729	701.6900	MON 11 BC CONC.

NOTE: STATION AND OFFSET DESCRIPTION REFERS TO CENTERLINE OF RIGHT OF WAY

SURVEY NOTES

1. HORIZONTAL DATUM = NAD 83(91).
2. BASIS OF BEARINGS = Washington State Plane North Zone
3. VERTICAL DATUM = NAVD 88
4. ALL DISTANCES SHOWN ON THIS PLAT ARE GRID DISTANCES. MULTIPLY BY A COMBINED SCALE FACTOR OF 1.00005850 TO DERIVE GROUND DISTANCES.
5. THE LOCATION AND DESCRIPTION OF ALL SURVEY MARKERS SHOWN HEREON ARE BASED ON FIELD OBSERVATIONS TAKEN ON 04/11/97, THROUGH 05/13/97, UNLESS OTHERWISE INDICATED.
6. WORK PERFORMED IN CONJUNCTION WITH THIS SURVEY UTILIZED THE FOLLOWING EQUIPMENT AND PROCEDURES: TC800
7. THIS SURVEY WAS PERFORMED WITHOUT THE BENEFIT OF A TITLE REPORT AND DOES NOT PURPORT TO SHOW ALL EASEMENTS.
8. THIS TOPOGRAPHIC SURVEY DRAWING ACCURATELY PRESENTS SURFACE FEATURES LOCATED DURING THE COURSE OF THIS SURVEY. UNDERGROUND UTILITIES SHOWN HEREON ARE BASED SOLELY UPON INFORMATION PROVIDED BY OTHERS WHO DO NOT ACCEPT RESPONSIBILITY OR ASSUME LIABILITY FOR THEIR ACCURACY OR COMPLETENESS. CONTRACTOR/ENGINEERS SHALL VERIFY EXACT SIZE AND LOCATION PRIOR TO CONSTRUCTION. CALL FOR LOCATE: UTILITY LOCATION SERVICE: 1-800-425-5555.



NOTE: SHEET NUMBERING REFERS TO PLAN & PROFILE DRAWINGS

20130226-5138 FERC PDF (Unofficial) 2/26/2013 3:46:43 PM

CAD NO. 10103-K1 PH1.dwg

NO.	REVISIONS	INT.	APP.	DATE

ACTION	BY	DATE
DESIGNED	TR	08/01/2012
DRAWN	MBM	08/01/2012
CHECKED (FIELD)		
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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

KEY PLAN

K1.0

SCALE: 1" =1000'

SUMMARY OF QUANTITIES

ITEM NO.	STD. ITEM NO.	TOTAL QUANTITY	UNIT	ITEM
				PREPARATION
1	0001	1	L.S.	MOBILIZATION
2	0030	3.65	ACRE	CLEARING AND GRUBBING
3	0050	1	L.S.	REMOVAL OF STRUCTURE AND OBSTRUCTION
4		2.07	ACRE	REMOVAL OF ORCHARD TREES
5	7038	1	L.S.	SURVEY AND STAKING
				GRADING
6	0310	1985	C.Y.	EXCAVATION AND EMBANKMENT ON SITE
7	0409	4922	C.Y.	GRAVEL BORROW INCL. HAUL
8	0470	10,670	S.Y.	SUBGRADE COMPACTION
				DRAINAGE / UTILITIES
9	1074	12	C.Y.	LIGHT LOOSE RIP RAP
10	7715	1	F.A.	IRRIGATION CARRIER AND CASING PIPE
				STRUCTURES
11	4006	750	L.S.	STRUCTURE EXCAVATION CLASS A INCL. HAUL
12	4013	1	L.S.	SHORING OR EXTRA EXCAVATION CL. A
13	1160	190	L.F.	UNDERDRAIN PIPE 6 IN. DIAMETER
14	7014	6	C.Y.	GRAVEL BACKFILL FOR DRAIN
15	4025	161	C.Y.	GRAVEL BACKFILL FOR WALL
16	4149	25,000	L.B.	ST.REINF. BAR FOR BRIDGE
17	4322	165	C.Y.	CONC. CLASS 4000 FOR BRIDGE
18	4058	784	L.F.	18 IN. DIAMETER AUGER CAST PILES
19	4412	120	L.F.	METAL FABRIC RAILING
20	-	1	L.S.	PREFABRICATED STEEL BRIDGE - SOUTH
21	-	1	L.S.	PREFABRICATED STEEL BRIDGE - NORTH
				SURFACING
22	5115	892	C.Y.	CRUSHED SURFACING TOP COURSE (CSTC)
23	5057	4850	C.Y.	GRAVEL BORROW
24	6405	190	C.Y.	TOPSOIL
				PAVING
25	5767	925	TON	COMMERCIAL HMA
26	5625	11.7	C.Y.	C.I.P. CONCRETE
				EROSION CONTROL
28	6403		DAY	ESC LEAD
29	6468	200	S.Y.	STABILIZED CONSTRUCTION ENTRANCE
30	6373	1,330	L.F.	SILT FENCE
31	6630	25	L.F.	HIGH VISIBILITY FENCE
32	6490	1	L.S.	EROSION & SEDIMENT CONTROL
33	6489	1	L.S.	STORMWATER POLLUTION PREVENTION PLAN
				PLANTING
34	6414	1.87	ACRE	HYDROSEEDING
35	6555	500	S.F.	SOD LAWN
				OTHER ITEMS
36		435	L.F.	SMOOOTH WIRE FENCE
37		39	L.F.	SPLIT RAIL FENCE
38	7041	6	EACH	BOLLARD TYPE 1 (FIXED)
39	7041	3	EACH	BOLLARD TYPE 2 (REMOVABLE)
40	7145	1	EACH	CONTROL GATE
41	6890	1	L.S.	SIGNING (REGULATORY/WARNING)
42		3	EACH	SIGNING (INTERPRETIVE) PANEL.S. FOIC
43	7058	1	L.S.	ADA CURB RAMP
44	7045	60	S.F.	DETECTABLE WARNING SURFACE
45	6856	1	L.S.	PAVEMENT MARKING
46	6806	96	L.F.	TRAIL (BOLLARD) STRIPING - YELLOW
47	7736	1	L.S.	SPCC PLAN
48	7715	$5,000	CALC.	IRRIGATION REPAIR
49		5	EACH	BENCH

CAD NO. 10103-K SHEETS-PH1.dwg

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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

SUMMARY OF QUANTITIES

K2.0

SCALE: NONE

SIGN SCHEDULE

Note: Provide 1 sign unless noted. Stationing is approximate and subject to change, stake location for approval. Height and setback of street/driveway signs as approved.

	SHEET NO.	STATION		SIGN	SIGN DESCRIPTION	SIZE IN INCHES	NOTES
1	L1.0	309+00	LEFT	R2-1	15 MPH	12 x 18	
2	L1.0	310+00	LEFT	Interp. Sign	TBD	34 X 20	DOUBLE LEG SLOPED PANEL SIGNS (3)
3	L1.1	310+20	LEFT	MILE MARKER (MM) + R2-1	1.0 miles & 15 MPH	8 X 16 & 12 X 18	
4	L1.1	313+75	RIGHT	R2-1	15 MPH	12 X 18	
5	L1.2	314+50	LEFT	W7-5	HILL	18 X 18	
6	L1.2	314+90	LEFT & RIGHT	OM3-L & OM3-R	OBJECT MARKERS	6 X 18	VERTICAL
7	L1.2	315+90	LEFT & RIGHT	OM3-L & OM3-R	OBJECT MARKERS	6 X 18	VERTICAL
8	L1.1	316+10	RIGHT	CUSTOM	"TEMPORARY TRAIL CLOSURE..."	18 X 24	FURNISHED BY OWNER. MOUNT ON CONTROL GATE
9	L1.1	322+00	RIGHT	CUSTOM	"NOTICE TO ROCKY REACH TRAIL USERS: YOU ARE ENTERING AN AGRICULTURAL USE AREA..."	24 X 24	FURNISHED BY OWNER.
10	L1.2	322+95	LEFT & RIGHT	OM3-L & OM3-R	OBJECT MARKERS	6 X 18	VERTICAL
11	L1.2	324+45	LEFT & RIGHT	OM3-L & OM3-R	OBJECT MARKERS	6 X 18	VERTICAL
12	L1.2	335+00	RIGHT	R2-1	15 MPH	12 x 18	
13	L1.3	336+60	LEFT	MILE MARKER (MM) + CUSTOM	0.5 miles + "STAY ON TRAIL"	8 X 16 & 12 X 18	
14	L1.3	336+40	RIGHT	R1-2 & R5-9	YIELD & NO MOTOR VEHICLES	18 X 18 X 18 & 24 X 24	
15	L1.3	336+85	OFFSET 50' LEFT	W11-15 & W11-15P	TRAIL CROSSING	18 X 18	FIELD LOCATE APPROX. 50' FROM CROSSING
16	L1.3	337+15	OFFSET 50' RIGHT	W11-15 & W11-15P	TRAIL CROSSING	18 X 18	FIELD LOCATE APPROX. 50' FROM CROSSING
17	L1.3	337+60	LEFT	R1-2 & R5-9	YIELD & NO MOTOR VEHICLES	18 X 18 X 18 & 24 X 24	
18	L1.3	341+00	RIGHT	W7-5	HILL	18 X 18	
19	L1.5	359+30	LEFT	R2-1	15 MPH	12 X 18	
20	L1.5	361+24	LEFT	MILE MARKER (MM)	0 miles	8 X 16	
21	L1.5	361+36	RIGHT	W2-1 & R5-3	INTERSECTION WARNING & NO MOTOR VEHICLES	18 X 18 & 24 X 24	MOUNT R5-3 ON BACK SIDE OF POST
22	L1.5	361+67	RIGHT	CUSTOM	"WELCOME TO LINCOLN ROCK STATE PARK"	24 X 24	FURNISHED BY OWNER
23	L1.5	361+94	OFFSET 46' LEFT	R1-2	YIELD	18 X 18 X 18	
24	L1.5	362+45	OFFSET 37' NORTH	R1-2	YIELD	18 X 18 X 18	

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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

SIGN SCHEDULE

K3.0

SCALE: NONE

RRT PH1

NUMBER	STARTING STATION	ENDING STATION	BEARING	DISTANCE	RADIUS	ARC LENGTH	DELTA	CHORD	START NORTHING	START EASTING	END NORTHING	END EASTING
C1	308+50.00	309+16.83			500.000	66.83	7°39'28"	N47°26'12"E, 66.78	192110.2348	1773228.9066	192155.4037	1773278.0903
L1	309+16.83	312+86.76	N51°15'56"E	369.929					192155.4037	1773278.0903	192386.8732	1773566.6551
C2	312+86.76	313+84.86			140.000	98.11	40°09'03"	N71°20'27"E, 96.11	192386.8732	1773566.6551	192417.6229	1773657.7151
L2	313+84.86	313+88.56	S88°35'02"E	3.699					192417.6229	1773657.7151	192417.5314	1773661.4131
C3	313+88.56	314+84.96			100.000	96.40	55°13'59"	N63°47'59"E, 92.71	192417.5314	1773661.4131	192458.4638	1773744.5978
L3	314+84.96	316+02.42	N36°11'00"E	117.457					192458.4638	1773744.5978	192553.2668	1773813.9407
C4	316+02.42	317+19.39			100.000	116.97	67°01'13"	N69°41'37"E, 110.42	192553.2668	1773813.9407	192591.5861	1773917.4952
L4	317+19.39	317+68.29	S76°47'47"E	48.902					192591.5861	1773917.4952	192580.4162	1773965.1048
C5	317+68.29	319+20.09			120.000	151.79	72°28'36"	N66°57'55"E, 141.88	192580.4162	1773965.1048	192635.9304	1774095.6679
L5	319+20.09	320+82.46	N30°43'37"E	162.374					192635.9304	1774095.6679	192775.5089	1774178.6320
C6	320+82.46	322+18.75			140.000	136.29	55°46'42"	N58°36'58"E, 130.97	192775.5089	1774178.6320	192843.7160	1774290.4435
C7	322+18.75	322+93.65			100.000	74.90	42°54'46"	N65°02'56"E, 73.16	192843.7160	1774290.4435	192874.5776	1774356.7738
L6	322+93.65	324+89.07	N43°35'33"E	195.416					192874.5776	1774356.7738	193016.1102	1774491.5181
C8	324+89.07	325+20.15			100.000	31.09	17°48'41"	N52°29'53"E, 30.96	193016.1102	1774491.5181	193034.9593	1774516.0811
L7	325+20.15	329+55.61	N61°24'14"E	435.457					193034.9593	1774516.0811	193243.3830	1774898.4190
C9	329+55.61	330+44.49			400.000	88.88	12°43'51"	N55°02'18"E, 88.70	193243.3830	1774898.4190	193294.2081	1774971.1084
L8	330+44.49	334+81.12	N48°40'23"E	436.629					193294.2081	1774971.1084	193582.5389	1775298.9965
C10	334+81.12	335+63.72			400.000	82.60	11°49'53"	N42°45'26"E, 82.45	193582.5389	1775298.9965	193643.0786	1775354.9730
L9	335+63.72	337+23.51	N36°50'29"E	159.796					193643.0786	1775354.9730	193770.9629	1775450.7873
C11	337+23.51	338+34.36			240.000	110.85	26°27'45"	N50°04'22"E, 109.86	193770.9629	1775450.7873	193841.4748	1775535.0372
L10	338+34.36	338+39.24	N63°18'15"E	4.879					193841.4748	1775535.0372	193843.6665	1775539.3958
C12	338+39.24	339+57.97			240.000	118.73	28°20'39"	N49°07'55"E, 117.52	193843.6665	1775539.3958	193920.5628	1775628.2673
L11	339+57.97	341+08.75	N34°57'35"E	150.784					193920.5628	1775628.2673	194044.1380	1775714.6666
C13	341+08.75	341+74.65			100.000	65.90	37°45'21"	N53°50'16"E, 64.71	194044.1380	1775714.6666	194082.3221	1775766.9109
L12	341+74.65	341+77.70	N72°42'57"E	3.056					194082.3221	1775766.9109	194083.2300	1775769.8288
C14	341+77.70	342+90.44			190.000	112.73	33°59'43"	N55°43'05"E, 111.09	194083.2300	1775769.8288	194145.8010	1775861.6168
L13	342+90.44	344+81.56	N38°43'14"E	191.123					194145.8010	1775861.6168	194294.9164	1775981.1685
C15	344+81.56	347+35.62			520.000	254.06	27°59'37"	N24°43'25"E, 251.54	194294.9164	1775981.1685	194523.4021	1776086.3745
L14	347+35.62	354+54.65	N10°43'37"E	719.033					194523.4021	1776086.3745	195229.8708	1776220.2058
C16	354+54.65	356+17.93			200.000	163.28	46°46'34"	N12°39'40"W, 158.78	195229.8708	1776220.2058	195384.7921	1776185.4030
L15	356+17.93	357+41.02	N36°02'57"W	123.088					195384.7921	1776185.4030	195484.3098	1776112.9685
C17	357+41.02	359+76.73			200.000	235.70	67°31'28"	N2°17'13"W, 222.30	195484.3098	1776112.9685	195706.4314	1776104.0975
L16	359+76.73	361+41.56	N31°28'30"E	164.832					195706.4314	1776104.0975	195847.0112	1776190.1610
C18	361+41.56	361+85.52			95.000	43.96	26°30'43"	N18°13'09"E, 43.57	195847.0112	1776190.1610	195888.3945	1776203.7824
L17	361+85.52	361+88.78	N4°57'47"E	3.263					195888.3945	1776203.7824	195891.6450	1776204.0647

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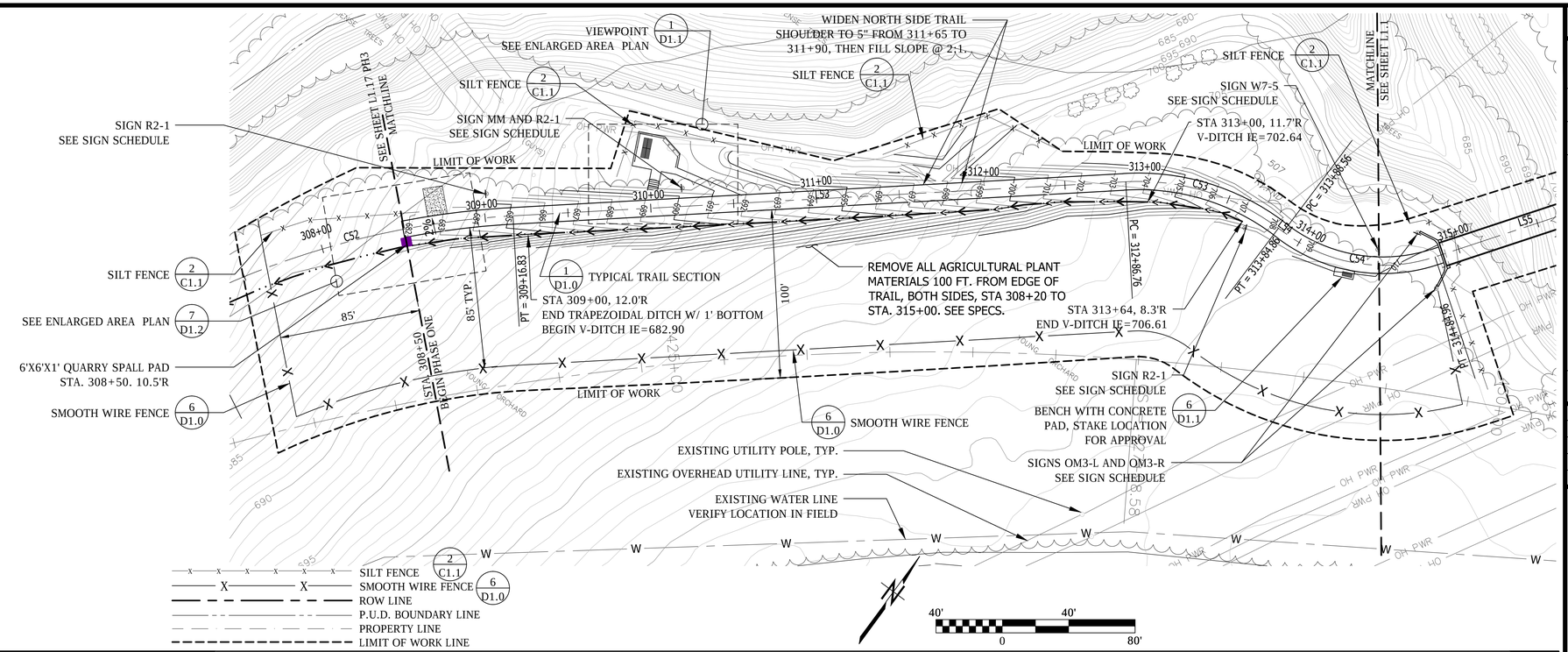
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

TRAIL COORDINATES

K4.0

SCALE: NONE



Call before you Dig.

1-800-424-5555
UNDERGROUND SERVICE (USA)

CAD NO. 10103-PLAN PROFILE-PH1.dwg

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RONALD L. LEINKUHLER
STATE OF WASHINGTON
24088
REGISTERED
PROFESSIONAL ENGINEER

WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

GRADING PLAN & PROFILE, TESC & DRAINAGE PLANS

L1.0

SCALE: H=1:40 V=1:4

SIGN OM3-L AND OM3-R
SEE SIGN SCHEDULE

2 D1.3 CONTROL GATE & SIGN, SEE SIGN SCHEDULE

SIGNS OM3-L AND OM3-R
SEE SIGN SCHEDULE

SIGN OM3-L
SEE SIGN SCHEDULE

6 D1.0 SMOOTH WIRE FENCE (TYP.)

2 C1.1 SILT FENCE

2' C1.1 SILT FENCE

P.U.D. BOUNDARY

SOUTH STEEL TRUSS BRIDGE
SEE "S" SHEETS

REMOVE ALL AGRICULTURAL PLANT MATERIALS 100 FT. FROM EDGE OF TRAIL, BOTH SIDES, STA 308+50 TO STA. 315+00.
SEE SPECS.
ORCHARD TREES NORTH OF STA 315+00 HAVE BEEN REMOVED

EXISTING UTILITY POLE, TYP.

EXISTING OVERHEAD UTILITY LINE, TYP.

PROTECT EXISTING WATER LINES
SEE C1.3 AND SPECS.

SEE SHEET L1.0 MATCHLINE

WSDOT ROW

LIMIT OF WORK

TYPICAL TRAIL SECTION 1 D1.0

SEEDLINGS

PROTECT EXISTING WATER LINES
SEE C1.3 AND SPECS.

SIGN "CUSTOM"
SEE SIGN SCHEDULE

SIGNS OM3-L AND OM3-R
SEE SIGN SCHEDULE

2 C1.1 SILT FENCE

NORTH STEEL TRUSS BRIDGE
SEE "S" SHEETS

PROTECT EXISTING IRRIGATION MAIN LINE DURING CONSTRUCTION

BRIDGE STAGING AREA (TYP.)

MATCHLINE SEE SHEET L1.2

Call before you Dig.
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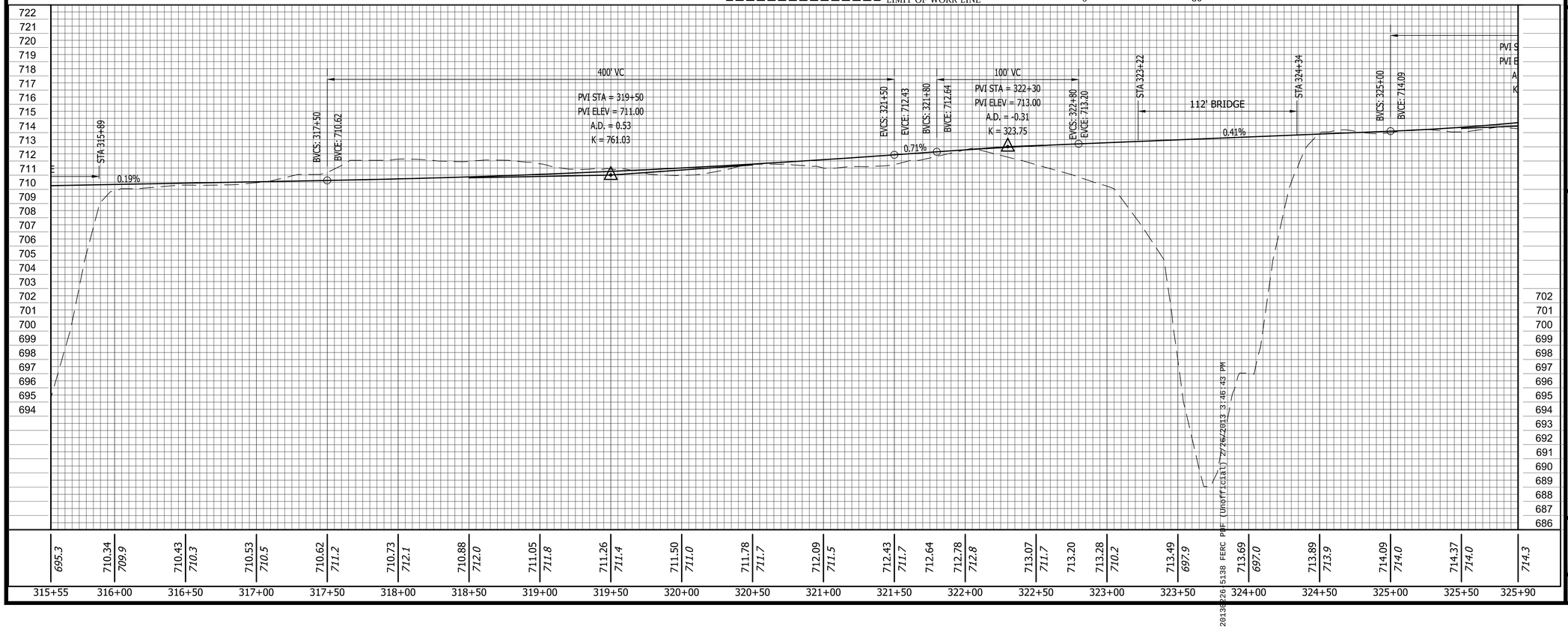
UNDERGROUND SERVICE (USA)

W EXISTING WATERLINE
SILT FENCE
ROW LINE
P.U.D. BOUNDARY LINE
PROPERTY LINE
LIMIT OF WORK LINE

40' 0 40' 80'



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ROCKY REACH TRAIL PHASE ONE

GRADING PLAN & PROFILE, TESC & DRAINAGE PLANS

L1.1

SCALE: H=1:40 V=1:4

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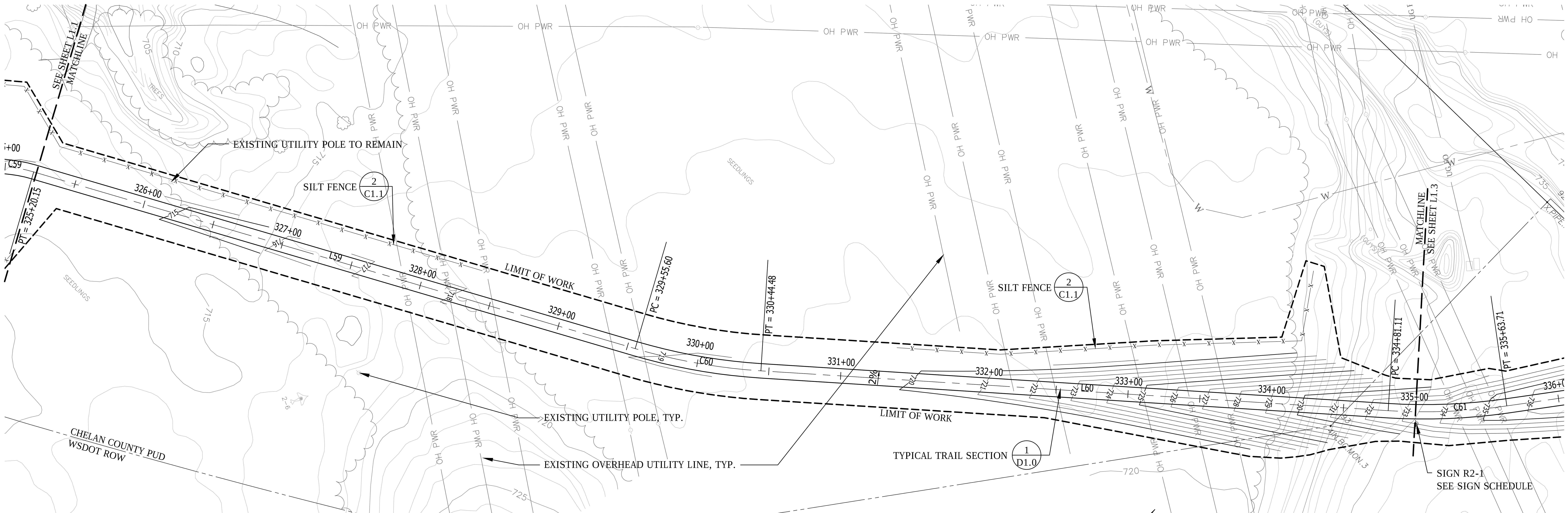

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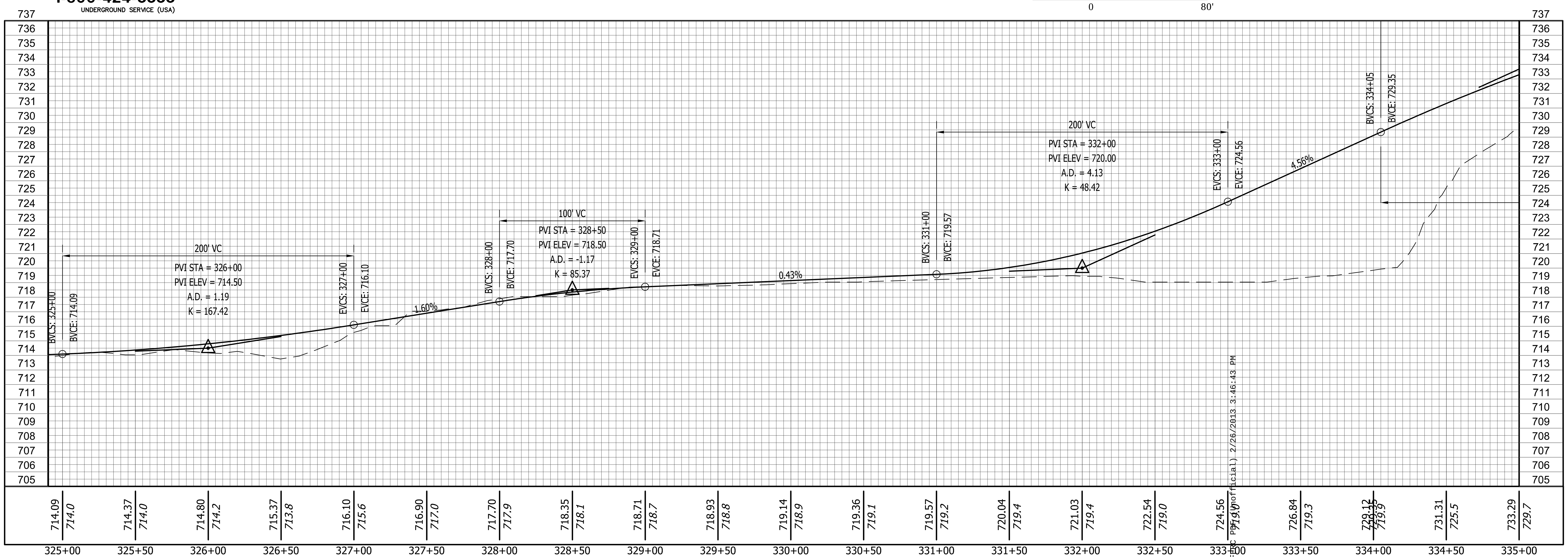
SILT FENCE 2 C1.1
ROW LINE
P.U.D. BOUNDARY LINE
PROPERTY LINE
LIMIT OF WORK LINE

40' 0 40' 80'



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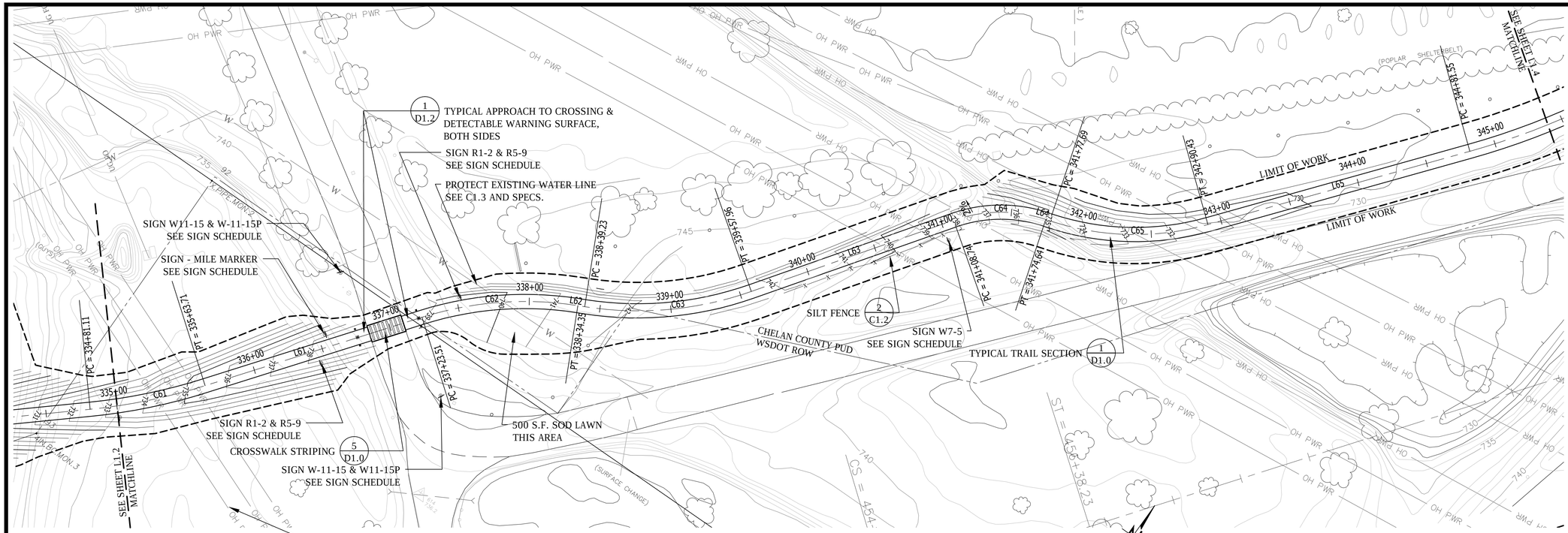
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

GRADING PLAN & PROFILE, TESC & DRAINAGE PLANS

L1.2

SCALE: H=1:40 V=1:4



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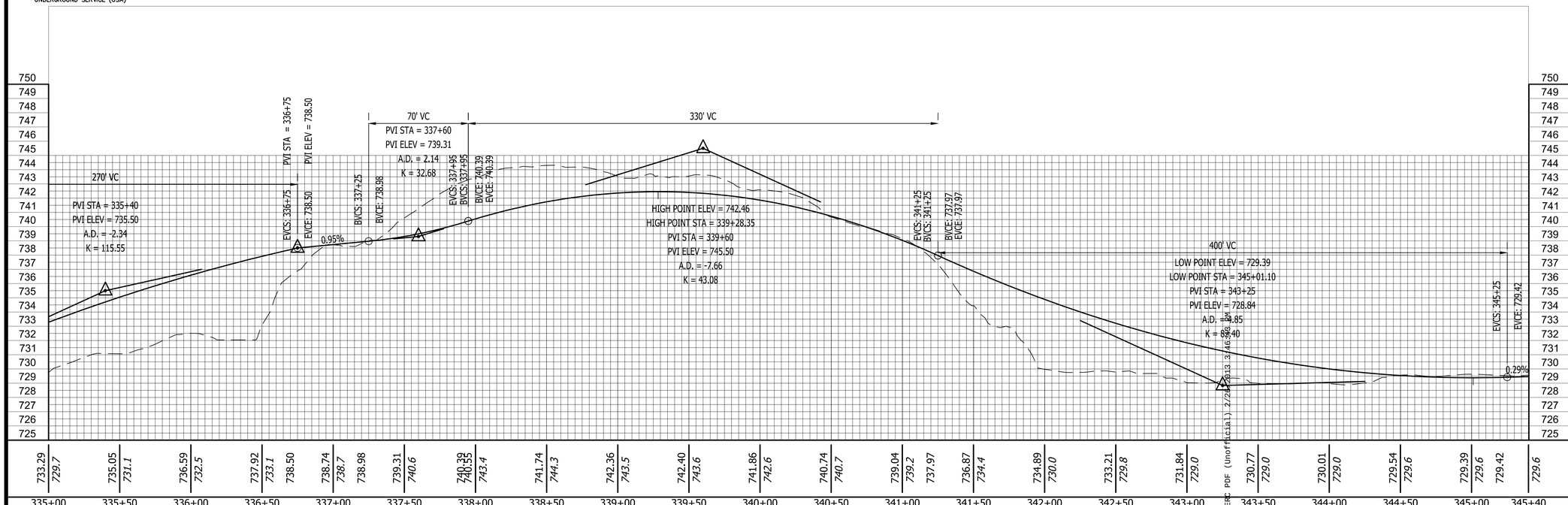
1-800-424-5555
UNDERGROUND SERVICE (USA)



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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

GRADING PLAN & PROFILE, TESC & DRAINAGE PLANS

L1.3

SCALE: H=1:40 V=1:4

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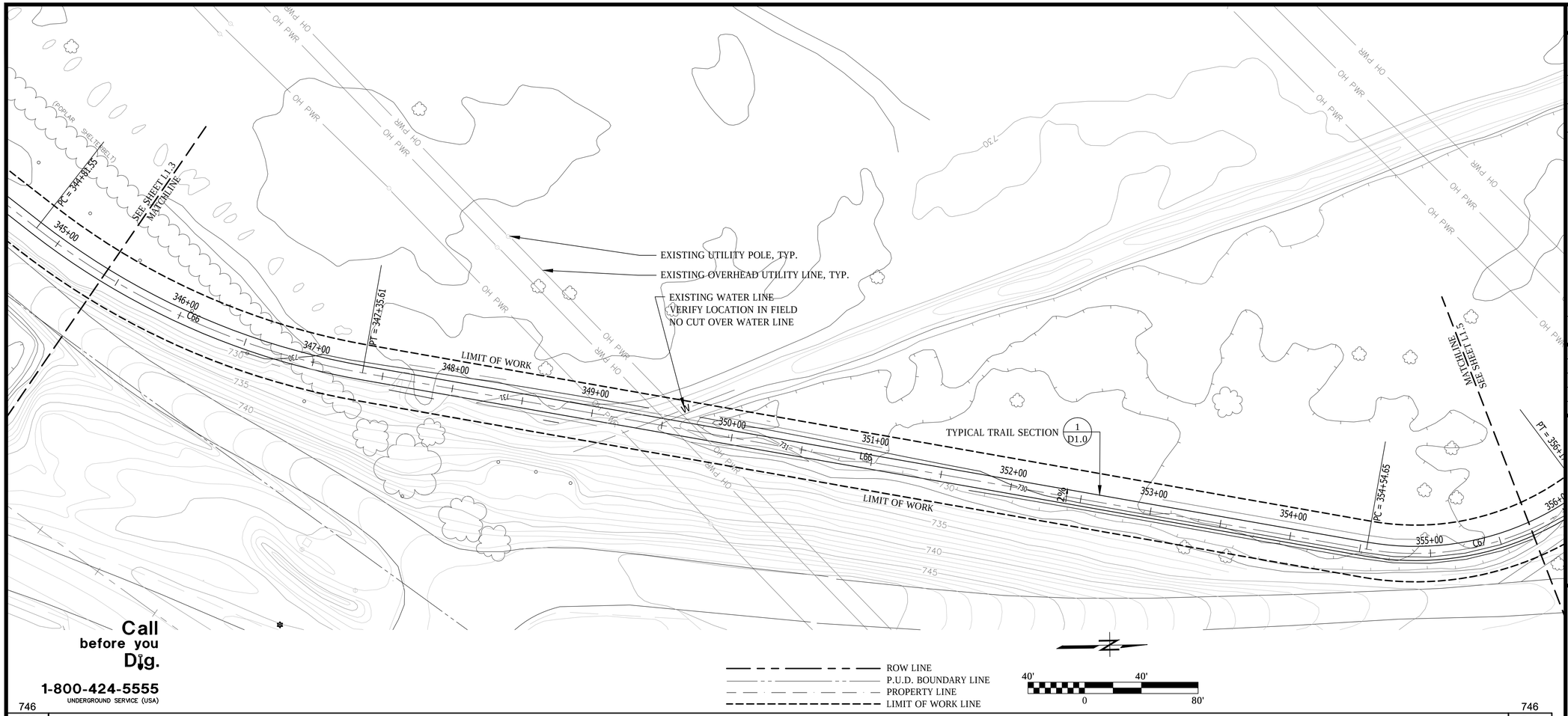

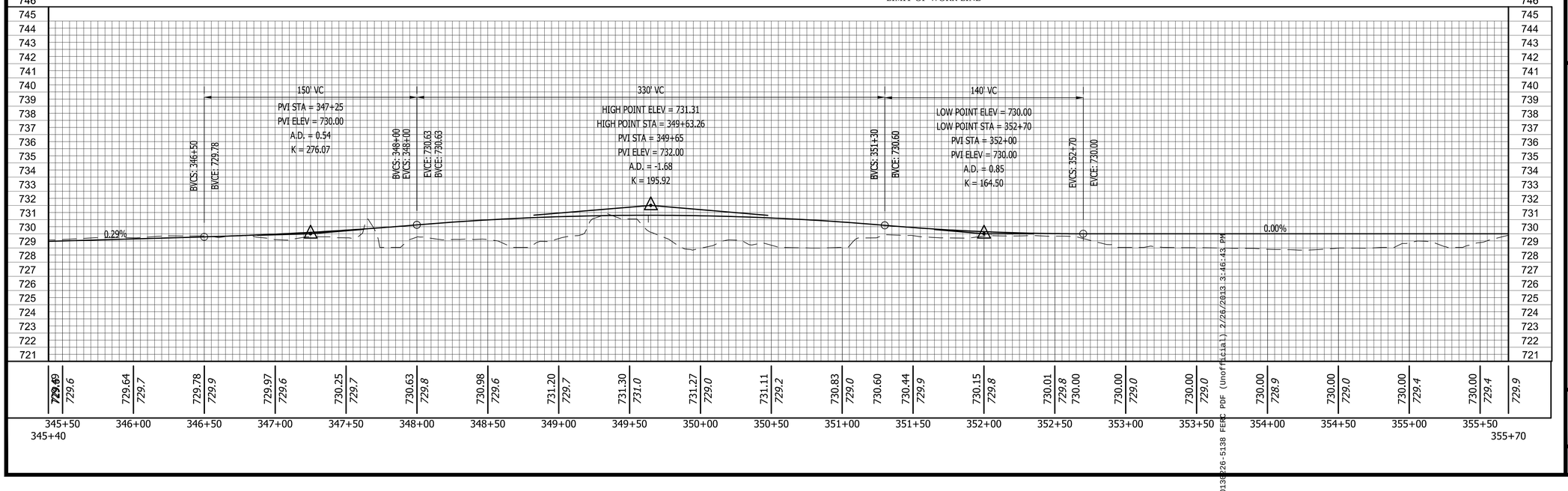

CAD NO. 10103-PLAN PROFILE-PH1.dwg

NO.	REVISIONS	INT.	APP.	DATE

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DESIGNED	TR	08/01/2012
DRAWN	MBM	08/01/2012
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CHECKED (HDQTS.)		

RONALD J. LEIMKUHLER
STATE OF WASHINGTON
24088
REGISTERED
PROFESSIONAL ENGINEER

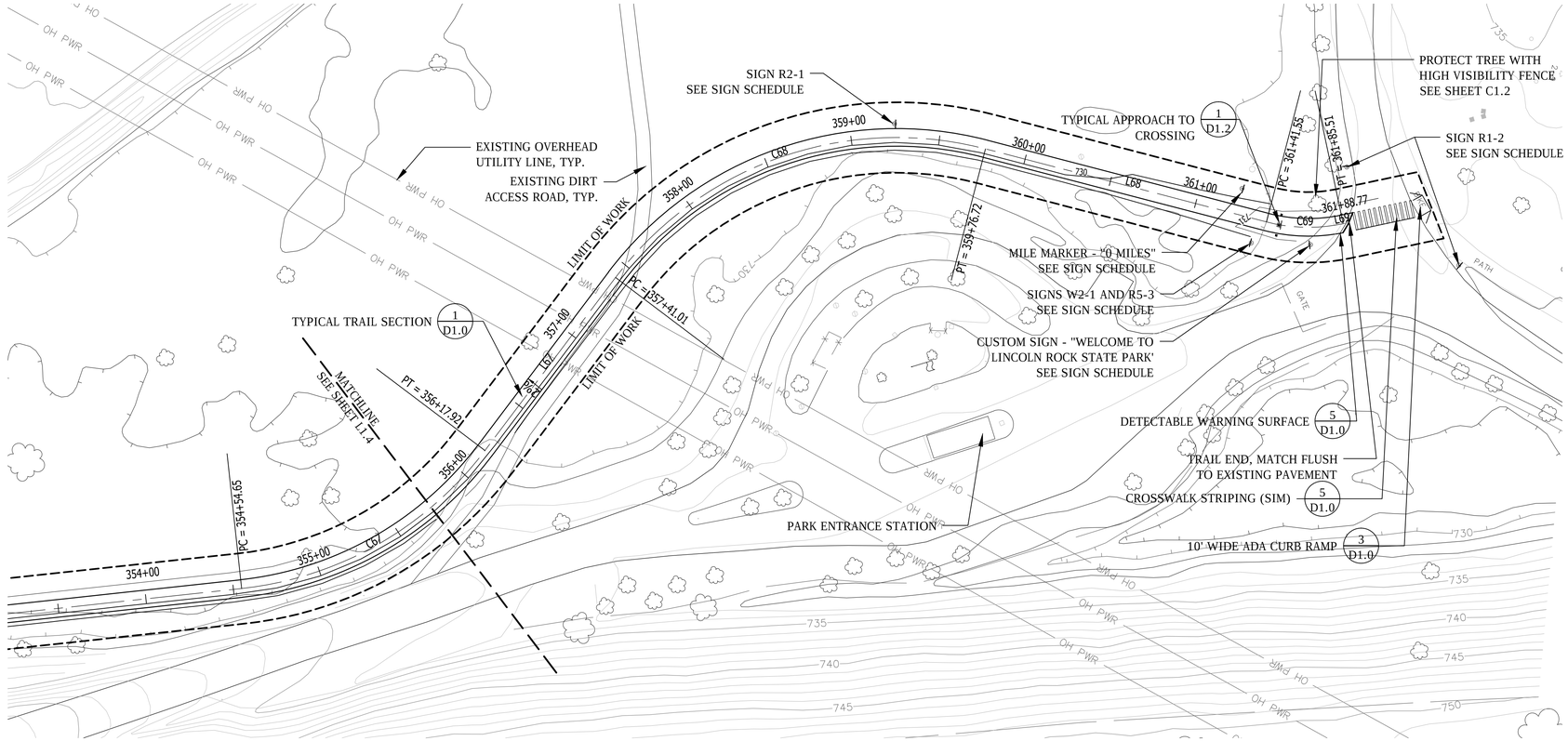
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

GRADING PLAN & PROFILE, TESC & DRAINAGE PLANS

L1.4

SCALE: H=1:40 V=1:4

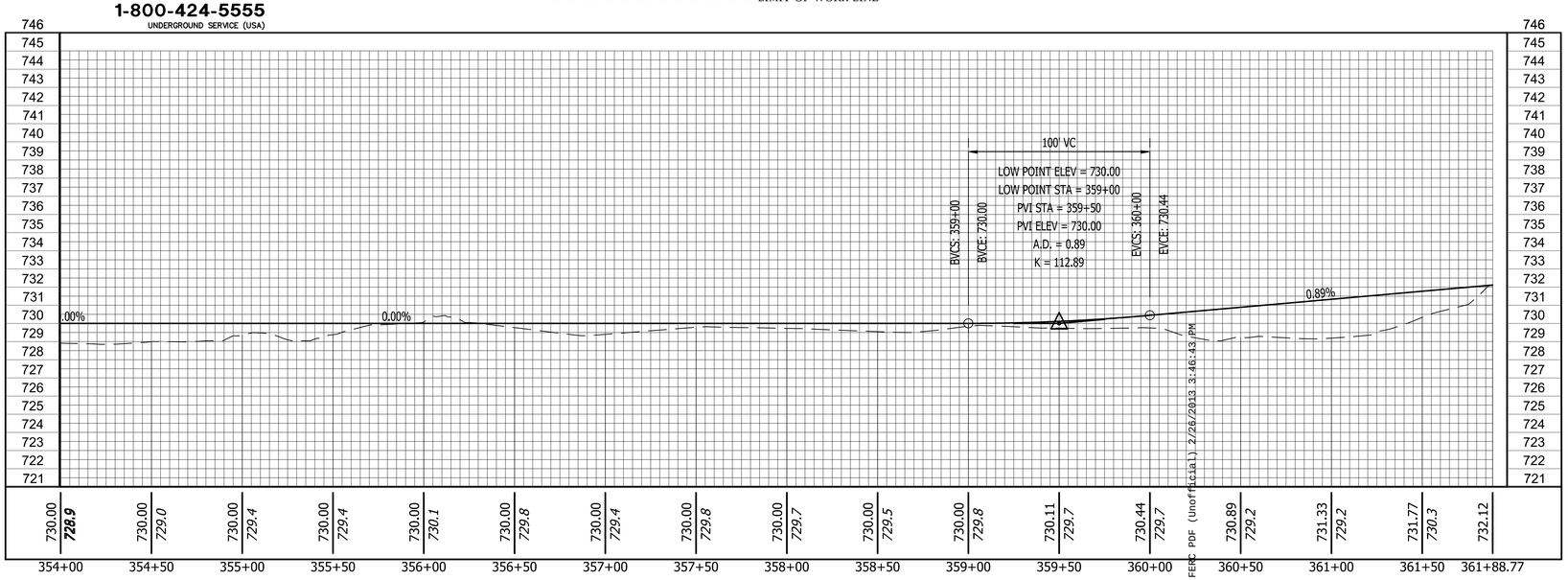


Call before you Dig.

1-800-424-5555
UNDERGROUND SERVICE (USA)

ROW LINE
P.U.D. BOUNDARY LINE
PROPERTY LINE
LIMIT OF WORK LINE

40' 0 40' 80'



2013-03-26-5138 FERC PDF (Unofficial) 2/26/2013 3:46:43 PM

CAD NO. 10103-PLAN PROFILE-PH1.dwg

NO.	REVISIONS	INT.	APP.	DATE

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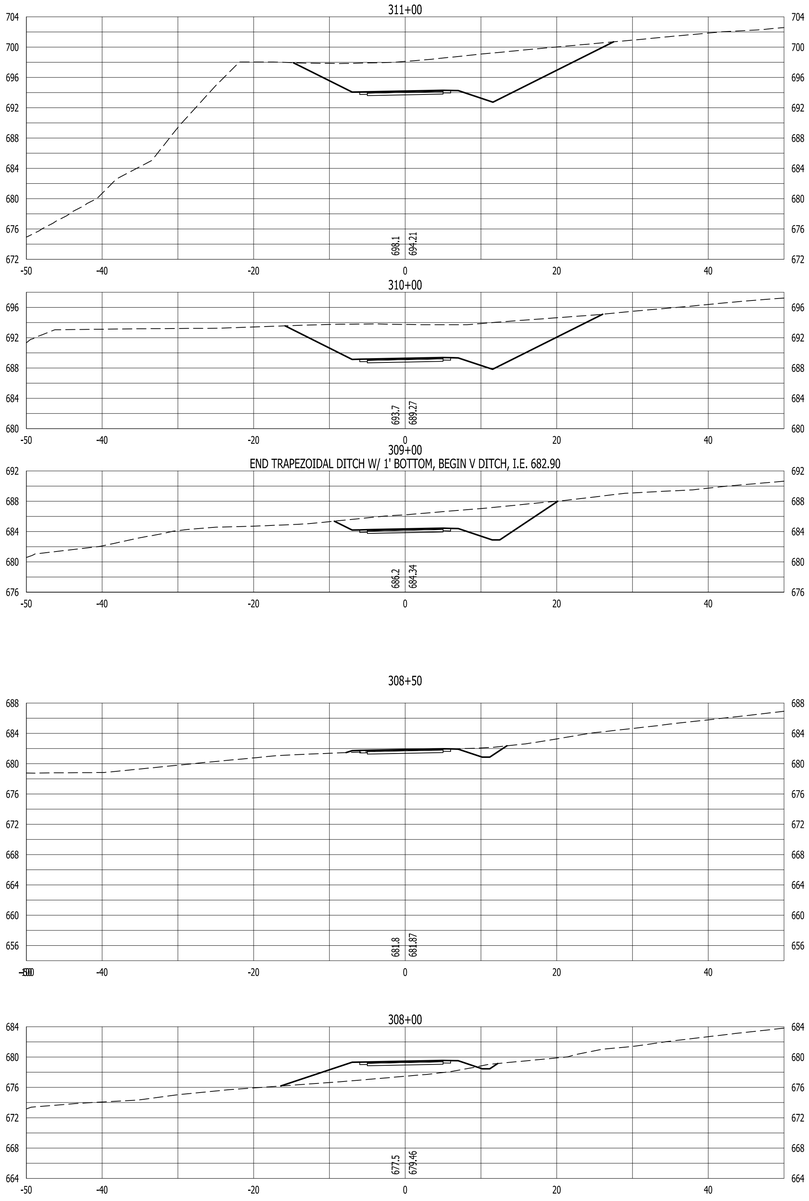
WASHINGTON STATE PARKS AND RECREATION COMMISSION

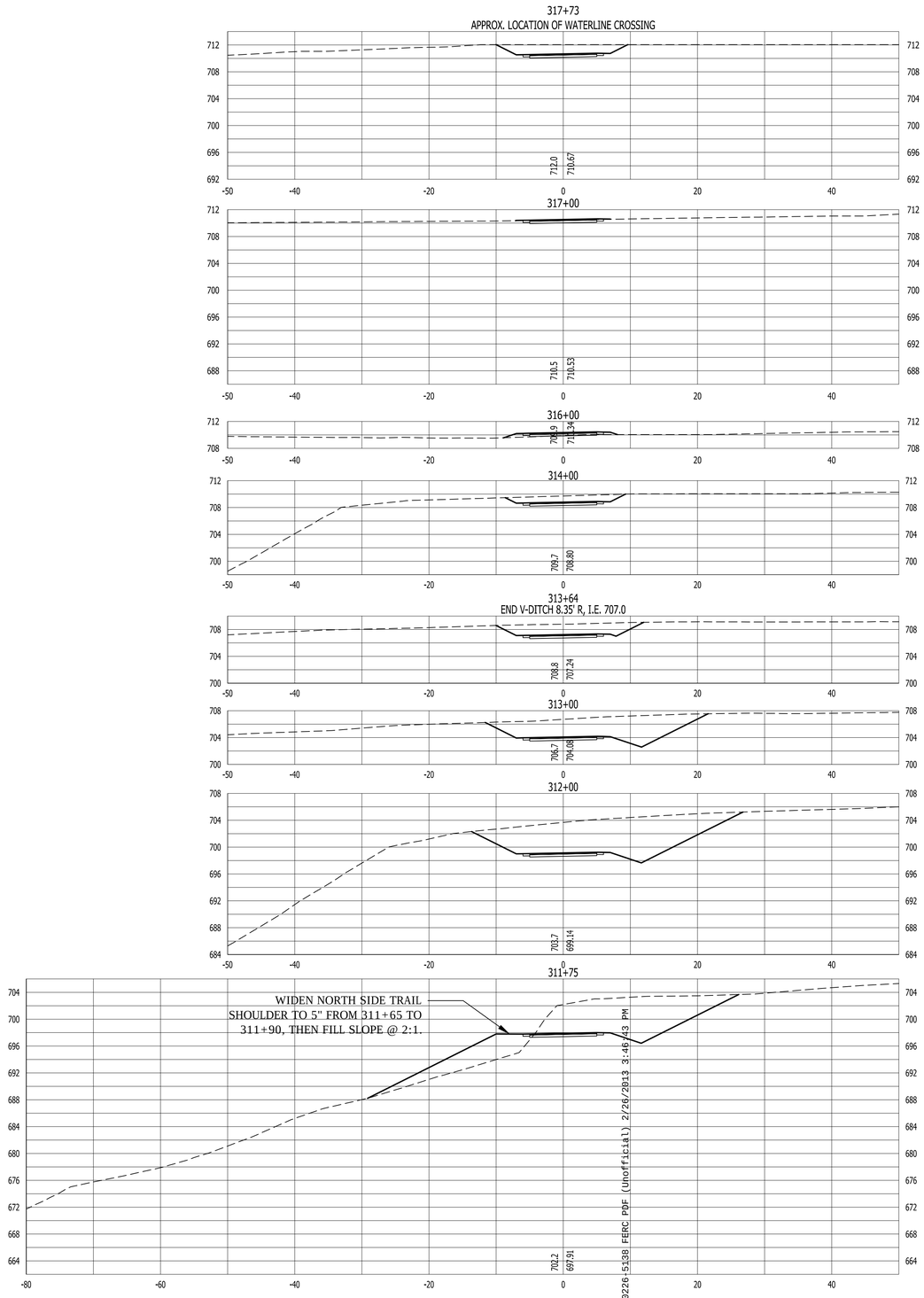
ROCKY REACH TRAIL PHASE ONE

GRADING PLAN & PROFILE, TESC & DRAINAGE PLANS

L1.5

SCALE: H=1:40 V=1:4





CAD NO. 10103-SECTIONS PH1.dwg

NO.	REVISIONS	INT.	APP.	DATE

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CHECKED (HDQTS.)		

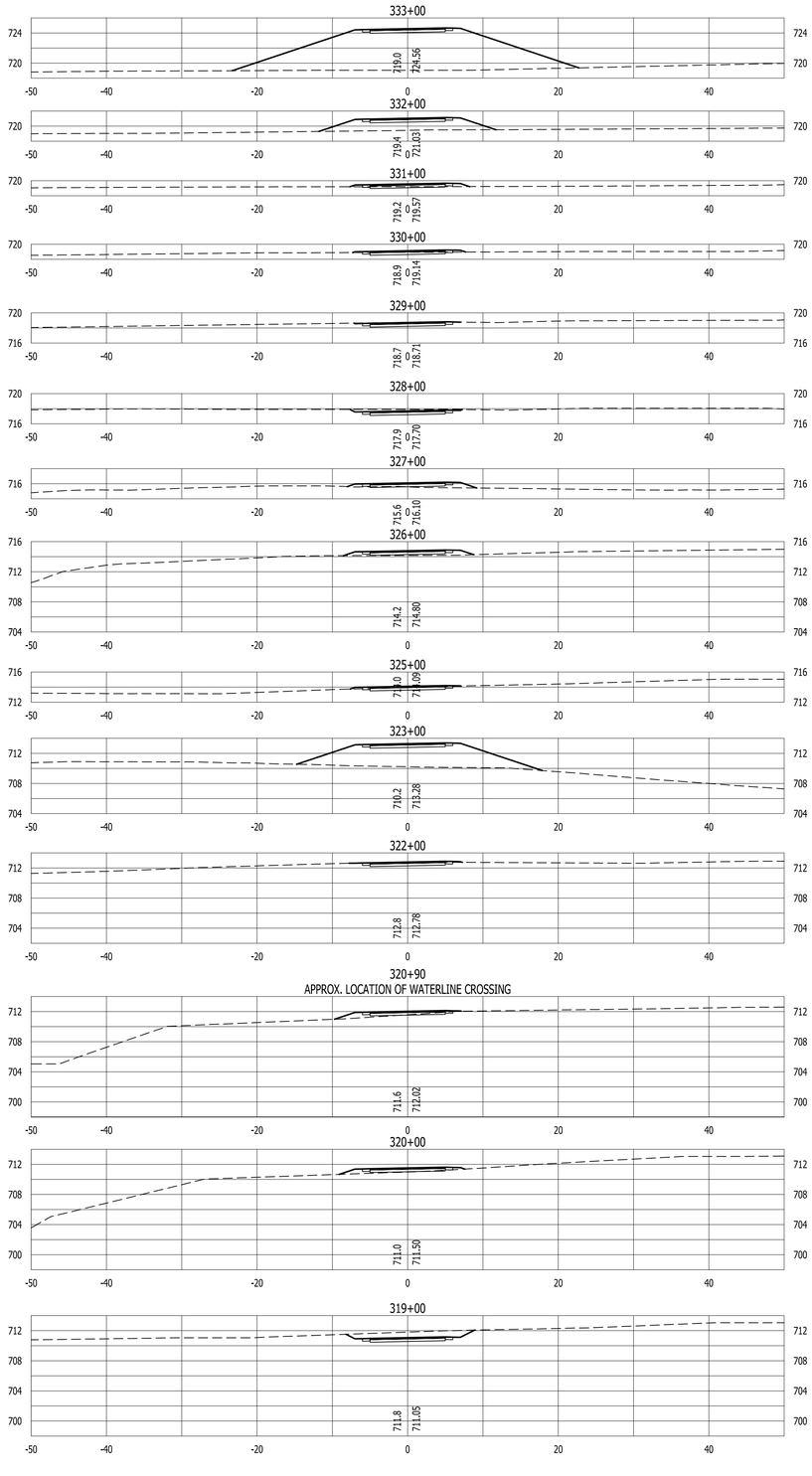
WASHINGTON STATE PARKS AND RECREATION COMMISSION

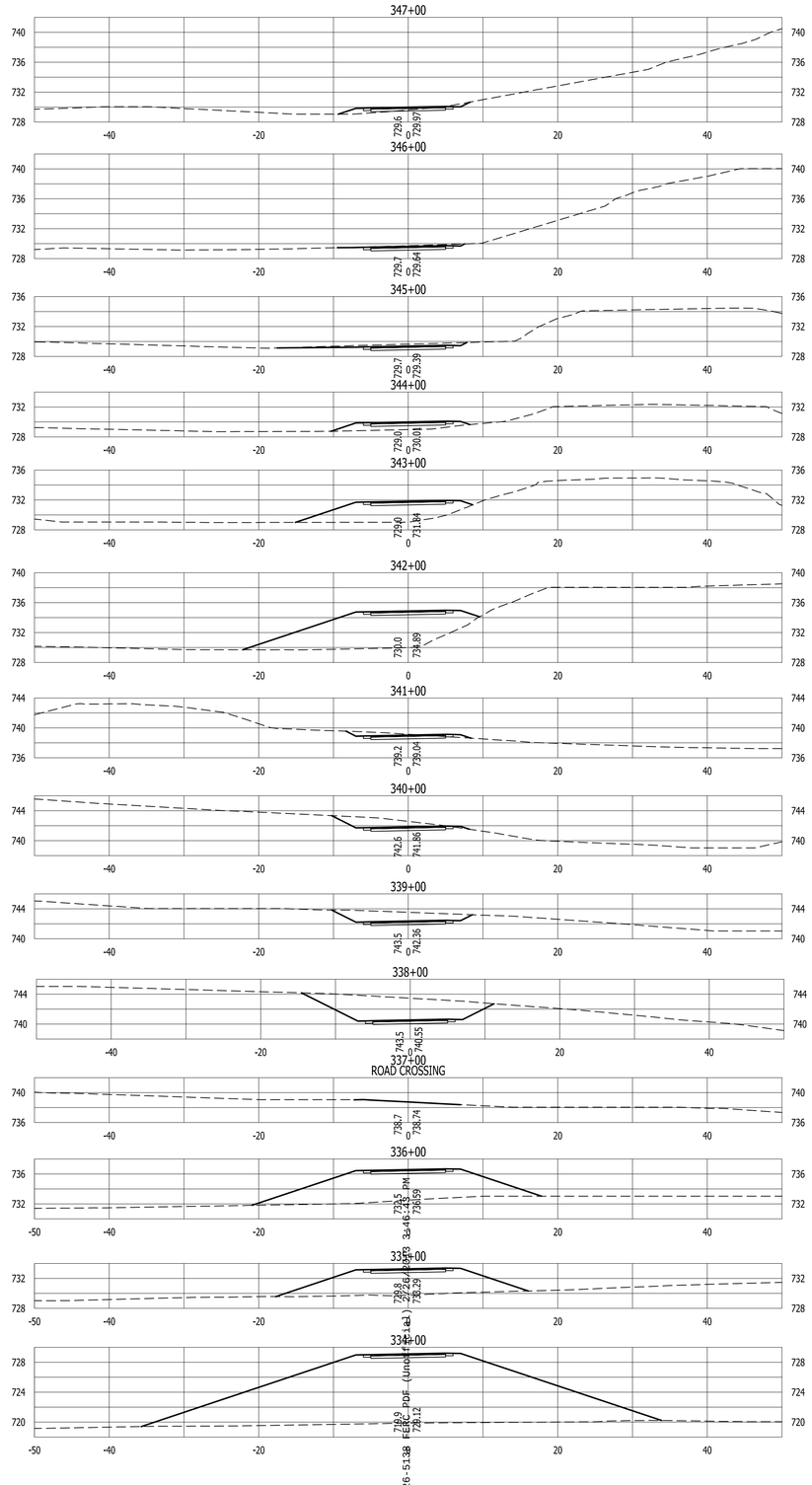
ROCKY REACH TRAIL PHASE ONE

TRAIL SECTIONS

L2.0

SCALE: 1"=10'





CAD NO. 10103-SECTIONS PH1.dwg

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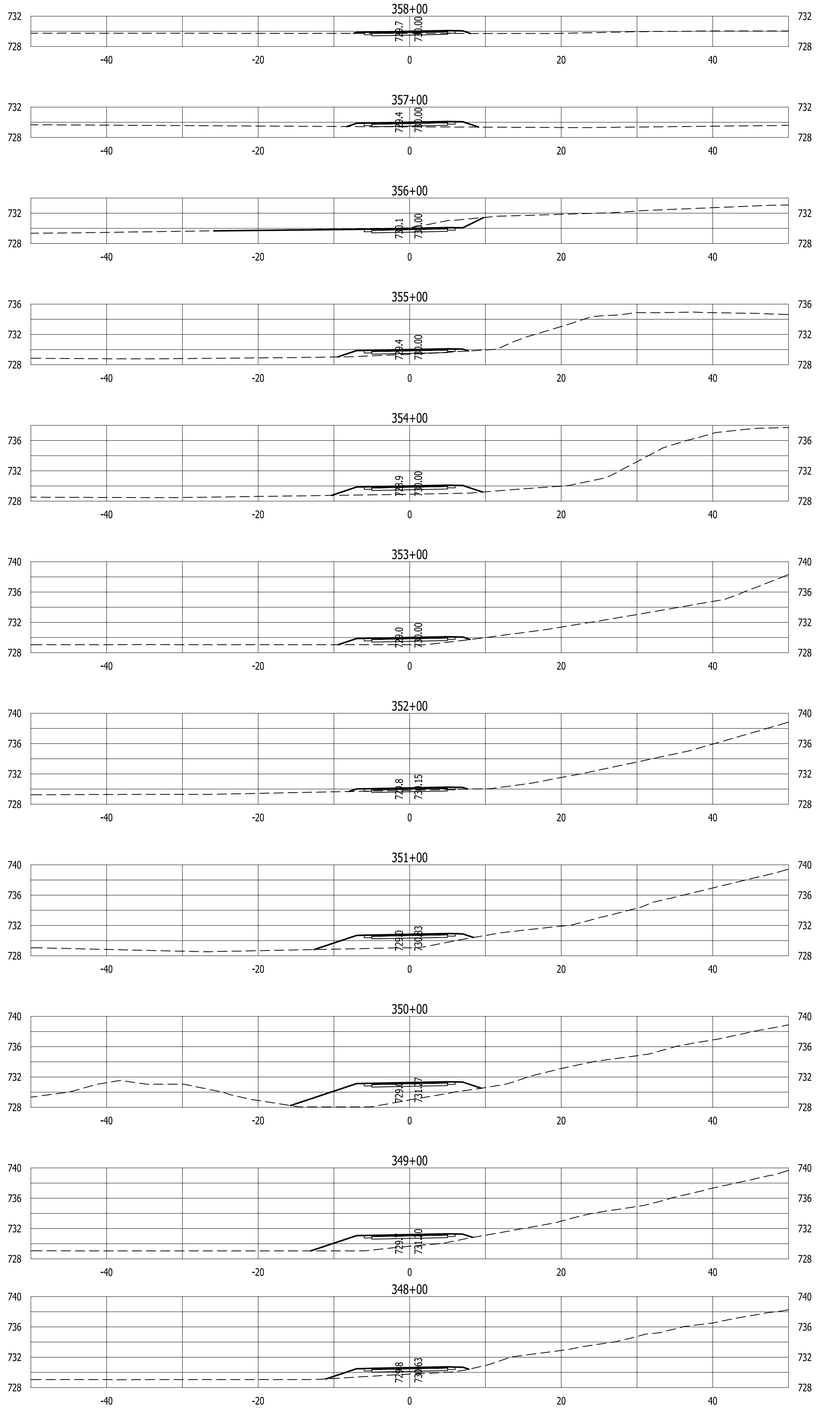
WASHINGTON STATE PARKS AND RECREATION COMMISSION

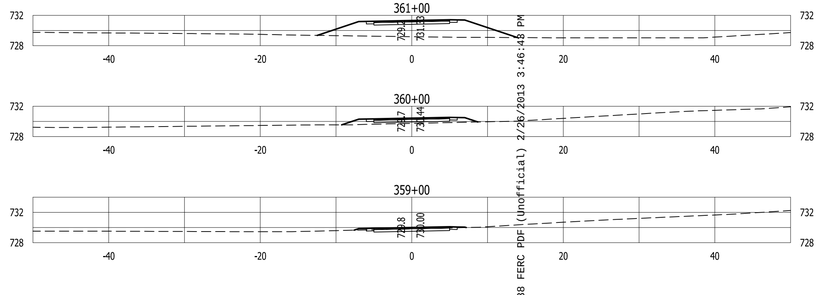
ROCKY REACH TRAIL PHASE ONE

TRAIL SECTIONS

L2.1

SCALE: 1"=10'





CAD NO. 10103-SECTIONS PH1.dwg

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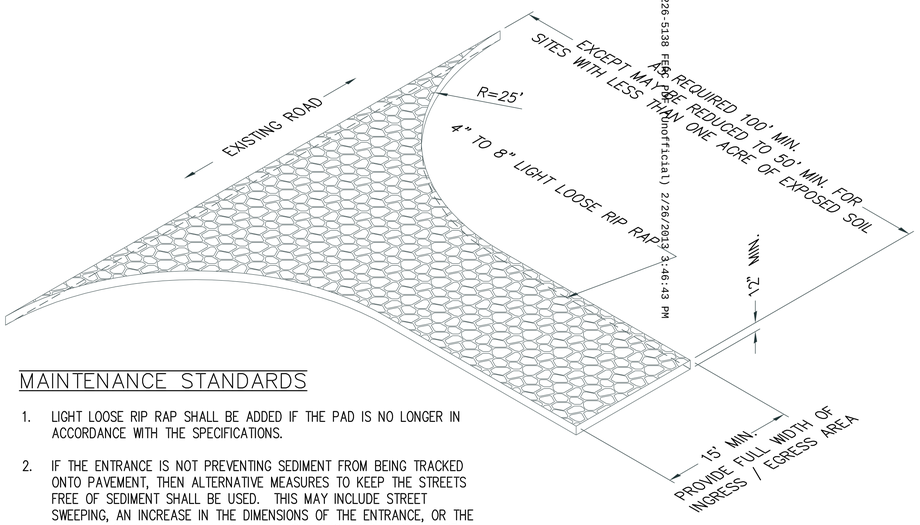
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

TRAIL SECTIONS

L2.2

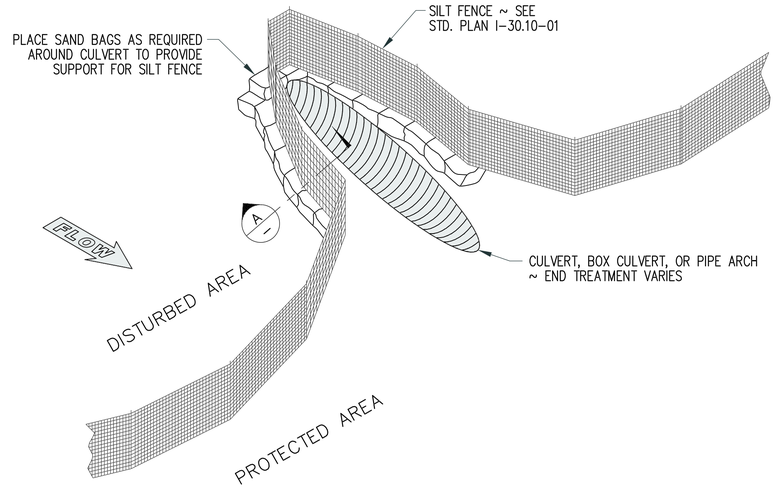
SCALE: 1"=10'



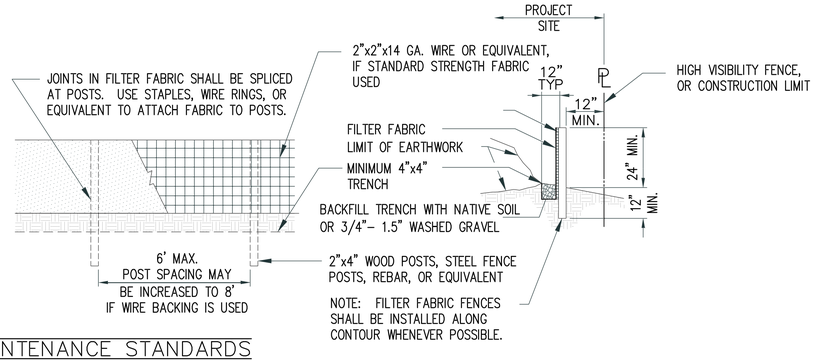
MAINTENANCE STANDARDS

1. LIGHT LOOSE RIP RAP SHALL BE ADDED IF THE PAD IS NO LONGER IN ACCORDANCE WITH THE SPECIFICATIONS.
2. IF THE ENTRANCE IS NOT PREVENTING SEDIMENT FROM BEING TRACKED ONTO PAVEMENT, THEN ALTERNATIVE MEASURES TO KEEP THE STREETS FREE OF SEDIMENT SHALL BE USED. THIS MAY INCLUDE STREET SWEEPING, AN INCREASE IN THE DIMENSIONS OF THE ENTRANCE, OR THE INSTALLATION OF A WHEEL WASH. IF WASHING IS USED, IT SHALL BE DONE ON AN AREA COVERED WITH CRUSHED ROCK, AND WASH WATER SHALL DRAIN TO A SEDIMENT TRAP OR POND.
3. ANY SEDIMENT THAT IS TRACKED ONTO PAVEMENT SHALL BE REMOVED IMMEDIATELY BY SWEEPING. THE SEDIMENT COLLECTED BY SWEEPING SHALL BE REMOVED OR STABILIZED ON-SITE. THE PAVEMENT SHALL NOT BE CLEANED BY WASHING DOWN THE STREET, EXCEPT WHEN SWEEPING IS INEFFECTIVE AND THERE IS A THREAT TO PUBLIC SAFETY. IF IT IS NECESSARY TO WASH THE STREETS, THE CONSTRUCTION OF A SMALL SUMP SHALL BE CONSIDERED. THE SEDIMENT WOULD THEN BE WASHED INTO THE SUMP.
4. ANY LIGHT LOOSE RIP RAP THAT ARE LOOSENED FROM THE PAD AND END UP ON THE ROADWAY SHALL BE REMOVED IMMEDIATELY.
5. IF VEHICLES ARE ENTERING OR EXITING THE SITE AT POINTS OTHER THAN THE CONSTRUCTION ENTRANCE(S), FENCING (SEE SECTION D.4.1) SHALL BE INSTALLED TO CONTROL TRAFFIC.

1 CONSTRUCTION ENTRANCE DETAIL



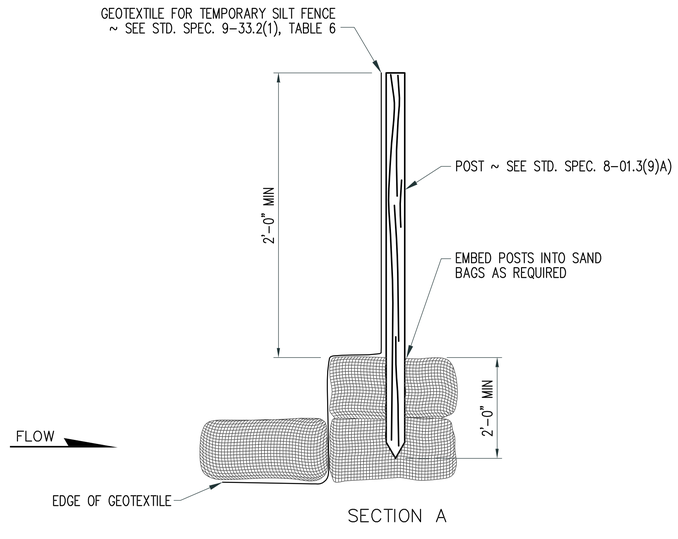
3 SILT FENCE AT OUTFALL DETAIL



MAINTENANCE STANDARDS

1. ANY DAMAGE SHALL BE REPAIRED IMMEDIATELY.
2. IF CONCENTRATED FLOWS ARE EVIDENT UPHILL OF THE FENCE, THEY MUST BE INTERCEPTED AND CONVEYED TO A SEDIMENT TRAP OR POND.
3. IT IS IMPORTANT TO CHECK THE UPHILL SIDE OF THE FENCE FOR SIGNS OF THE FENCE CLOGGING AND ACTING AS A BARRIER TO FLOW AND THEN CAUSING CHANNELIZATION OF FLOWS PARALLEL TO THE FENCE. IF THIS OCCURS, REPLACE THE FENCE OR REMOVE THE TRAPPED SEDIMENT.
4. SEDIMENT MUST BE REMOVED WHEN THE SEDIMENT IS 6" HIGH.
5. IF THE FILTER FABRIC HAS DETERIORATED DUE TO ULTRAVIOLET BREAKDOWN, IT SHALL BE REPLACED.
6. SILT FENCE SHALL BE LOCATED WITHIN LIMITS SET BY HIGH VISIBILITY FENCING OR 12" FROM LIMIT OF GROUND DISTURBANCE EARTHWORK, WHICHEVER IS MORE RESTRICTIVE.

2 SILT FENCE DETAIL



SECTION A

kpff Consulting Engineers
1601 Fifth Avenue, Suite 1600
Seattle, Washington 98101-3665
(206) 622-5822 Fax (206) 622-8130

CAD NO.L450-2010-E01-01EC.DWG

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RONALD J. LEIMKUHLER STATE OF WASHINGTON 24089 REGISTERED PROFESSIONAL ENGINEER

REGISTERED STAMP

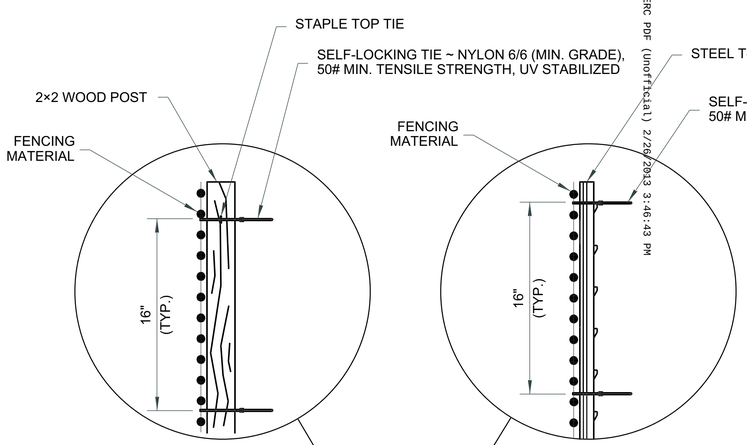
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

EROSION CONTROL & DRAINAGE DETAILS

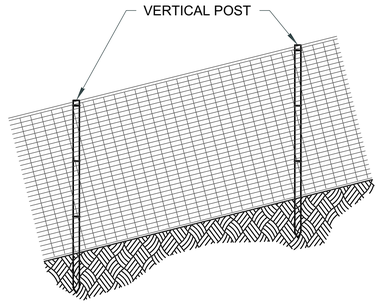
SHT. NO. C1.1

SCALE: NTS

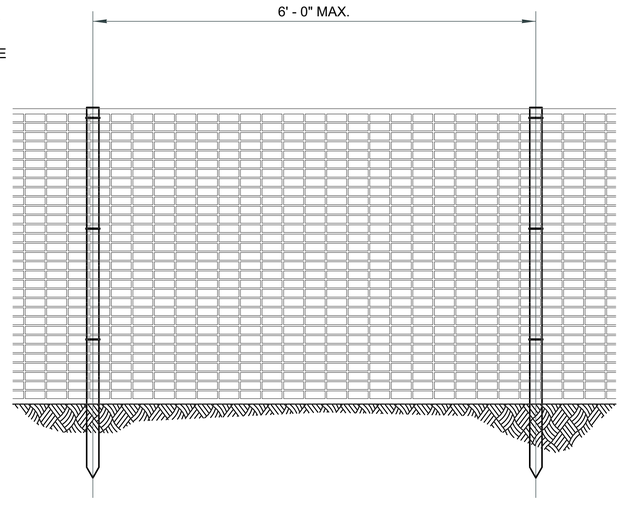


NOTE: WHERE HIGH VISIBILITY FENCE TO PROTECT EXISTING TREES ARE TO BE INSTALLED AROUND DRIP LINE OF TREE.

TYPICAL SECTION



ELEVATION
FENCE ON SLOPE

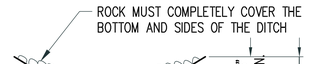


ELEVATION

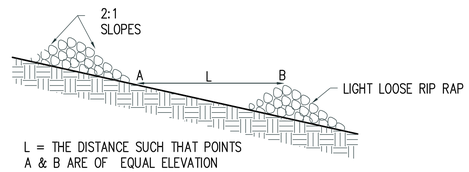
NOTE

1. POST SHALL HAVE SUFFICIENT STRENGTH AND DURABILITY TO SUPPORT THE FENCE THROUGH THE LIFE OF THE PROJECT.

1/– *HIGH VISIBILITY FENCE*



CROSS SECTION



L = THE DISTANCE SUCH THAT POINTS A & B ARE OF EQUAL ELEVATION

CHECK DAM SPACING

* CHECK DAMS ARE TO BE PLACED ON ALL SLOPES GREATER THAN 2.5%

MAINTENANCE STANDARDS

1. ANY SEDIMENT DEPOSITION OF MORE THAN 0.5 FEET SHALL BE REMOVED SO THAT THE CHANNEL IS RESTORED TO ITS DESIGN CAPACITY.
2. THE CHANNEL SHALL BE EXAMINED FOR SIGNS OF SCOURING AND EROSION OF THE BED AND BANKS. IF SCOURING OR EROSION HAS OCCURRED, AFFECTED AREAS SHALL BE PROTECTED BY RIP-RAP OR AN EROSION CONTROL BLANKET OR NET.

2/– *ROCK CHECK DAM*

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RONALD J. LEIMKUHLER STATE OF WASHINGTON 24089 REGISTERED PROFESSIONAL ENGINEER

REGISTERED STAMP

WASHINGTON STATE PARKS AND RECREATION COMMISSION

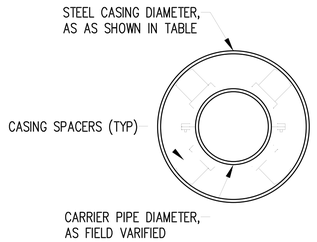
ROCKY REACH TRAIL
PHASE ONE

EROSION CONTROL & DRAINAGE DETAILS

C1.2
SHT. NO.

SCALE: NTS

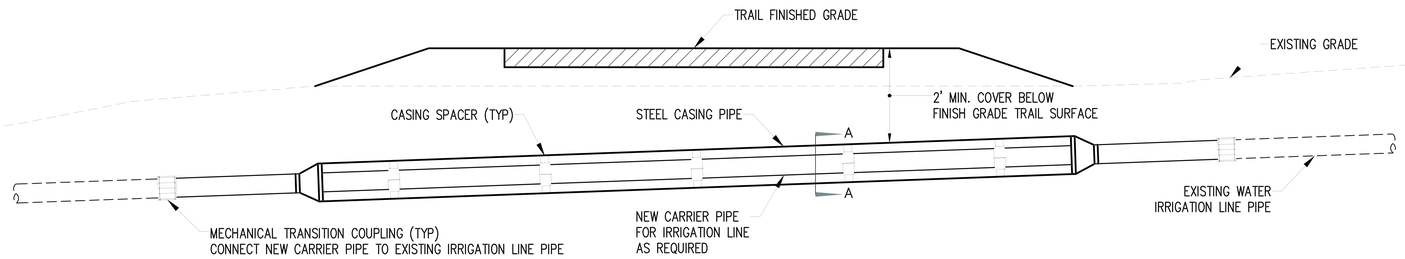
CARRIER PIPEØ	MIN. CASING Ø
4"	10"
6"	12"
8"	16"
12"	20"
14"	24"
18"	30"
>18"	BY ENGINEER



SECTION A-A

NOTES:

1. FIELD VERIFY LOCATIONS OF EXISTING WATER IRRIGATION LINES AND DETERMINE DEPTH AND SIZE. FIELD MODIFY REPLACEMENT EXTENT LENGTH AS REQUIRED, 20 FOOT MINIMUM CENTERED BENEATH TRAIL CENTERLINE, TO PROVIDE DEPTH OF COVER SHOWN.
2. STEEL PIPE CASING SHALL BE SHALL BE SMOOTH STEEL PIPE FABRICATED IN SECTIONS IN ACCORDANCE WITH ASTM A53 AND A139 GRADE B. LENGTH OF CASING PIPE SHALL BE AS LONG AS PRACTICAL FOR SITE CONDITIONS.
3. JOINTS SHALL CONFORM TO THE REQUIREMENTS OF AWWA C206.
4. CASING SPACERS SHALL BE RESTRAINED-TYPE BOLTED SPACERS AND SHALL HAVE A MAXIMUM SPACING OF 3 PER 18 FOOT OF PIPE, OR AS RECOMMENDED BY MANUFACTURER, WHICHEVER IS CLOSER. PIPE CASING SPACERS SHALL BE MANUFACTURED BY PIPE SEAL AND INSULATORS, INC., ADVANCE PRODUCTS AND SYSTEMS, INC. CALPICO, CASCADE WATER WORKS MFG, INC., OR APPROVED EQUAL.
5. CASING END SEALS SHALL BE SYNTHETIC NEOPRENE RUBBER PULL-ON TYPE END SEALS WITH STAINLESS STEEL BANDS, AS MANUFACTURED BY THE ABOVE MANUFACTURERS, OR APPROVED EQUAL.
6. VERIFY EXISTING WATER IRRIGATION LINE PIPE MATERIAL AND SELECT MECHANICAL TRANSITION COUPLING ACCORDINGLY TO PROPERLY RECONNECT NEW CARRIER PIPE TO EXISTING IRRIGATION LINE PIPE. MECHANICAL COUPLING SHALL BE ROMAC INDUSTRIES, INC., DRESSER INC., OR APPROVED EQUAL.



X/– CASING DETAIL

SCALE: 1" = 2'

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STATE OF WASHINGTON
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PROFESSIONAL ENGINEER

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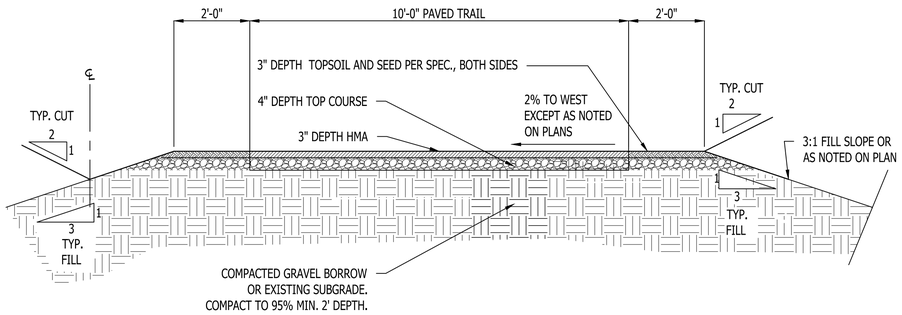
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

UTILITY DETAIL

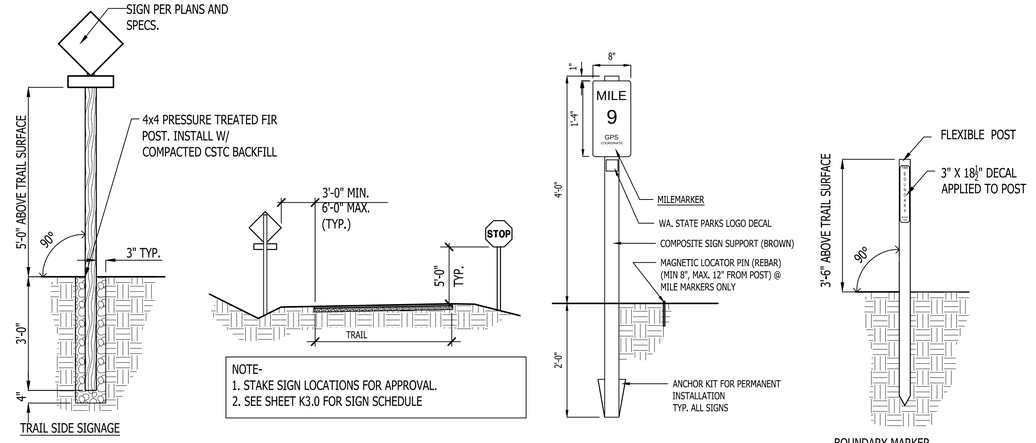
C1.3
SHT. NO.

SCALE: NTS



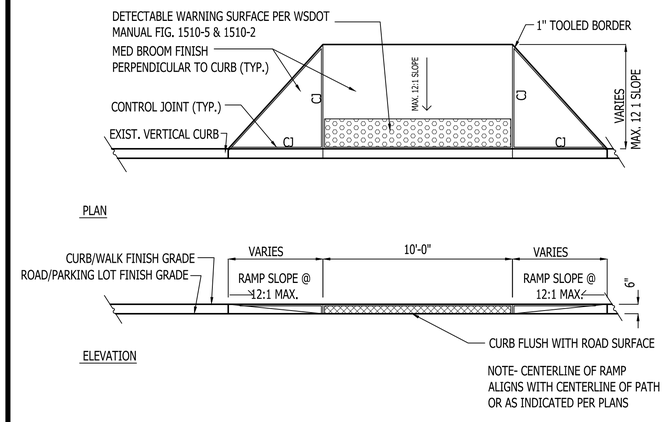
1 TYPICAL TRAIL SECTION

0 2' 4'



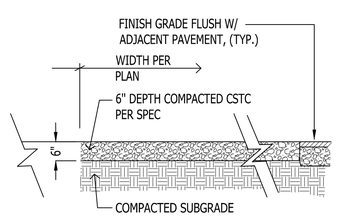
2 TRAIL SIGNS

N.T.S.



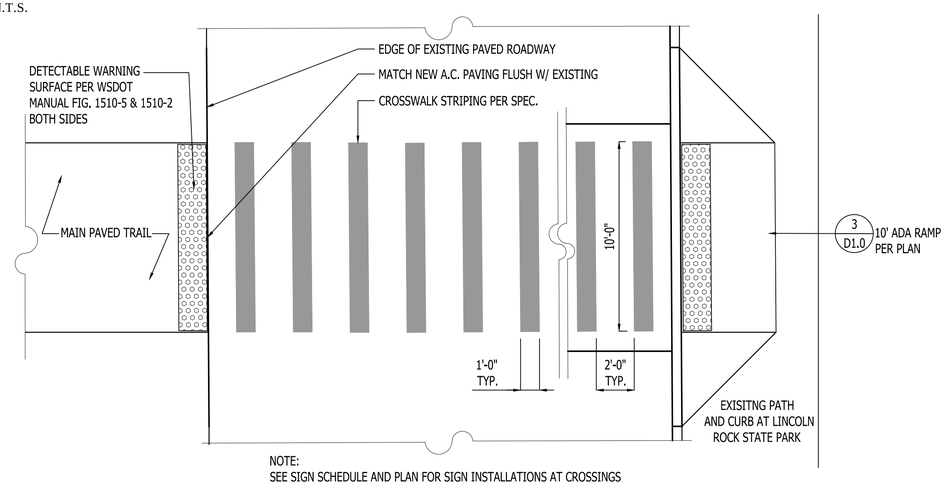
3 10' ADA CURB RAMP

N.T.S.



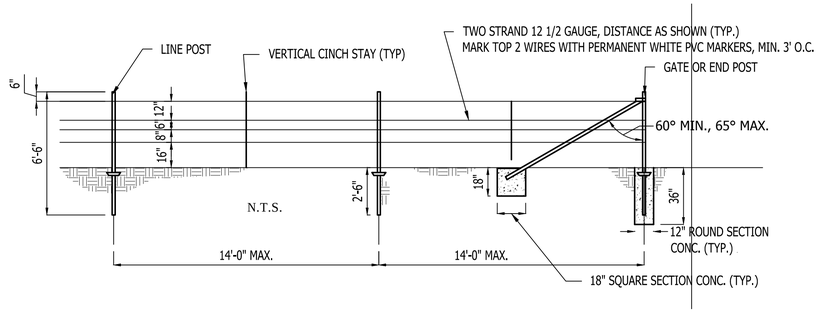
4 CRUSHED ROCK SURFACE

N.T.S.



5 CROSSWALK STRIPING

0 4' 8'



6 SMOOTH WIRE FENCE

N.T.S.

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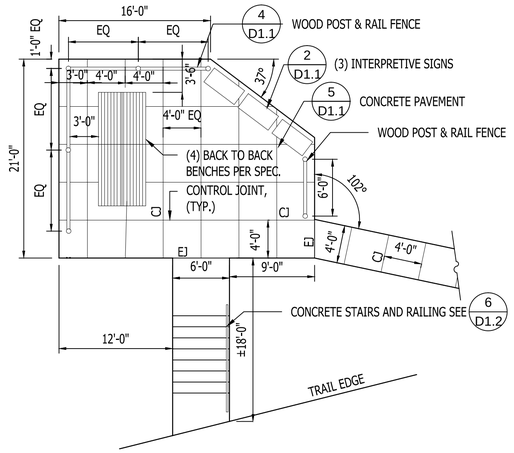
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

DETAILS

D1.0

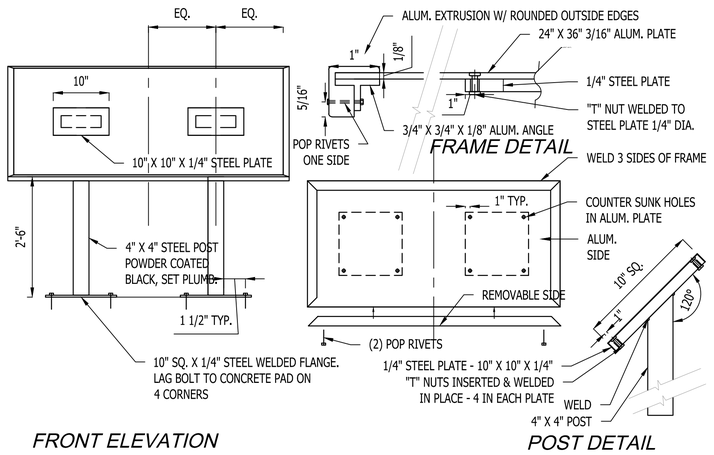
SCALE: AS NOTED



VIEWPOINT WITH INFORMATION / INTERPRETIVE SIGN

1

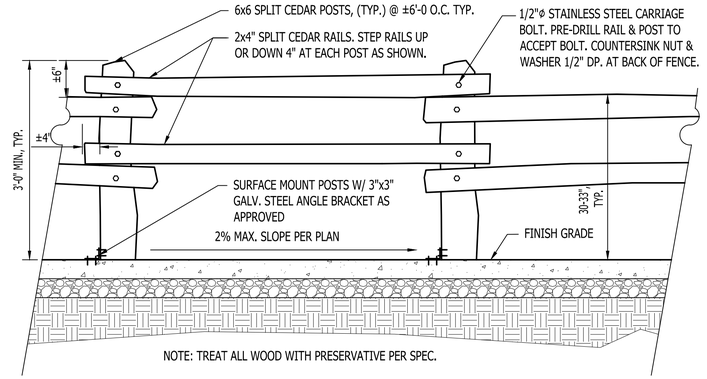
0 8' 16'



INTERPRETIVE SIGN

2

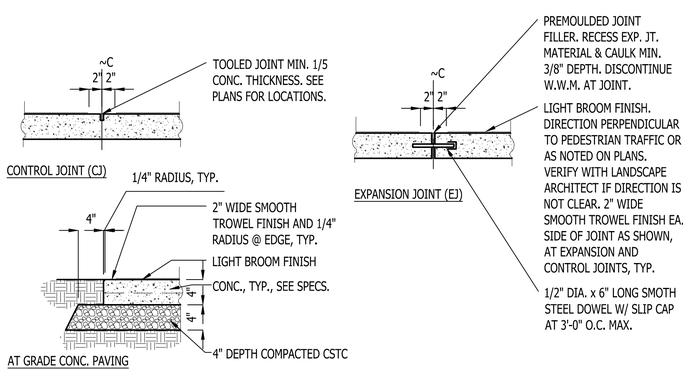
N.T.S.



WOOD POST & RAIL FENCE

4

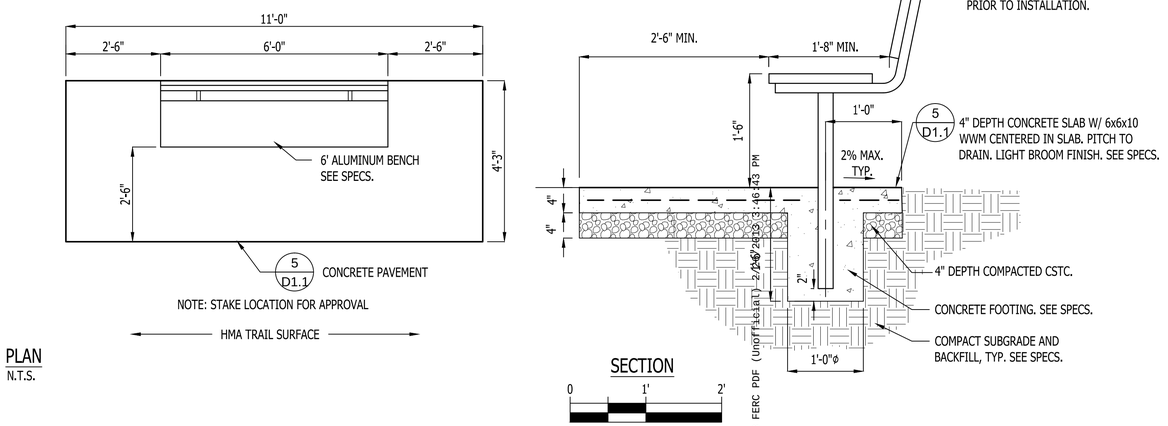
N.T.S.



CONCRETE PAVEMENT @ VIEWPOINT

5

N.T.S.



BENCH WITH CONCRETE PAD

6

SCALE: AS NOTED

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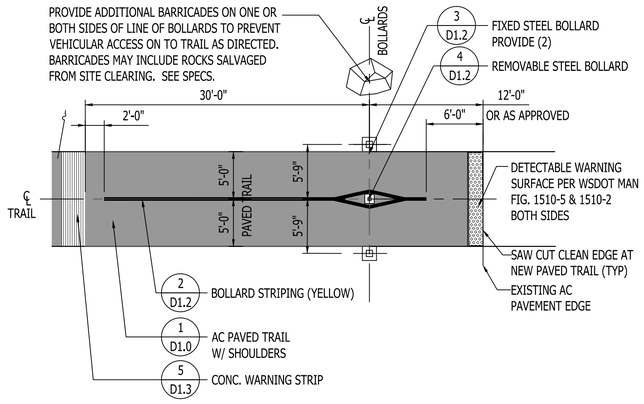
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

DETAILS

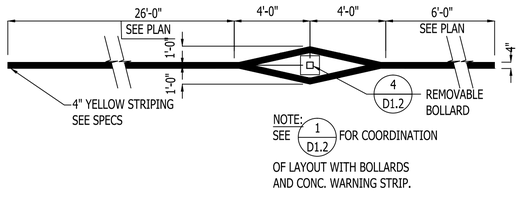
D1.1

SCALE: AS NOTED



1 TYPICAL APPROACH TO CROSSING

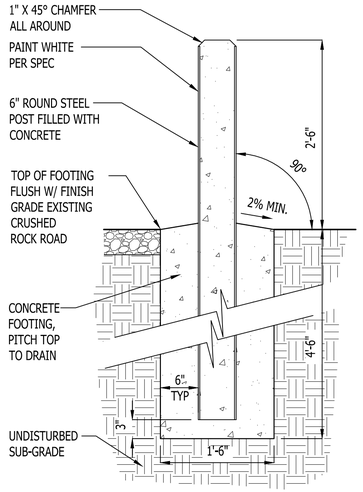
0 8' 16'



NOTE: SEE 1 D1.2 FOR COORDINATION OF LAYOUT WITH BOLLARDS AND CONC. WARNING STRIP.

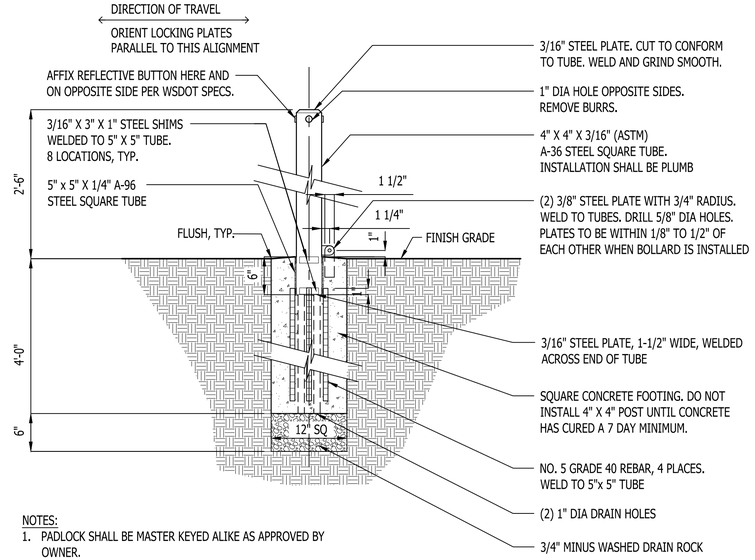
2 BOLLARD STRIPING

0 4' 8'



3 FIXED STEEL BOLLARD

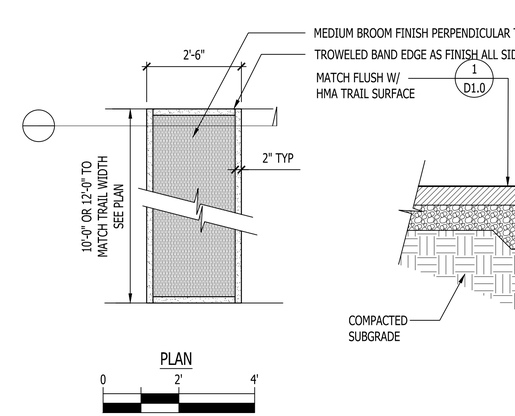
0 1' 2'



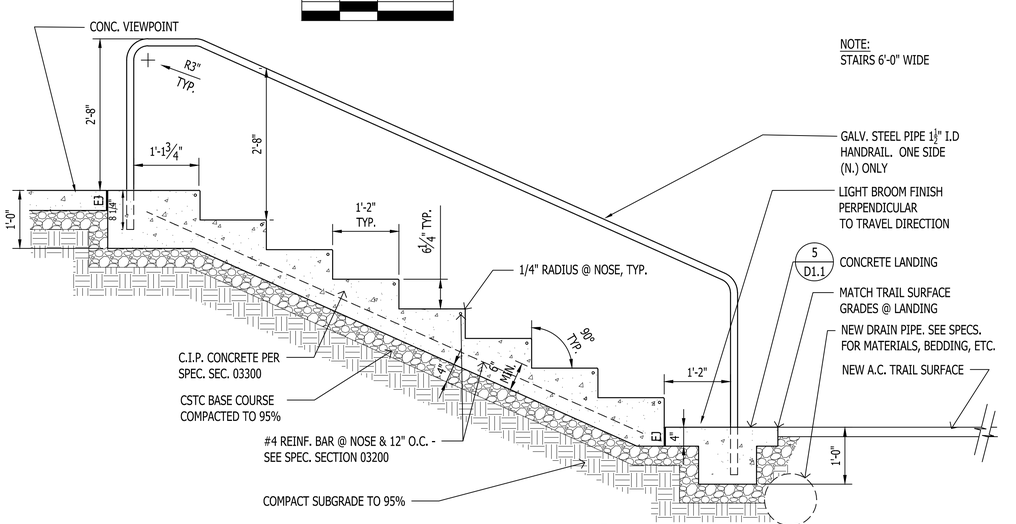
NOTES:
1. PADLOCK SHALL BE MASTER KEYED ALIKE AS APPROVED BY OWNER.
2. SLOPE TOP OF CONCRETE FOOTING TO DRAIN AWAY FROM POST.
3. HOT DIP GALV. & PRIME PAINT - FINISH COLOR TO BE WHITE MARINE ENAMEL

4 REMOVABLE STEEL BOLLARD

0 1' 2'



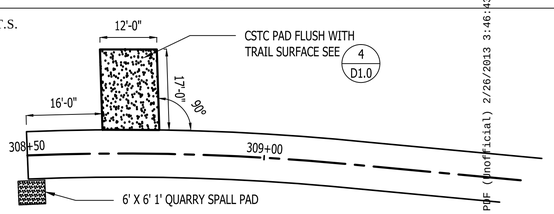
5 CONCRETE WARNING STRIP @ ROAD CROSSINGS



NOTE: STAIRS 6'-0" WIDE

6 CONCRETE STAIRS AND RAILING

N.T.S.



7 ENLARGED AREA PLAN

N.T.S.

CAD NO. 10103-DETAILS-PH1.dwg

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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

DETAILS

D1.2

SCALE: AS NOTED

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NOTE A
GATES FOR ONE WAY ROADS SHALL HAVE SINGLE ARM W/GATE ARM HOLDER TO LOCK GATE IN CLOSED POSITION.
NOTE B
ALL GATES TO BE "TILT-INSTALLED" TO FORCE THE GATE TO SWING OPEN OR CLOSED.
NOTE C
GATE FOR THIS PROJECT WILL HAVE ONLY A SINGLE ARM.
15'-9"
CENTER OF GATE ARM HOLDER
ANCHOR POST BASE PLATE, 1" HRS
EDGE OF PAVEMENT
1/2" H.R.S. PLATE
EDGE OF TRAIL PAVEMENT
2'-6"
TYP. BOTH SIDES
(4) 13/16" ϕ DRILLED HOLES
PLAN
STOP SIGN FACING TRAFFIC FLOW. INSTALL SIGN WITH STAINLESS BOLTS, NUTS, AND LOCK WASHERS
℄ OF STOP SIGN = TRAFFIC LANE (TYPICAL)
WRAP REFLECTIVE TAPE AROUND STEEL POSTS AS SHOWN, TYPICAL OF 4 PLACES
GATE POST
ANCHOR POST
WELD 1" ϕ RING TO GATE FOR LOCK
GATE POST BASE PLATE
4" X 2" X 0.120" WALL RECTANGULAR STRUCTURAL STEEL TUBING (TYP.)
WRAP REFLECTIVE TAPE AROUND STEEL TUBING AS SHOWN, TYPICAL OF 10 PLACES
ELEVATION
ANCHOR POST, 6" 40 STEEL PIPE
TOP: 3'-0" SQUARE
FINISH GRADE
(4) 3/4"ϕ X 16" ANCHOR BOLTS (4" THREAD) W/ ADJUSTING NUTS @ TOP PLATE
18" x 18" x 11GA" PLATE (SEE NOTE B ABOVE.)
CONCRETE FOOTING
BOTTOM: 3'-6" SQUARE
UNDISTURBED EARTH
COMPACTED C.S.T.C. BACK FILL (TYP.)
1" N.C. BOLT WELDED AT TOP TO ROD
4"ϕ x 1/2" H.R.S. PLATE
3'-10" x 1" ROD WITH 1 1/2" N.C. THREAD AT TOP, 2 1/4" AT BOTTOM
GATE POST, 4" SCH 40 STEEL PIPE, 3'-6" LONG
1/4" GAP
ONE 4"ϕ H.R.S. x 1/2" ROUND WASHER WITH 1 1/16"ϕ HOLE DRILLED AT CENTER. GRIND UPPER EDGE TO PROVIDE WELD CLEARANCE. APPLY GREASE BOTH SIDES
1" N.C. NUT
GATE POST SECTION
1 1/16" ϕ HOLE
4"ϕ x 1/2" H.R.S. PLATE
GATE POST END DETAIL
NOTE: TYPICAL FOR BOTH ENDS
1/2" H.R.S. PLATE
3 3/4" RADIUS
1 1/16" ϕ HOLE
3" RADIUS
TOP PLATE PLAN
NOT TO SCALE
1/2" X 1'-0" KEY WAY CENTERED IN STEEL TUBING
3' WOOD 2 X 4 SLIDE PLACED INSIDE GATE
1" ϕ RING BEYOND, WELDED TO GATE AT END OF KEY WAY
SIZE 0 WOOD THREAD SCREW EYE, SCREWED IN FOR HANDLE & LOCK
SECTION
3 5/16" RADIUS
1/2" H.R.S. PLATE
3/8" GUSSET
PLAN
1 1/16" ϕ HOLE
ELEVATION
GATE POST BASE PLATE
1/4"x 4" F.B.
HASP
ADJUST HEIGHT OF BAR HOLDER TO HOLD GATE ARM HORIZONTAL WHEN GATE IS IN OPEN POSITION. LOCATE 1' FROM END OF BAR.
4" X 2" X 0.120" WALL RECTANGULAR STRUCTURAL STEEL TUBING
12" ϕ CONCRETE FOOTING
GATE ARM HOLDER DETAIL
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ROCKY REACH TRAIL PHASE ONE
DETAILS
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SCALE: NOT TO SCALE
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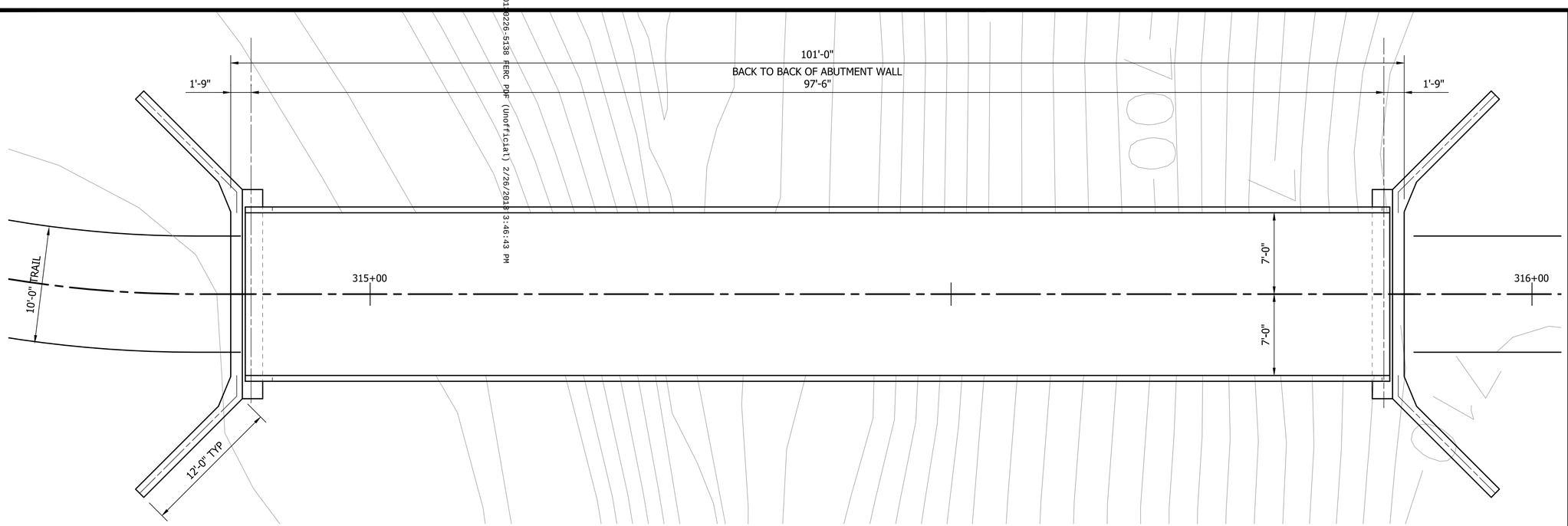
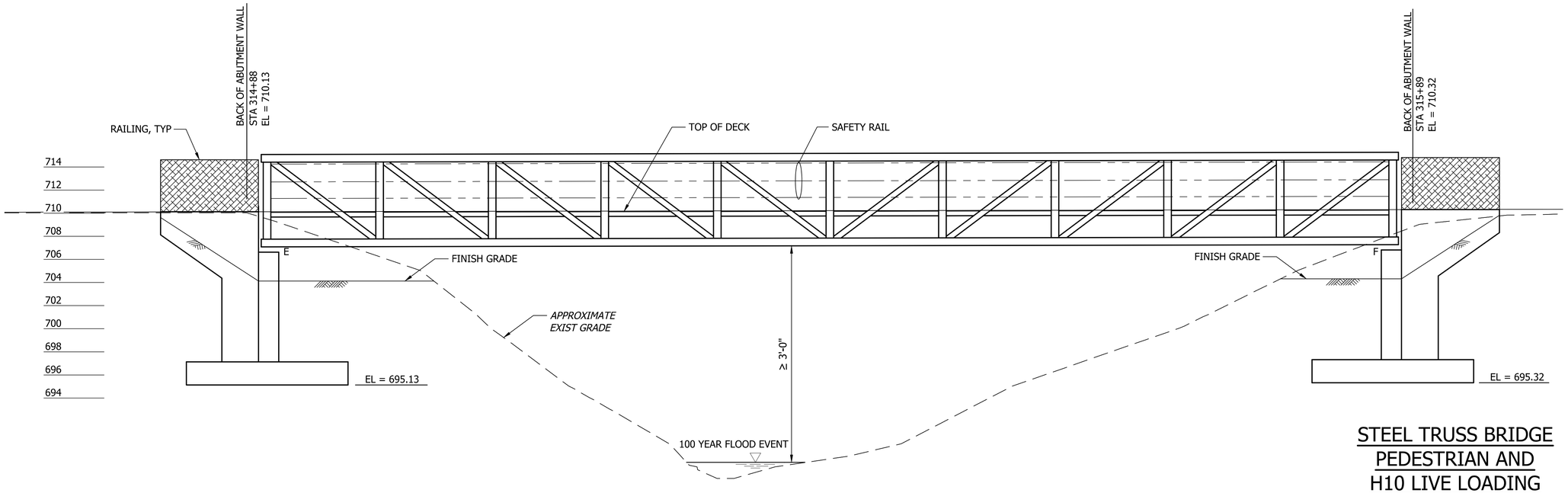


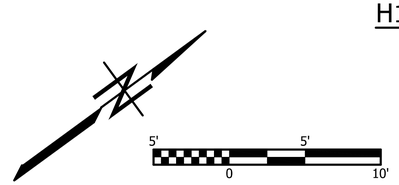
PLAN - SOUTH BRIDGE

BEARING OF ALL PIERS NORMAL TO CL BRIDGE



ELEVATION - SOUTH BRIDGE

STEEL TRUSS BRIDGE
PEDESTRIAN AND
H10 LIVE LOADING



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IZZAT H. HASAN
STATE OF WASHINGTON
26933
STRUCTURAL ENGINEER
PROFESSIONAL ENGINEER

REGISTERED STAMP

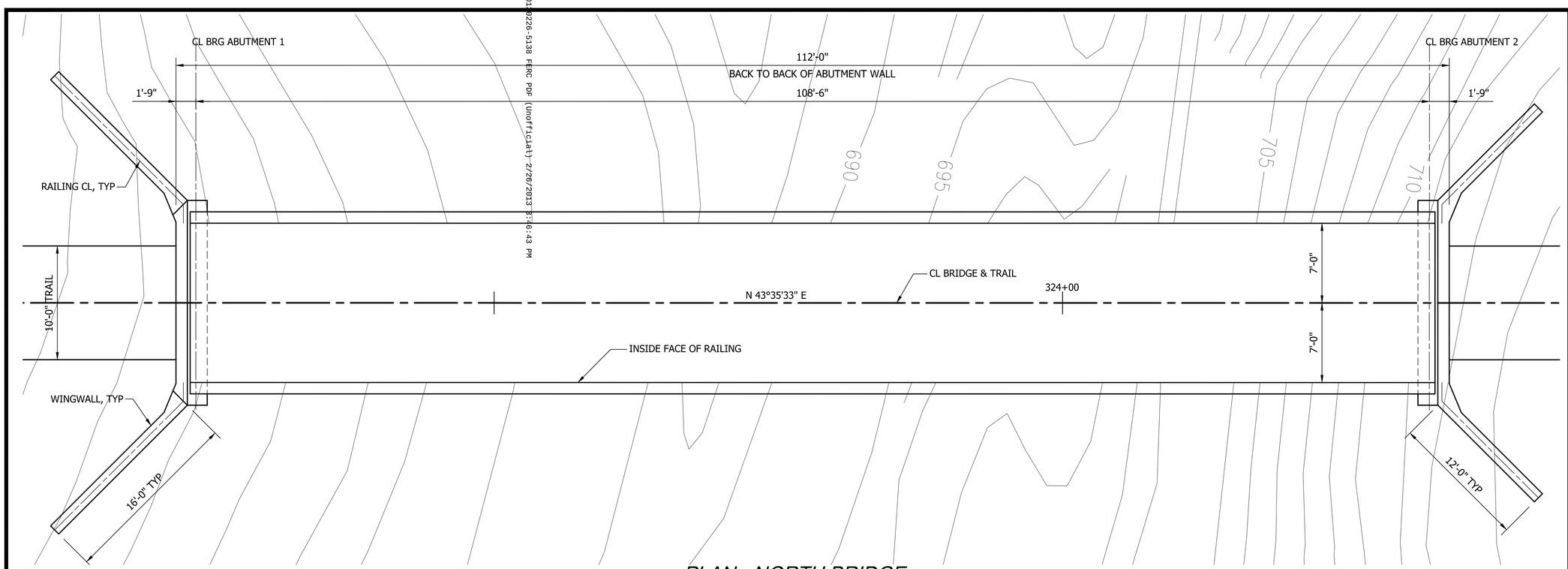
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

SOUTH BRIDGE
LAYOUT

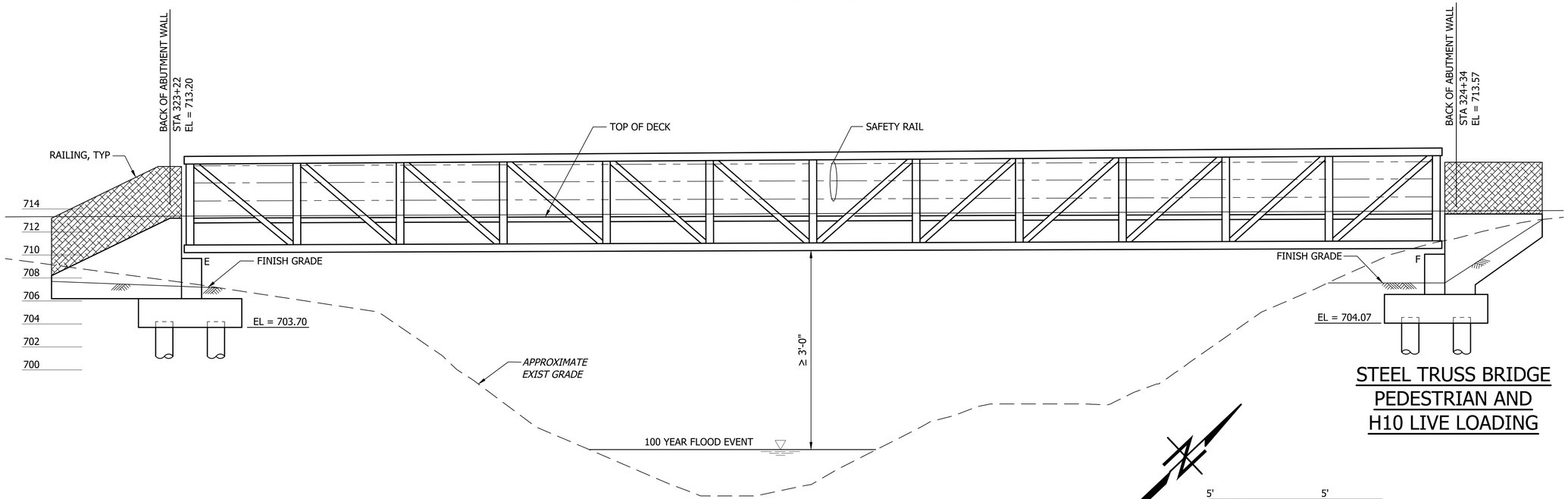
SHT. NO. S1.0

SCALE: AS SHOWN

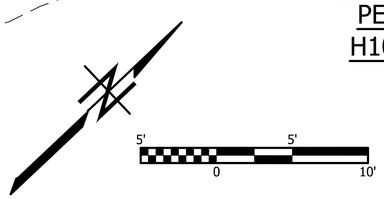


PLAN - NORTH BRIDGE

BEARING OF ALL PIERS NORMAL TO CL BRIDGE



ELEVATION - NORTH BRIDGE



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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

STEEL TRUSS BRIDGE
PEDESTRIAN AND
H10 LIVE LOADING

NORTH BRIDGE
LAYOUT

S2.0
SHT. NO.

SCALE: AS SHOWN

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GENERAL NOTES:

1. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE STATE OF WASHINGTON, DEPARTMENT OF TRANSPORTATION'S "STANDARD SPECIFICATIONS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION", DATED 2012 AND AMENDMENTS.

2. THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 4TH EDITION - DATED 2007 AND INTERIMS THRU 2008, AND AASHTO LRFD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES, 2ND EDITION, 2009.

3. SEISMIC DESIGN HAS BEEN COMPLETED WITH THE FOLLOWING PARAMETERS:

   - SITE CLASS - D
   - PGA = - 0.15g
   - Ss = - 0.336g
   - S_1= - 0.12g

4. DESIGN LOADS:

   DEAD LOAD:
   - CONCRETE - 160 PCF
   - STEEL - 490 PCF

   LIVE LOAD:
   - PEDESTRIAN - 90 PSF
   - TRUCK - H10

   EARTH SOIL LOADS:
   - SOIL DENSITY - 120 PCF
   - FRICTION ANGLE - 34°
   - ACTIVE EARTH PRESSURE - 35 PCF EQUIVALENT FLUID WEIGHT
   - ACTIVE PLUS SEISMIC PRESSURE - 40 PCF EQUIVALENT FLUID WEIGHT

5. SOIL RESISTANCE DESIGN PARAMETERS:
   - COEFFICIENT OF FRICTION AGAINST SLIDING - 0.5
   - ALLOWABLE SERVICE SOIL BEARING PRESSURE - 2,579 PSF WITH 1.5" SETTLEMENT
   - ULTIMATE SOIL BEARING CAPACITY - 44,945 PSF
   - SEE HWA GEOSCIENCES GEOTECHNICAL REPORT FOR AUGERCAST PILE CAPACITIES
   - PASSIVE EARTH PRESSURE - 275 PCF EQUIVALENT FLUID WEIGHT (FS = 1 S)

6. THE CONCRETE IN THE AUGERED CONCRETE PILES SHALL BE CLASS 4000P. ALL OTHER CAST-IN-PLACE CONCRETE SHALL BE CLASS 4000.

7. FALSEWORK SHALL BE CAREFULLY RELEASED TO PREVENT IMPACT OR UNDUE STRESS IN THE STRUCTURE.

8. MINIMUM CLEAR CONCRETE COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS:
   - 1 1/2" TYPICAL
   - 2" AT FORMED SURFACE PERMANENTLY EXPOSED TO EARTH
   - 3" CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH

9. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" UNLESS NOTED OTHERWISE.

10. ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO AASHTO M31, GRADE 60 UNLESS NOTED OTHERWISE.

LEGEND

- A / 20 — IDENTIFIES SECTION, VIEW OR DETAIL; TAKEN OR SHOWN ON BRIDGE SHEET NO.
- A / - — TAKEN OR SHOWN ON THE SAME SHEET
- E — DENOTES EPOXY COATED REINF

ABBREVIATIONS

DL	DEAD LOAD
E	EXPANSION
F	FIXED
K	KIPS
SPA	SPACED
WHS	WELDED HEADED STUD

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ROCKY REACH TRAIL
PHASE ONE

GENERAL NOTES

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SHT. NO. S3.0

SCALE: AS SHOWN

PREFABRICATED BRIDGE SPECIFICATIONS

1. DESCRIPTION

THE TERM "PREFABRICATED BRIDGE" AS USED HEREIN CONSTITUTES THE FULLY ENGINEERED STEEL BRIDGE STRUCTURE INCLUDING RAILING, DECK, BEARINGS, ANCHORAGE TO ABUTMENTS, ERECTED AS SHOWN ON THE PLANS AND IN ACCORDANCE WITH THESE SPECIFICATIONS.

2. QUALIFICATIONS

PREFABRICATED BRIDGE SUPPLIER MUST HAVE AT LEAST FIVE YEARS OF EXPERIENCE FABRICATING THIS TYPE OF STRUCTURES AND PROVIDE A LIST OF AT LEAST FIVE SUCCESSFUL BRIDGE PROJECTS.

PREFABRICATED STEEL FABRICATOR SHALL BE CERTIFIED UNDER THE AISC QUALITY CERTIFICATION PROGRAM, "MAJOR STEEL BRIDGES CATEGORY" AS SET FORTH IN THE AISC CERTIFICATION PROGRAM WITH FRACTURE CRITICAL ENDORSEMENT.

3. GENERAL FEATURES OF DESIGN

PREFABRICATED BRIDGES SHALL BE DESIGNED AS A CONTINENTAL® HALF-THROUGH PONY SYSTEM, MODEL NUMBER P1-SQ (OR EQUAL), THAT HAS ONE (1) DIAGONAL PER PANEL AND PLUMB VERTICAL MEMBERS. CONCEPTUAL TYPICAL SECTION IS SHOWN IN FIGURE 1.

ALL MEMBERS OF THE VERTICAL TRUSSES (TOP AND BOTTOM CHORDS, VERTICALS, AND DIAGONALS) SHALL BE FABRICATED FROM SQUARE AND/OR RECTANGULAR STRUCTURAL STEEL TUBING. OTHER STRUCTURAL MEMBERS AND BRACING SHALL BE FABRICATED FROM STRUCTURAL STEEL SHAPES OR SQUARE AND RECTANGULAR STRUCTURAL STEEL TUBING.

THE SOUTH BRIDGE AND NORTH BRIDGE SPANS ARE 97'-6", 108' -6", RESPECTIVELY AND SHALL BE AS MEASURED FROM THE CL OF BEARINGS FOR EACH BRIDGE STRUCTURE. BRIDGES WIDTH SHALL BE 14'-0" AND SHALL BE AS MEASURED FROM THE INSIDE FACE OF RAILING.

BRIDGES SHALL BE DESIGNED UTILIZING AN UNDERHUNG FLOOR BEAM (TOP OF FLOOR BEAM WELDED TO THE BOTTOM OF THE BOTTOM CHORD) OR BE DESIGNED UTILIZING AN H-SECTION CONFIGURATION WHERE THE FLOOR BEAMS ARE PLACED UP INSIDE THE TRUSSES AND ATTACHED TO THE TRUSS VERTICALS. THE TOP OF THE TOP CHORD SHALL NOT BE LESS THAN 54" ABOVE THE DECK SURFACE

BRIDGES SHALL BE DESIGNED AS A SINGLE SIMPLE SUPPORTED SPAN. BRIDGE BEARINGS AND ANCHORAGE TO ABUTMENTS SHALL BE DESIGNED FOLLOWING THE FIXITY CONDITIONS INDICATED IN FIGURE 2.

BRIDGES SHALL BE DESIGNED SUCH THAT THE UNFACTORED DEAD LOAD AND SEISMIC LATERAL BRIDGE REACTIONS AT THE ABUTMENTS DO NOT EXCEED THE VALUES SHOWN IN FIGURE 2.

BRIDGES SHALL HAVE A VERTICAL CAMBER DIMENSION AT MIDSPAN EQUAL TO 100% OF THE FULL DEAD LOAD DEFLECTION PLUS 1% OF THE FULL LENGTH OF THE BRIDGE.

BRIDGES SHALL BE FURNISHED WITH A STAY-IN-PLACE GALVANIZED STEEL FORM DECK SUITABLE FOR POURING A REINFORCED CONCRETE SLAB. THE FORM DECK SHALL BE DESIGNED TO CARRY THE DEAD LOAD OF THE WET CONCRETE, WEIGHT OF THE FORM DECKING, PLUS A CONSTRUCTION LOAD OF 20 PSF UNIFORM LOAD OR A 150 POUND CONCENTRATED LOAD ON A 1'-0" WIDE SECTION OF DECK. WHEN EDGE SUPPORTS ARE USED, DEFLECTION IS LIMITED TO 1/180 OF THE SPAN OR 3/4", WHICHEVER IS LESS. WITHOUT EDGE SUPPORTS, DEFLECTION SHALL BE LIMITED TO 1/180 OF THE SPAN OR 3/8", WHICHEVER IS LESS.

STEEL SAFETY RAILS SHALL BE PLACED ON THE STRUCTURE AT A HEIGHT OF 42" ABOVE THE DECK SURFACE. STEEL SAFETY RAILS SHALL BE PLACED SO AS TO PREVENT A 4" SPHERE FROM PASSING THROUGH THE TRUSS. STEEL SAFETY RAILS SHALL BE PLACED ON THE INSIDE OF THE STRUCTURE AT THE BRIDGE FABRICATOR'S OPTION. STEEL SAFETY RAILS PLACED SHALL HAVE THEIR ENDS SEALED AND GROUND SMOOTH SO AS TO PRODUCE NO SHARP EDGES.

BRIDGES SHALL BE SUPPLIED WITH A 1"X 5-1/2" TIMBER RUBRAILS (ACTUAL SIZE) ATTACHED FLUSH TO THE INSIDE FACE OF THE BRIDGE TRUSS VERTICALS AND FASTENED WITH TWO CARRIAGE BOLTS AT EACH SUPPORT LOCATION. THE SPAN OF THE RUBRAIL FROM CENTERLINE TO CENTERLINE OF SUPPORT SHALL NOT EXCEED 6'-6'. THE TOP OF THE TIMBER RUBRAIL SHALL BE 4' - 0" ABOVE THE TOP OF THE DECK (MEASURED AT THE OUTSIDE EDGE OF THE DECK).

BRIDGES SUPERSTRUCTURES SHALL BE DESIGNED FOR EACH OF THE FOLLOWING LIVE LOADS: PEDESTRIAN LIVE LOAD OF 90 PSF OF BRIDGE WALKWAY AREA; H10 VEHICULAR LOADS; AND A 1000 LB CONCENTRATED EQUESTRIAN LOAD PLACED ON A 4" X 4" DECK AREA.

BRIDGE SHALL BE DESIGNED TO ACCOMMODATE A TEMPERATURE RANGE OF -30° F TO 120° F. AT LEAST 1" CLEARANCE SHALL BE PROVIDED BETWEEN THE BRIDGE AND THE CONCRETE ABUTMENT BACKWALL.

FOR ADDITIONAL RAILING, WIND, SNOW, AND SEISMIC LOADING REFER TO THE DESIGN CODES UNDER SECTION 3 OF THESE SPECIFICATIONS. FOR SEISMIC SITE PARAMETERS REFER TO SHEET S3.0 IN THE PLANS.

4. DESIGN CODES

AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 4TH EDITION - DATED 2007 AND INTERIMS THRU 2008

AASHT0 LRFD GUIDE SPECIFICATIONS FRO THE DESIGN OF PEDESTRIAN BRIDGES, 2ND EDITION, 2009

5. MATERIALS

PROVIDE MATERIALS AND CONSTRUCT PREFABRICATED BRIDGES IN ACCORDANCE WITH THE DETAILS SHOWN ON THE PLANS, THE REQUIREMENTS OF THIS ITEM, AND THE PERTINENT REQUIREMENTS OF WSDOT STANDARD SPECIFICATIONS.

5.1 STEEL

BRIDGES SHALL BE FABRICATED FROM HIGH STRENGTH, LOW ALLOY, ATMOSPHERIC CORROSION RESISTANT ASTM A847 COLD-FORMED WELDED SQUARE AND RECTANGULAR TUBING AND/OR ASTM A588, OR ASTM A242, ASTM A606 PLATE AND STRUCTURAL STEEL SHAPES (FY = 50 KSI).
THE MINIMUM CORROSION INDEX OF ATMOSPHERIC CORROSION RESISTANT STEEL, AS DETERMINED IN ACCORDANCE WITH ASTM G101, SHALL BE 6.0.

THE MINIMUM THICKNESS OF ALL STRUCTURAL STEEL MEMBERS SHALL BE 3/16" NOMINAL AND BE IN ACCORDANCE WITH THE AISC MANUAL OF STEEL CONSTRUCTIONS' "STANDARD MILL PRACTICE GUIDELINES". FOR ASTM A500 AND ASTM A847 TUBING, THE SECTION PROPERTIES USED FOR DESIGN SHALL BE PER THE STEEL TUBE INSTITUTE OF NORTH AMERICA'S HOLLOW STRUCTURAL SECTIONS "DIMENSIONS AND SECTION PROPERTIES".

FRACTURE TOUGHNESS SHALL BE INCLUDED IN THE MATERIAL REQUIREMENTS PER AASHTO-LRFD BRIDGE DESIGN SPECIFICATIONS SECTION 6.6.2 WITH TEMPERATURE ZONE 2.

5.2 CONCRETE DECK

THE FORM DECK SHALL BE EITHER SMOOTH OR COMPOSITE. THE FORM DECK MATERIAL SHALL BE SUPPLIED IN ACCORDANCE WITH ASTM A653 AND GALVANIZED TO A MINIMUM G90 COATING WEIGHT. THE DECK SHALL BE CONSTRUCTED USING CONCRETE CLASS 4000D PER WSDOT STANDARD SPECIFICATIONS

5.3 TIMBER RUBRAIL

THE TIMBER RUBRAIL MATERIAL SHALL BE NATURALLY DURABLE HARDWOOD IPE (TABEBUIA SPP LAPACHO GROUP) RUBRAIL. RUBRAIL SHALL BE PARTIALLY AIR DRIED TO A MOISTURE CONTENT OF 15% TO 20%, SHALL BE SUPPLIED S4S (SURFACED FOUR SIDES), E4E (EASED FOUR EDGES), WITH THE EDGES EASED TO A RADIUS OF 1/8". MEASURED AT 30% MOISTURE CONTENT, THE WIDTH AND THICKNESS SHALL NOT VARY FROM SPECIFIED DIMENSIONS BY MORE THAN 0.04 INCHES. ENDS OF EACH PIECE SHALL BE SEALED WITH "ANCHORSEAL" MOBIL CER-M OR AN EQUAL AQUIOUS WAX LOG SEALER.

6. CONSTRUCTION

FABRICATE AND INSTALL PREFABRICATED BRIDGES IN ACCORDANCE WITH THIS SPECIFICATION AND THE DETAILS AND DIMENSIONS SHOWN ON THE PLANS OR APPROVED IN WRITING BY THE ENGINEER. LOCATE PREFABRICATED BRIDGES AS SHOWN ON THE PLANS.

7. SUBMITTALS

FURNISH 6 SETS OF SUBMITTAL DRAWINGS AND CALCULATIONS OF THE PREFABRICATED STEEL BRIDGES AND BEARING ANCHOR BOLTS TO THE ENGINEER AT THE PROJECT ADDRESS. DO NOT BEGIN WORK BEFORE THE SUBMITTALS ARE APPROVED BY THE ENGINEER.

8. MEASUREMENT

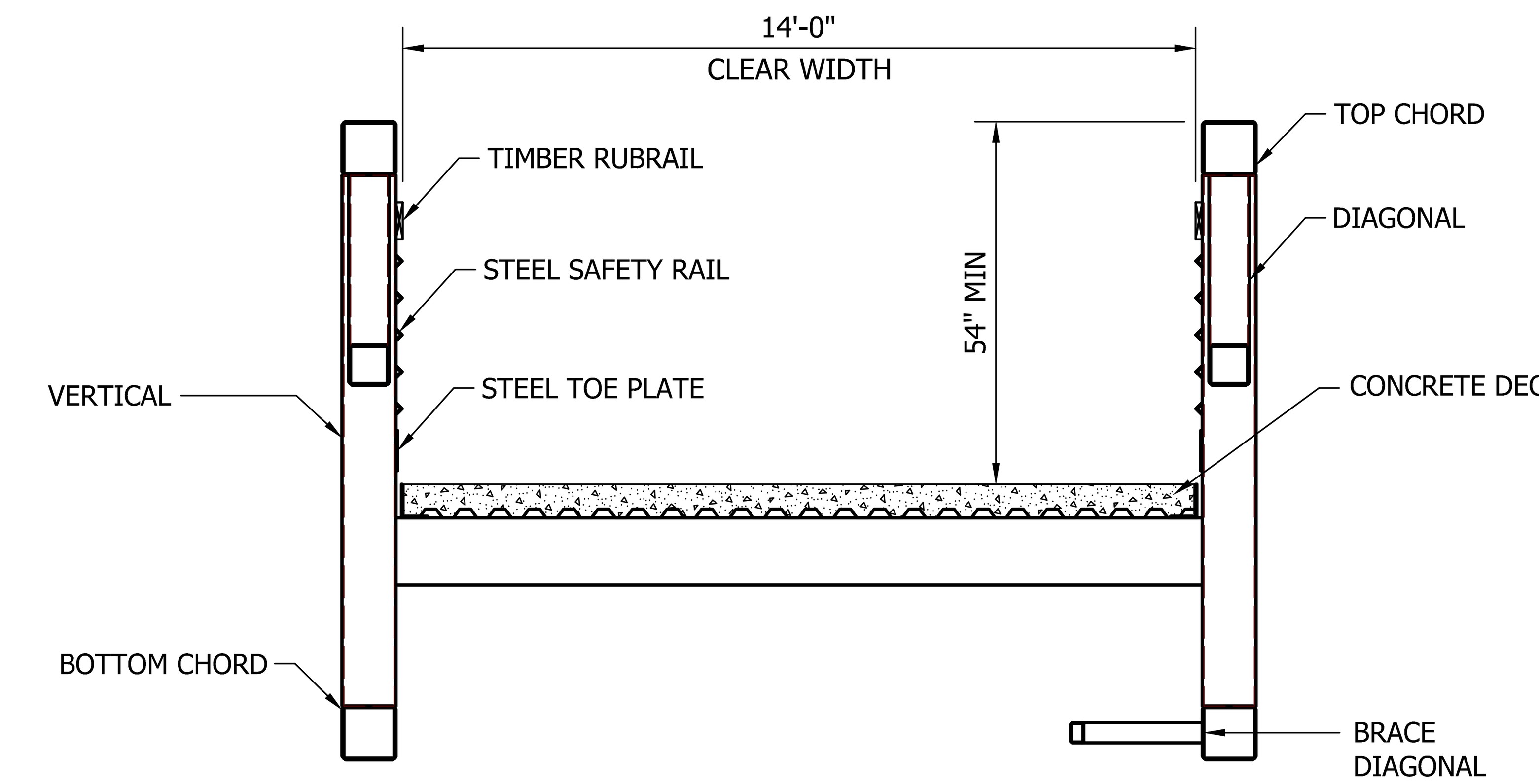
THIS ITEM WILL BE MEASURED BY LUMP SUM COMPLETE IN PLACE.

9. PAYMENT

THE WORK PERFORMED AND MATERIALS FURNISHED IN ACCORDANCE WITH THIS ITEM AND MEASURED AS PROVIDED UNDER "MEASUREMENT", WILL BE PAID FOR AT THE UNIT PRICE BID FOR "PRE-FABRICATED BRIDGE". THIS PRICE IS FULL COMPENSATION FOR FURNISHING, INSTALLING AND TESTING; AND FOR ALL LABOR, TOOLS, EQUIPMENT AND INCIDENTALS.

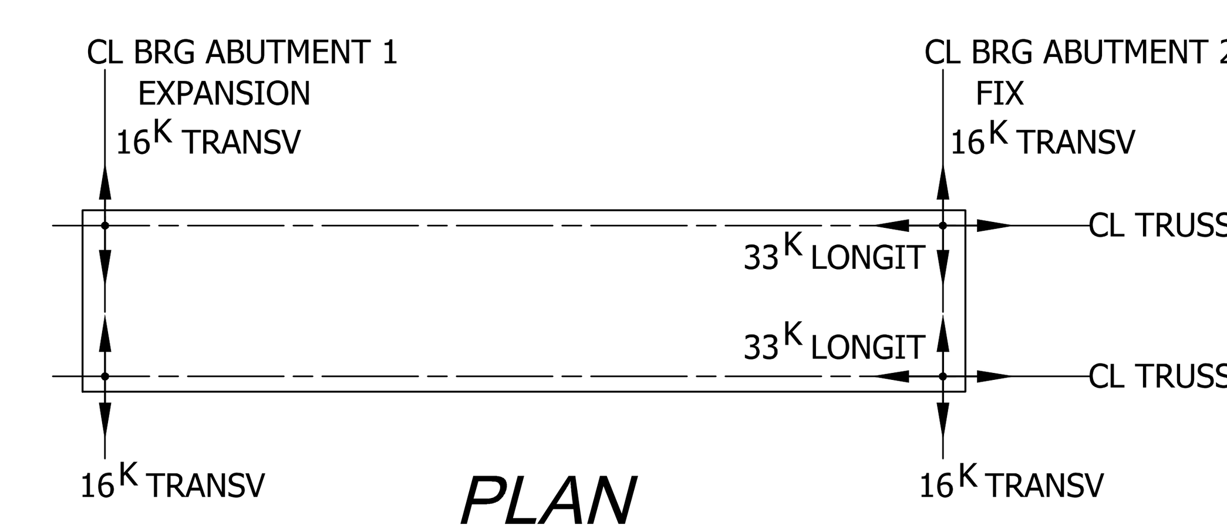
10. WARRANTY

THE MANUFACTURER SHALL PROVIDE A WARRANTY AGAINST DEFECTS IN MATERIAL AND WORKMANSHIP FOR A PERIOD OF 15 YEARS.



CONCEPTUAL TYPICAL SECTION

FIGURE 1



PLAN

MAXIMUM LATERAL LOAD REACTIONS

SEISMIC

ELEVATION

MAXIMUM VERTICAL DL REACTIONS

REACTIONS ARE FOR ENTIRE BRIDGE

FIGURE 2

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IZZAT H. HASAYEN, STATE OF WASHINGTON, 26933, REGISTERED STRUCTURAL ENGINEER

REGISTERED STAMP

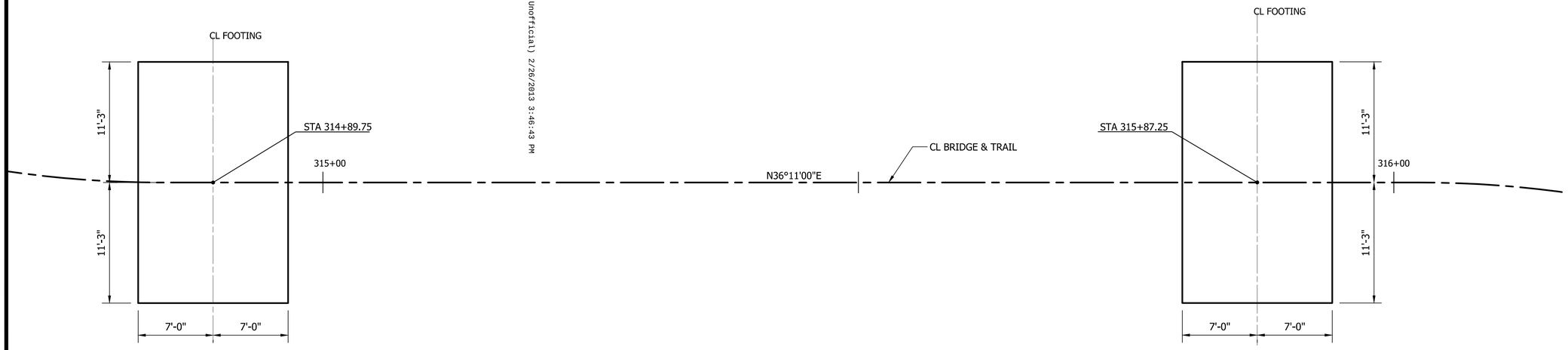
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

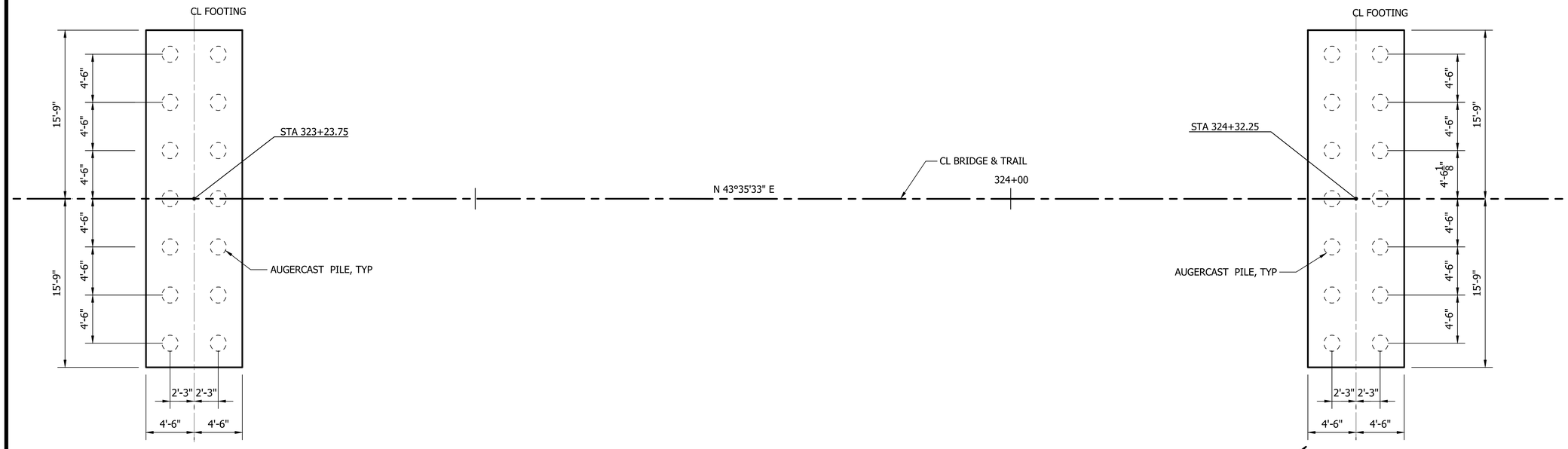
PREFABRICATED BRIDGE SPECIFICATIONS

SHT. NO. S3.1

SCALE: AS SHOWN



PLAN - SOUTH BRIDGE



PLAN - NORTH BRIDGE

CAD NO. L450-2010-E01-04SPLAN.DWG		
		DATE
		APP.
		INT.
		REVISIONS
		NO.

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26933
STRUCTURAL ENGINEER
PROFESSIONAL ENGINEER

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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

FOUNDATION LAYOUT

S4.0
SHT. NO.

SCALE: AS SHOWN

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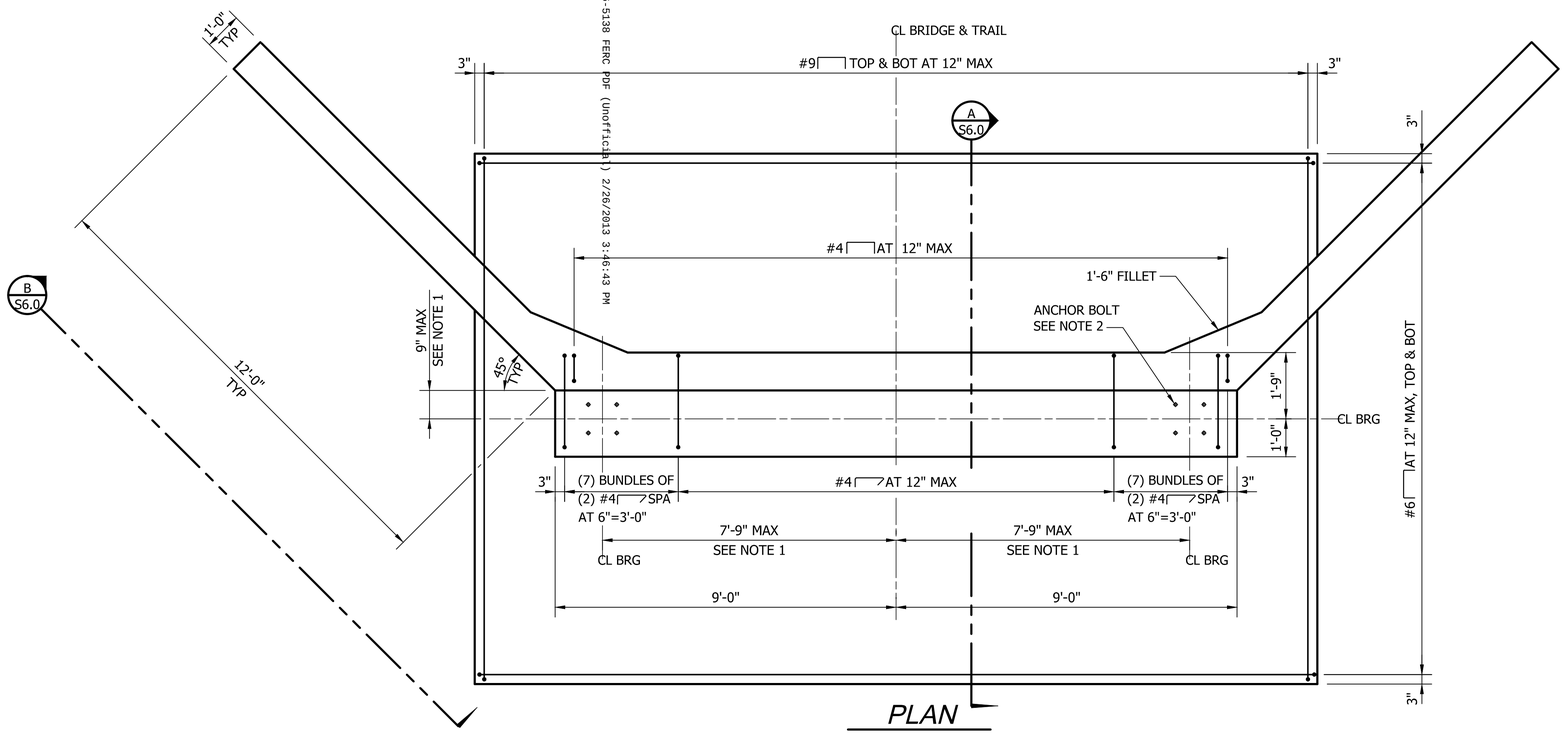
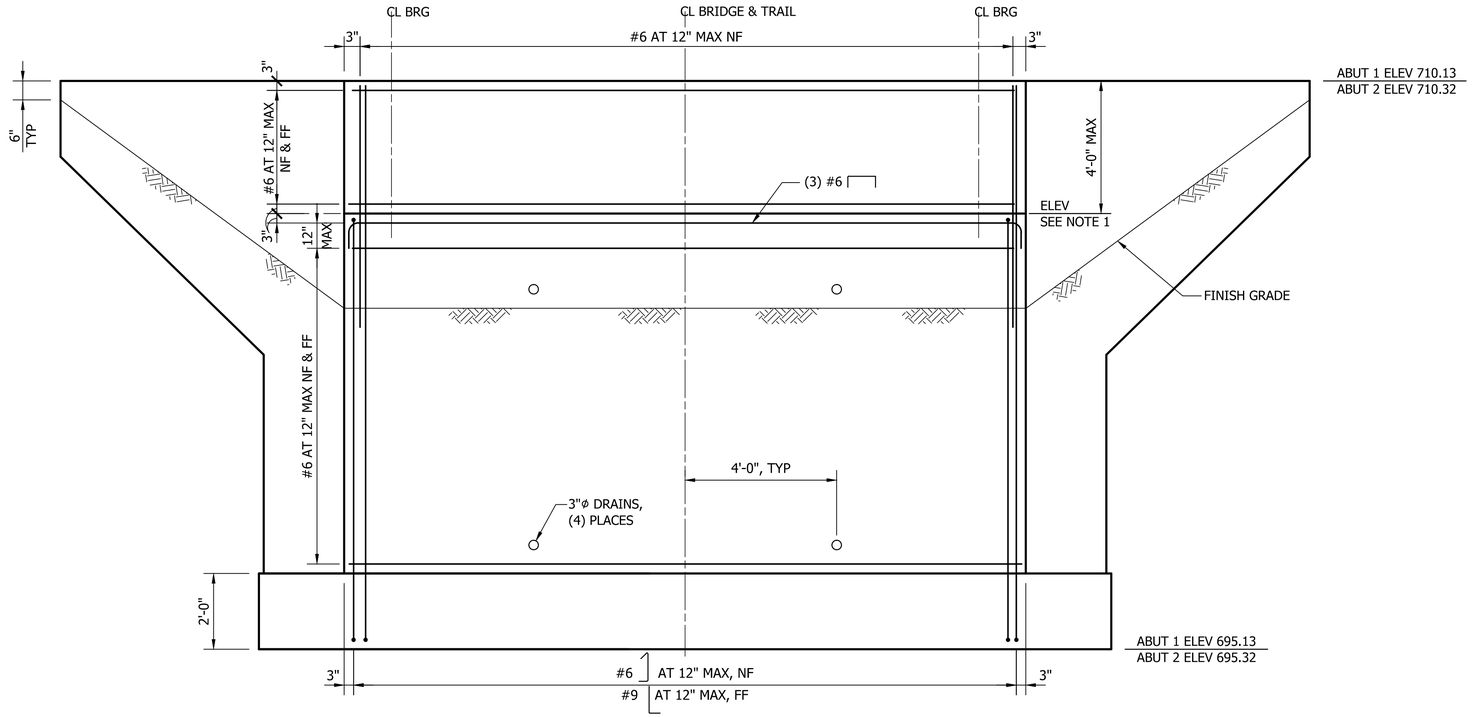


PLAN



ELEVATION

NOTES:

1. CONTRACTOR TO COORDINATE ELEVATION OR DIMENSION WITH BRIDGE SUPPLIER.
2. CONTRACTOR TO COORDINATE BRIDGE BEARING ANCHOR BOLT TYPE, SIZE AND LOCATION WITH BRIDGE SUPPLIER.
3. SEE 3/S14.0 FOR RAILING.
4. SEE 5/S14.0 FOR EXPANSION JOINT DETAIL.

1' 4'
0 2'

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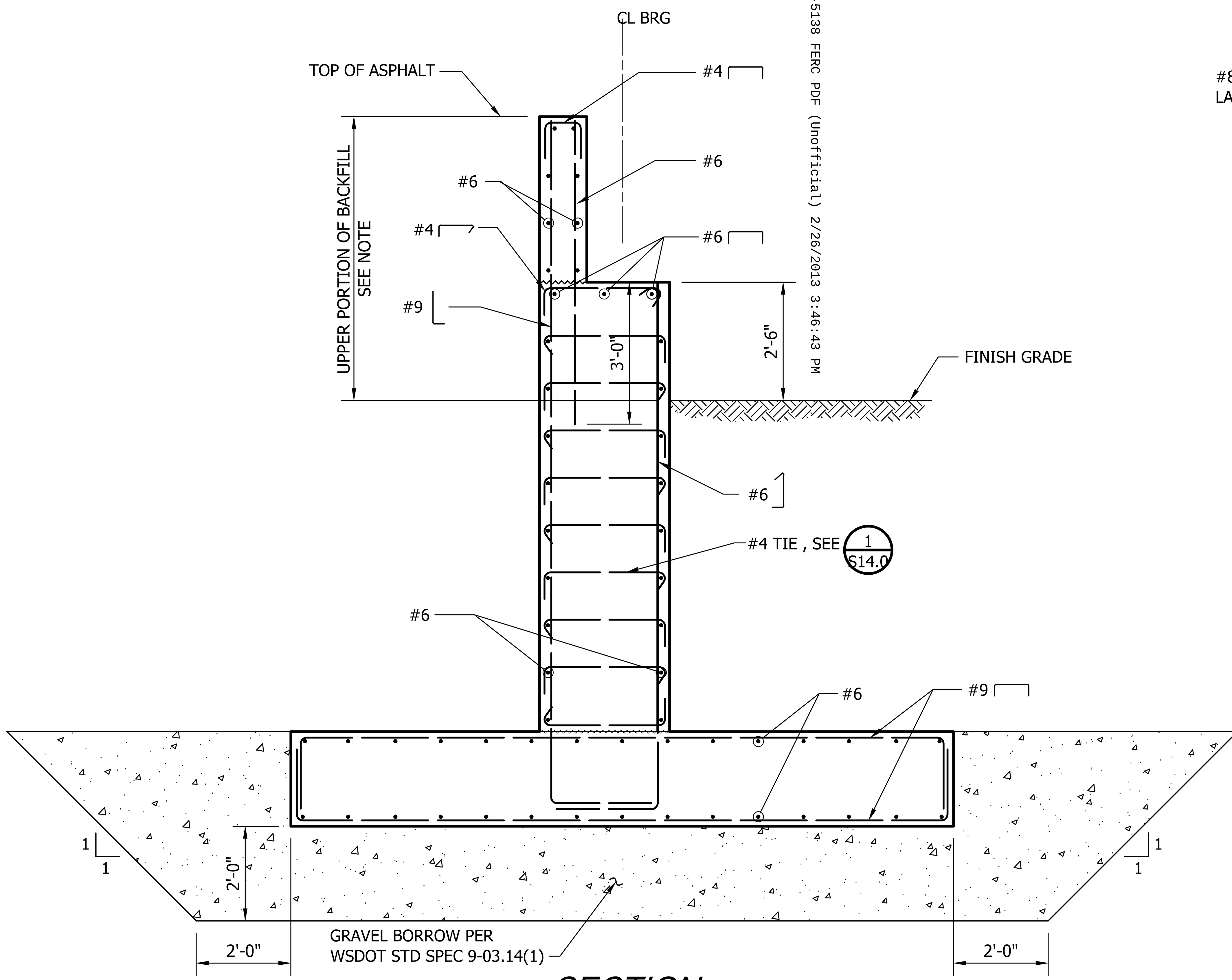
ROCKY REACH TRAIL PHASE ONE

SOUTH BRIDGE ABUTMENT PLAN & ELEVATION

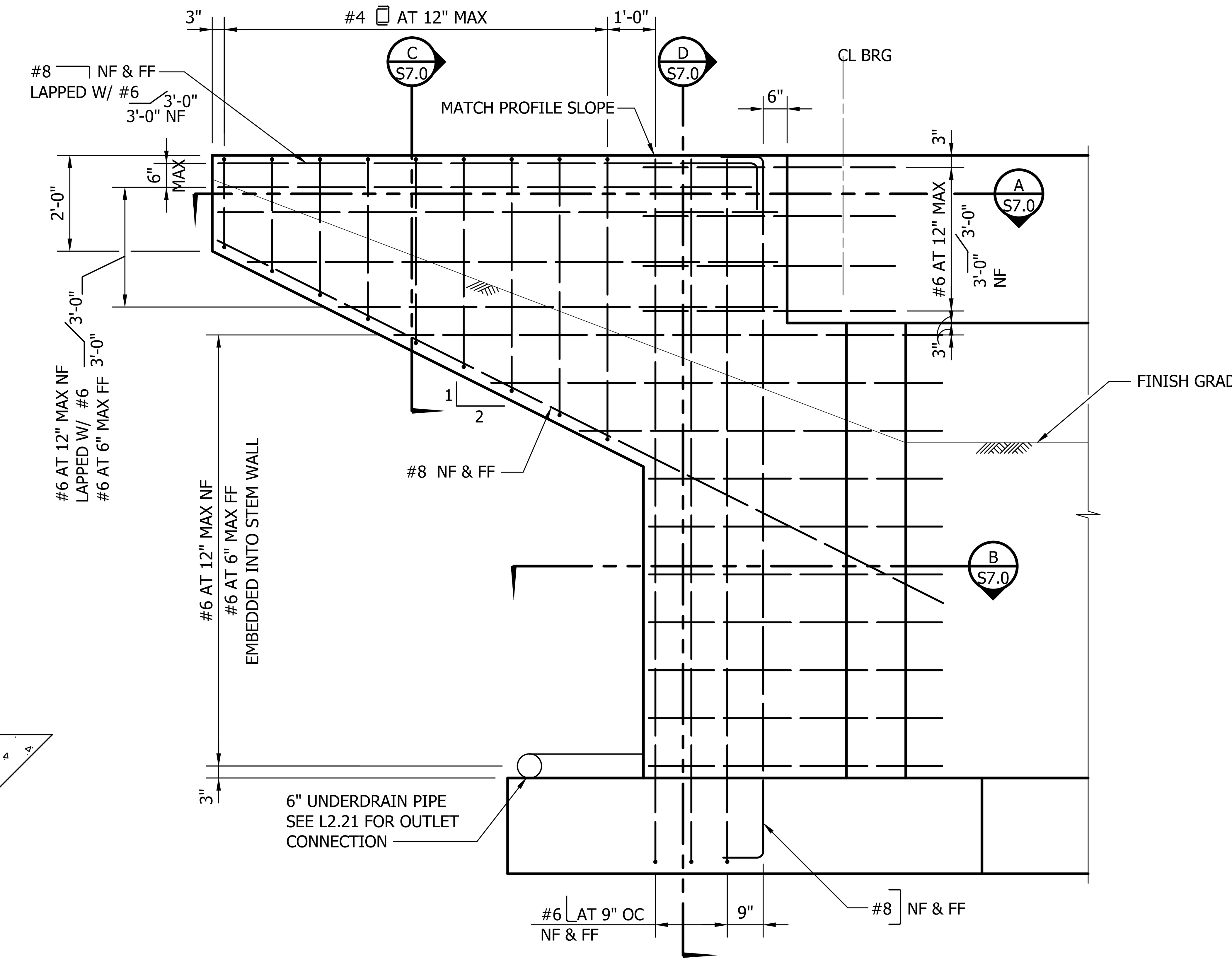
SHT. NO. S5.0

SCALE: AS SHOWN

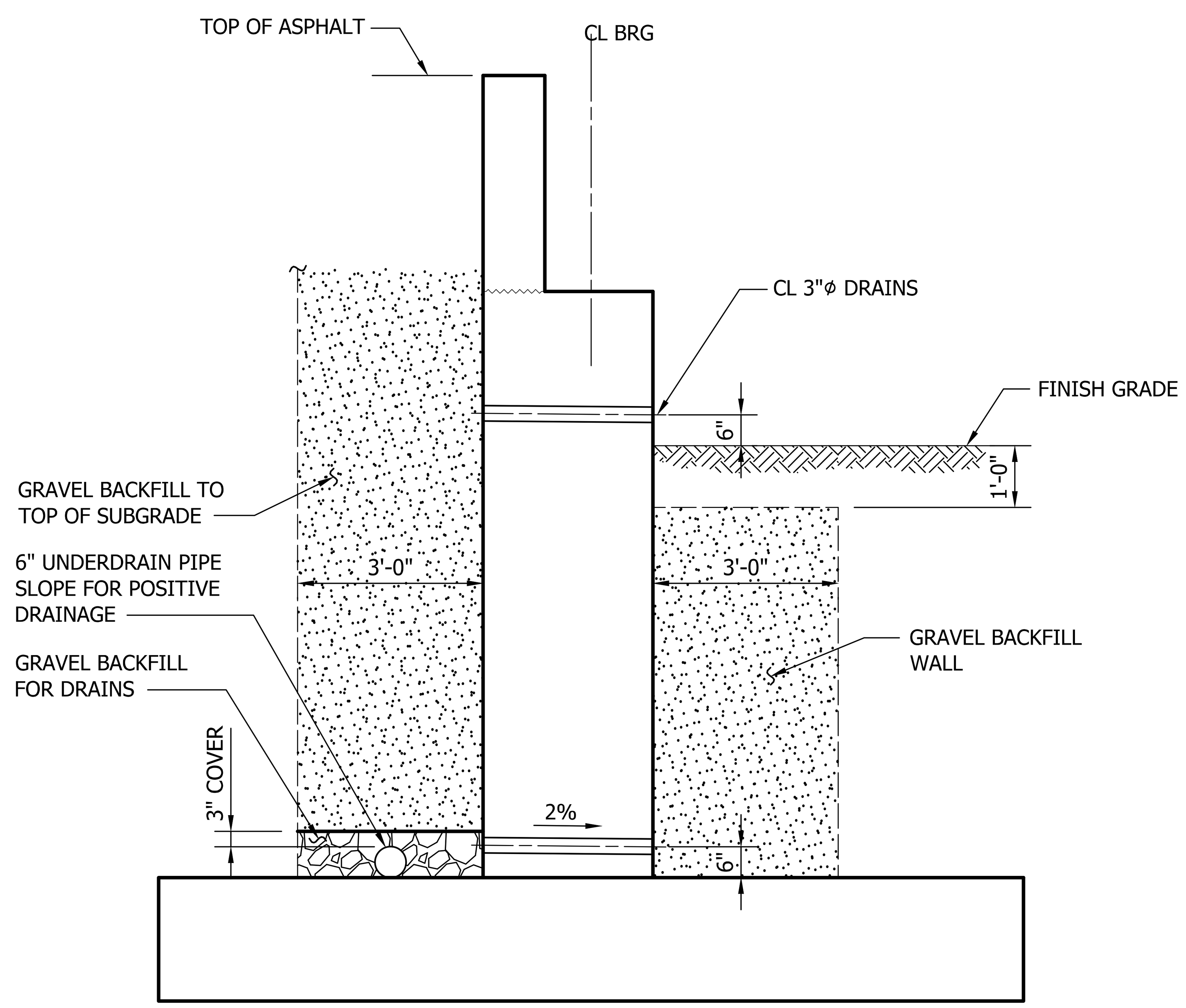


A S5.0 SECTION

NOTE: PLACE UPPER PORTION OF BACKFILL AFTER COMPLETING FINAL GRADING IN FRONT OF ABUTMENT WALL



B S5.0 VIEW



1 - DRAINAGE DETAIL

1' 0 2'

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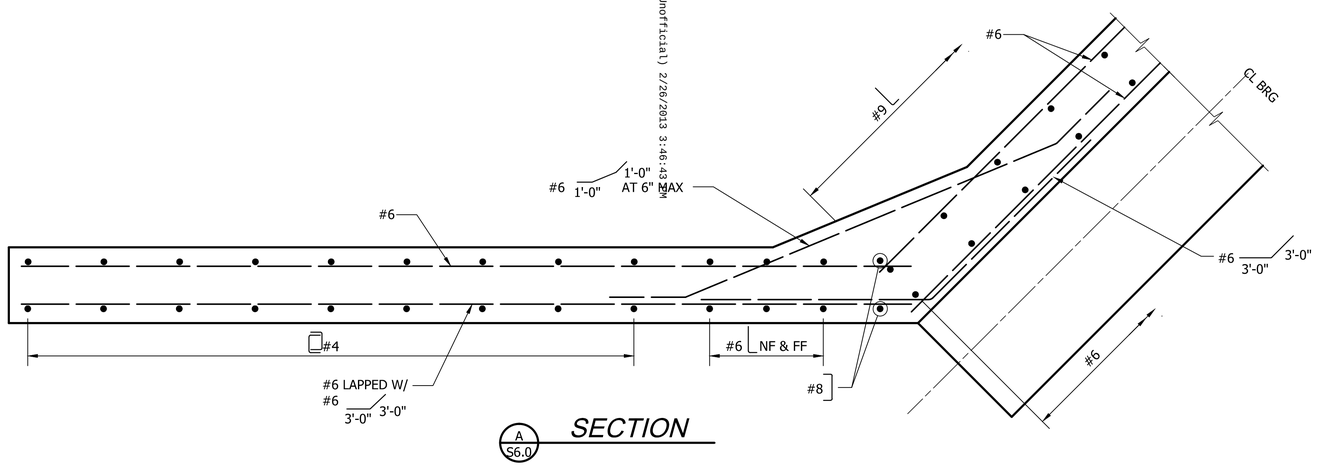
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

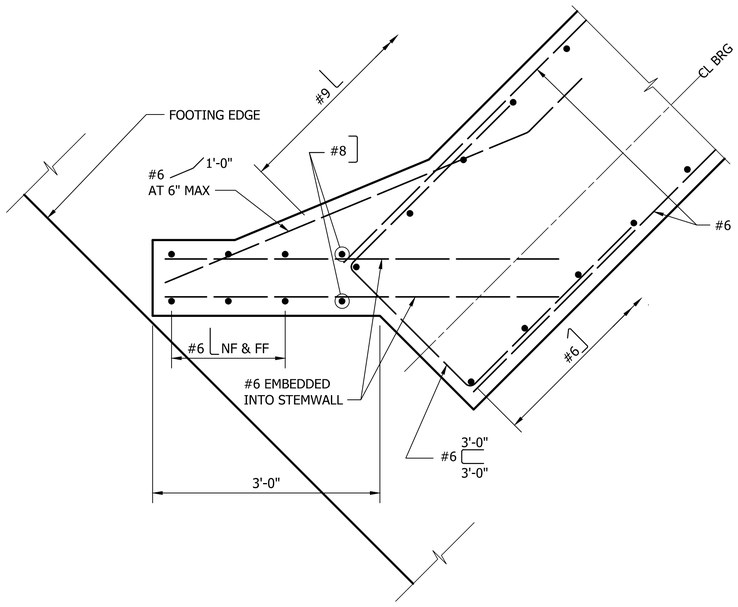
SOUTH BRIDGE ABUTMENT SECTIONS 1

SHT. NO. S6.0

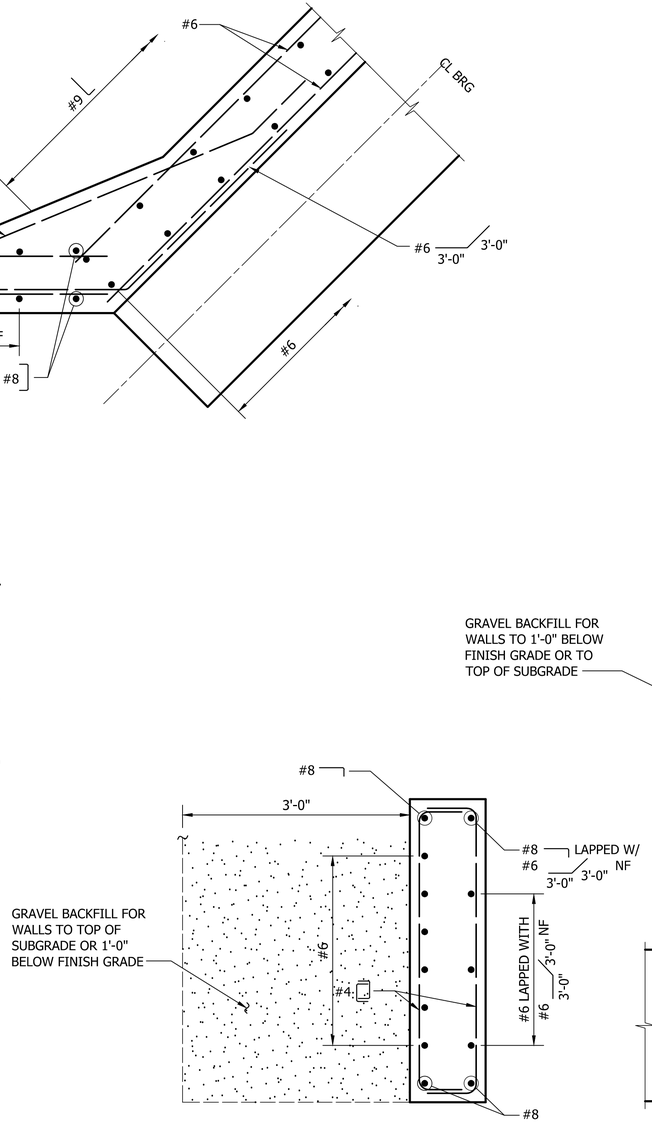
SCALE: AS SHOWN



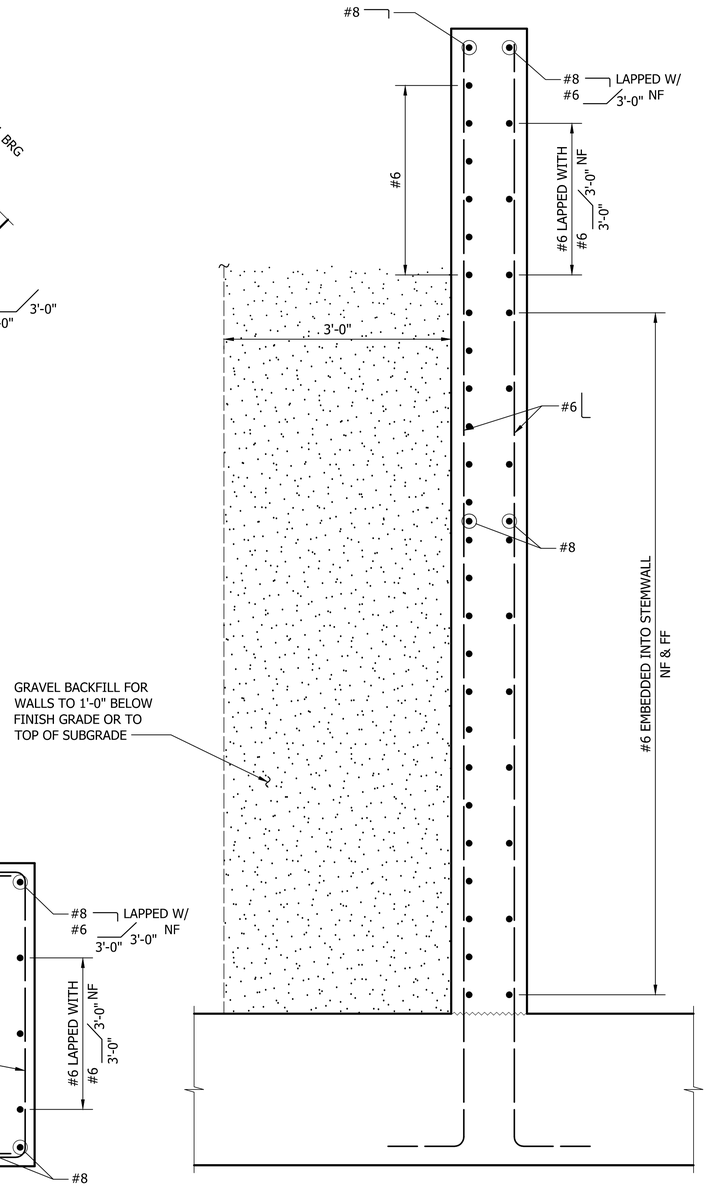
A S6.0 SECTION



B S6.0 SECTION



C S6.0 S11.0 SECTION



D S6.0 SECTION

1' 0 2'

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CAD NO. L450-2010-E01-07SSECT.DWG

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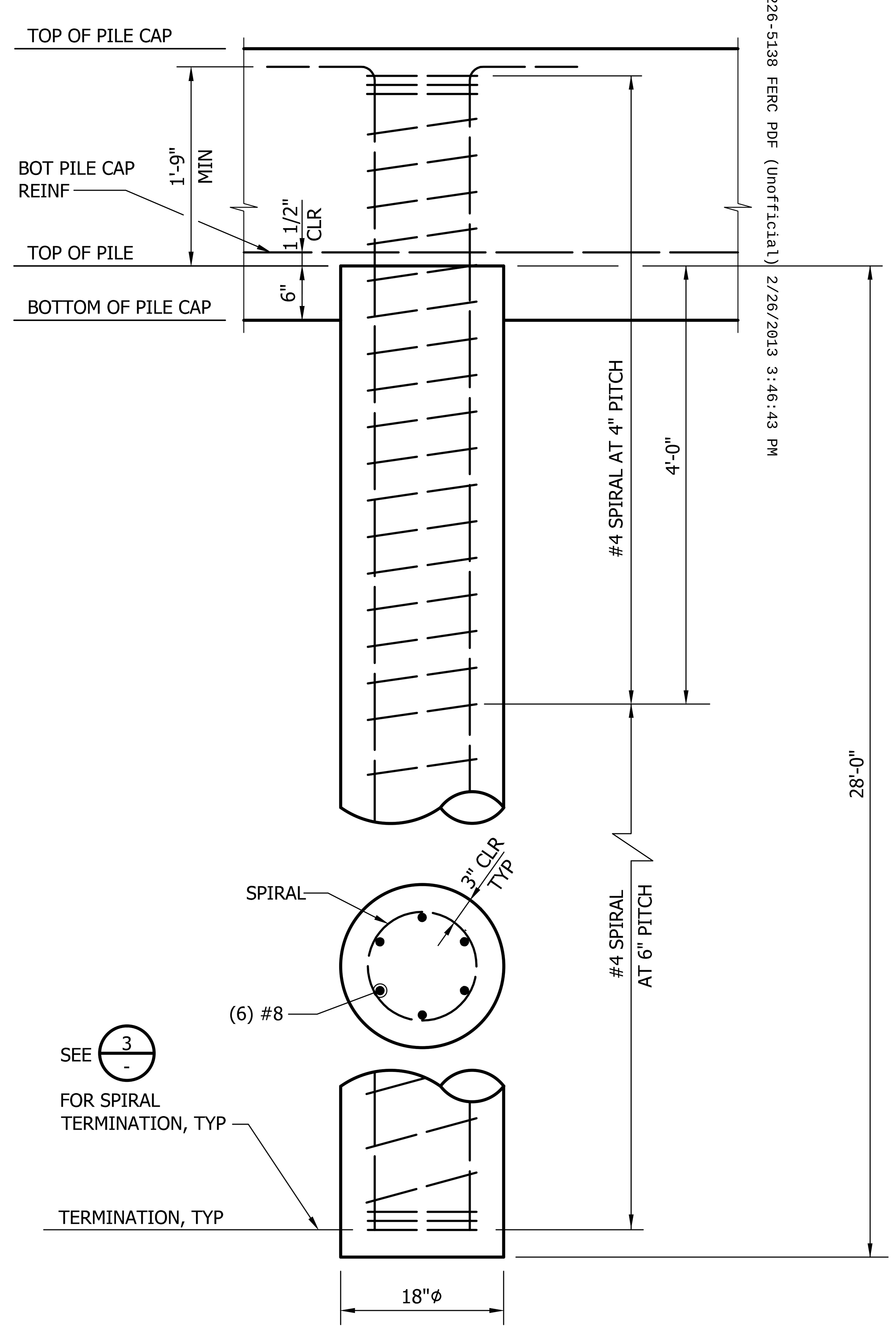
ROCKY REACH TRAIL
PHASE ONE

SOUTH BRIDGE
ABUTMENT
SECTIONS 2

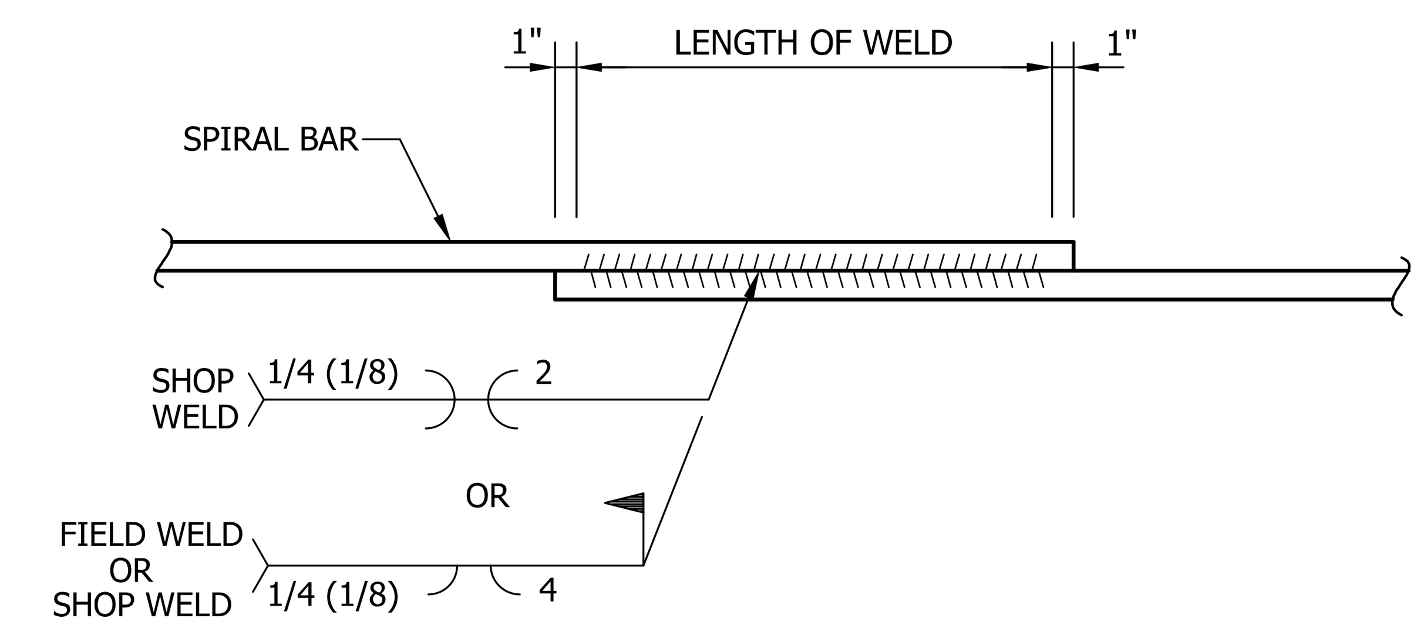
S7.0
SHT. NO.

SCALE: AS SHOWN



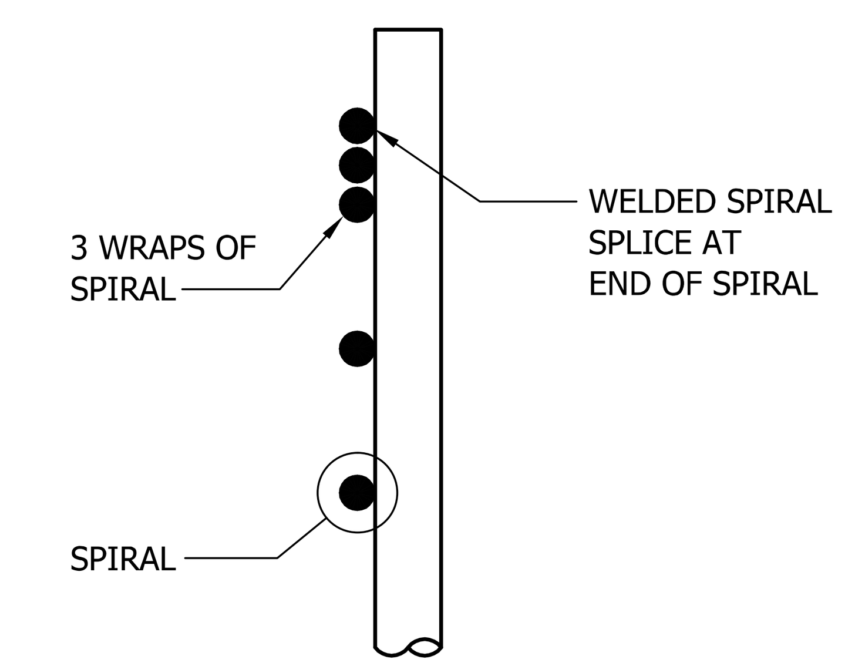
1 - AUGER CAST PILE ELEVATION

0 1' 2'



2 - WELDED SPIRAL SPLICE DETAIL

NO SCALE



3 - DETAIL

NOTES ON SPIRALS:

1. SPIRALS SHALL BE FABRICATED FROM #4 DEFORMED REINFORCING BARS (ASTM A706).
2. WELDED SPLICES ONLY, EXCEPT AS NOTED.

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CAD NO. L450-2010-E01-08SPILEDET.DWG

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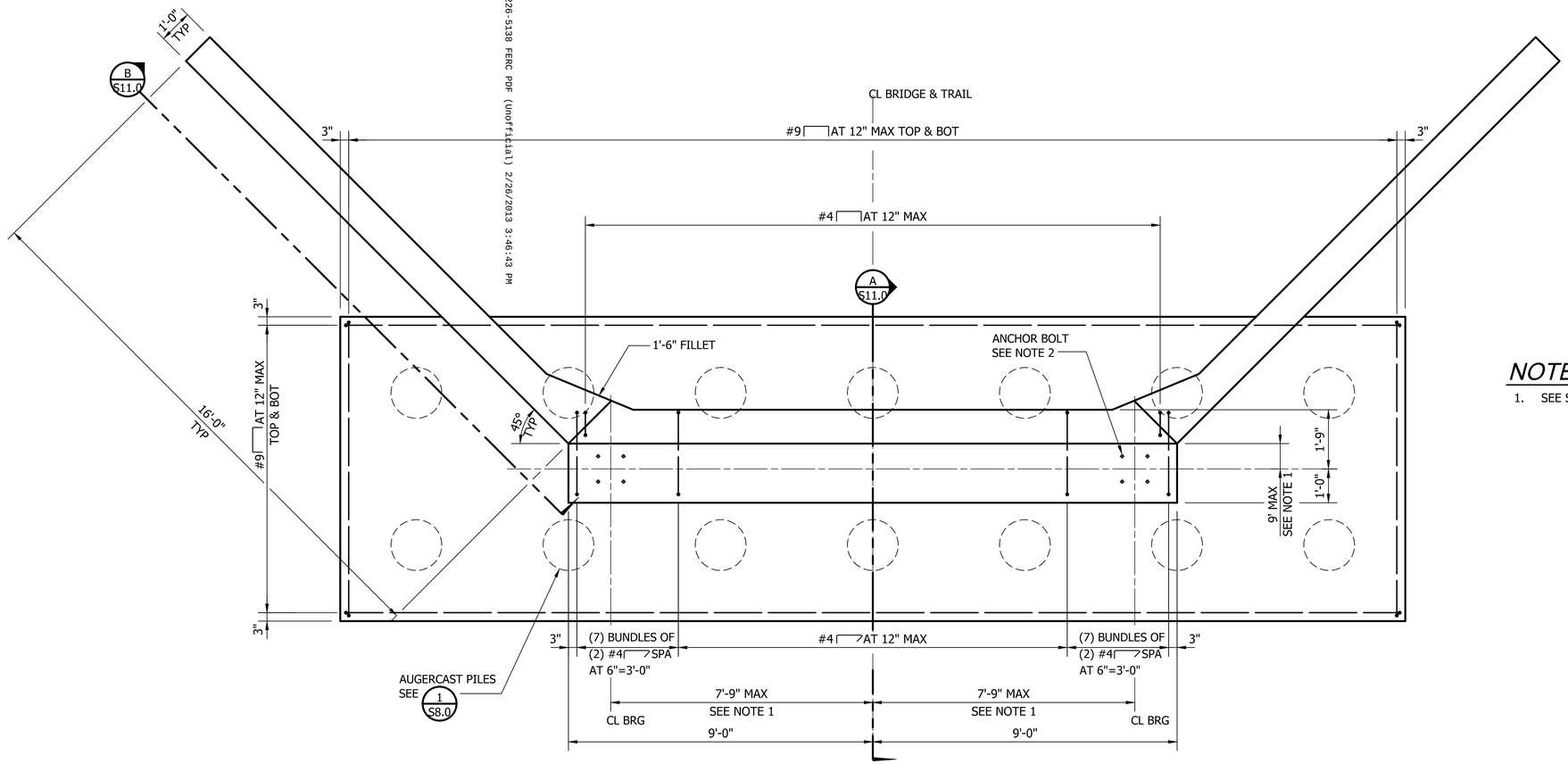
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL PHASE ONE

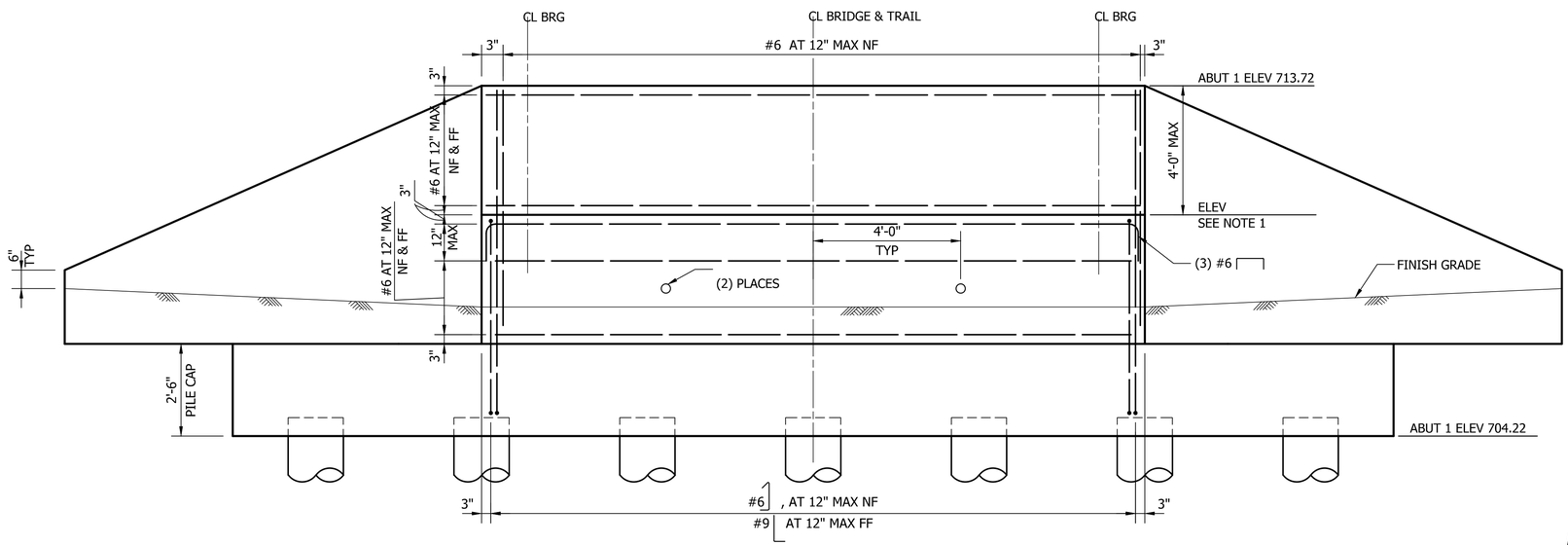
NORTH BRIDGE AUGERCAST PILE DETAILS

SHT. NO. S8.0

SCALE: AS SHOWN



PLAN



ELEVATION

NOTES:

1. SEE SHEET S5.0 FOR NOTES.

1' 4'
0 2'

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REGISTERED STAMP

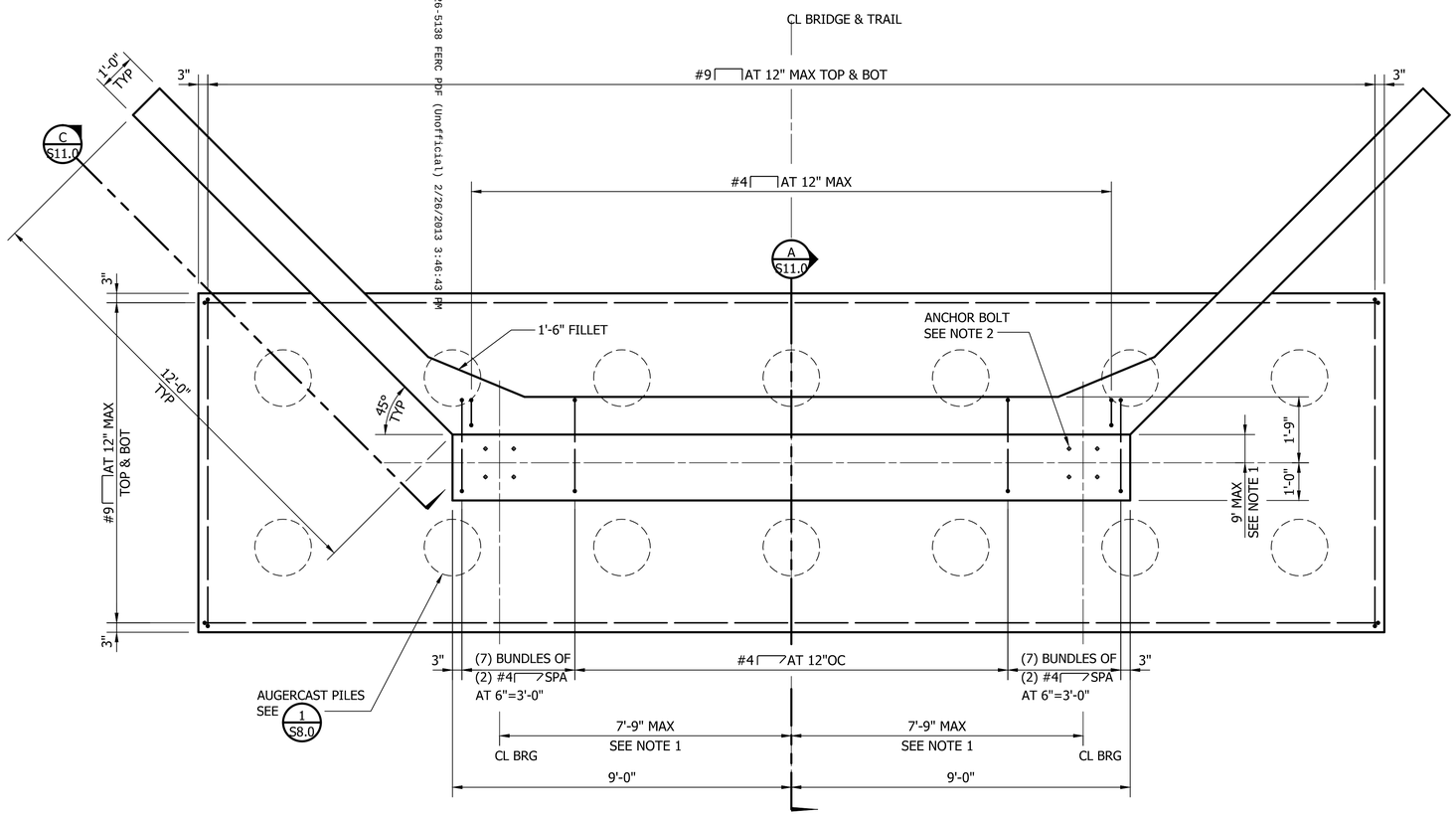
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

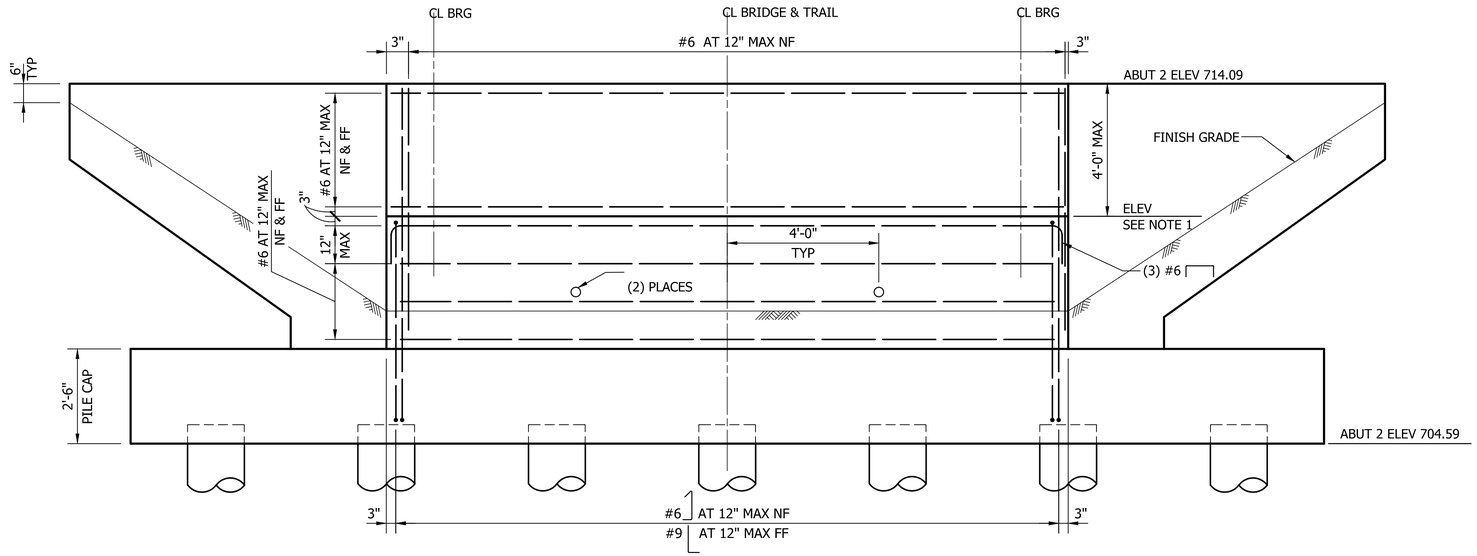
NORTH BRIDGE
ABUTMENT 1
PLAN & ELEVATION

S9.0
SHT. NO.

SCALE: AS SHOWN



PLAN



ELEVATION

NOTES:

1. SEE SHEET S5.0 FOR NOTES.

CAD NO. L450-2010-E01-010SPLNELV.DWG		
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		INT.
		REVISIONS
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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

NORTH BRIDGE
ABUTMENT 2
PLAN & ELEVATION

1' 0 2' 4'

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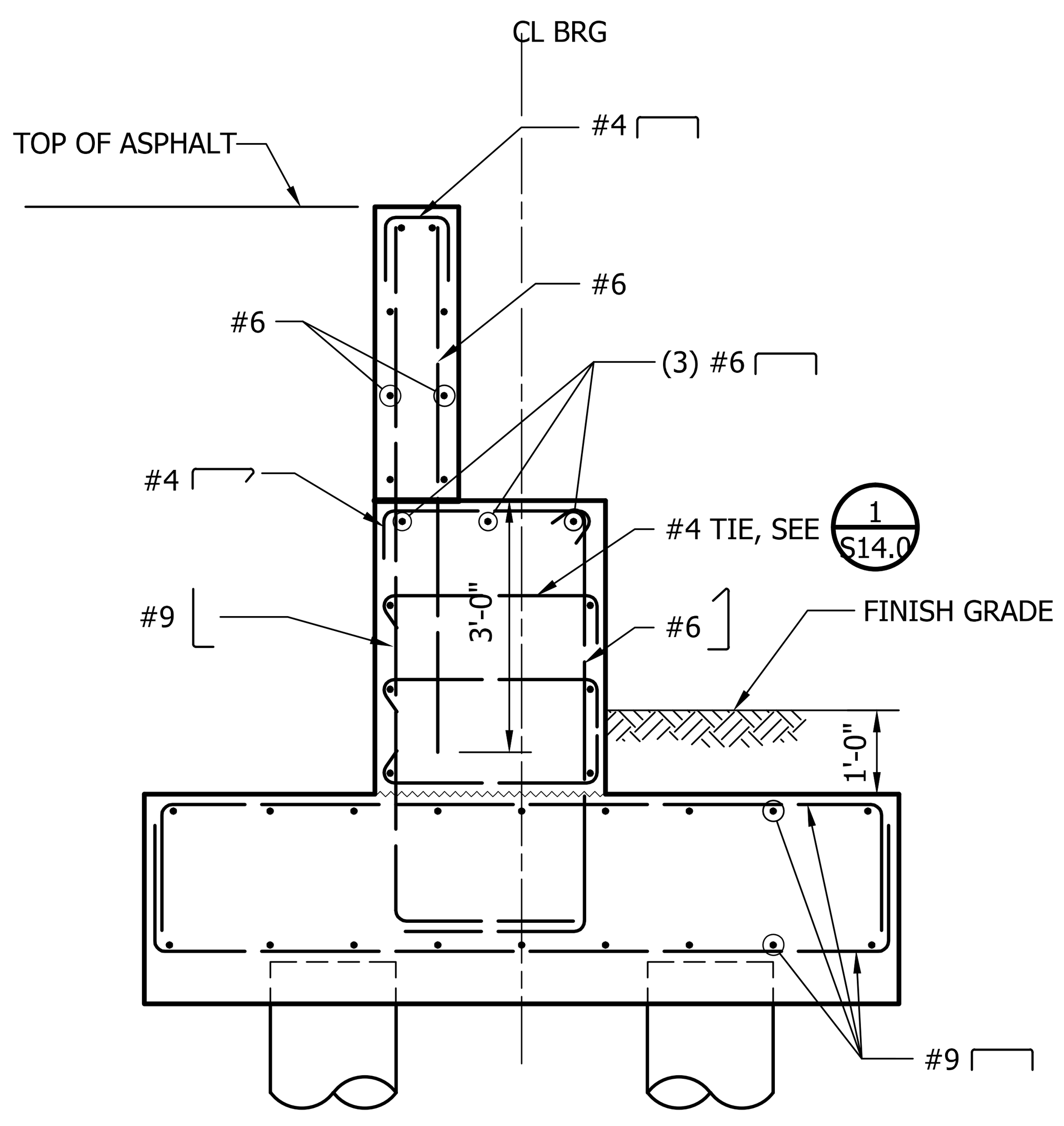
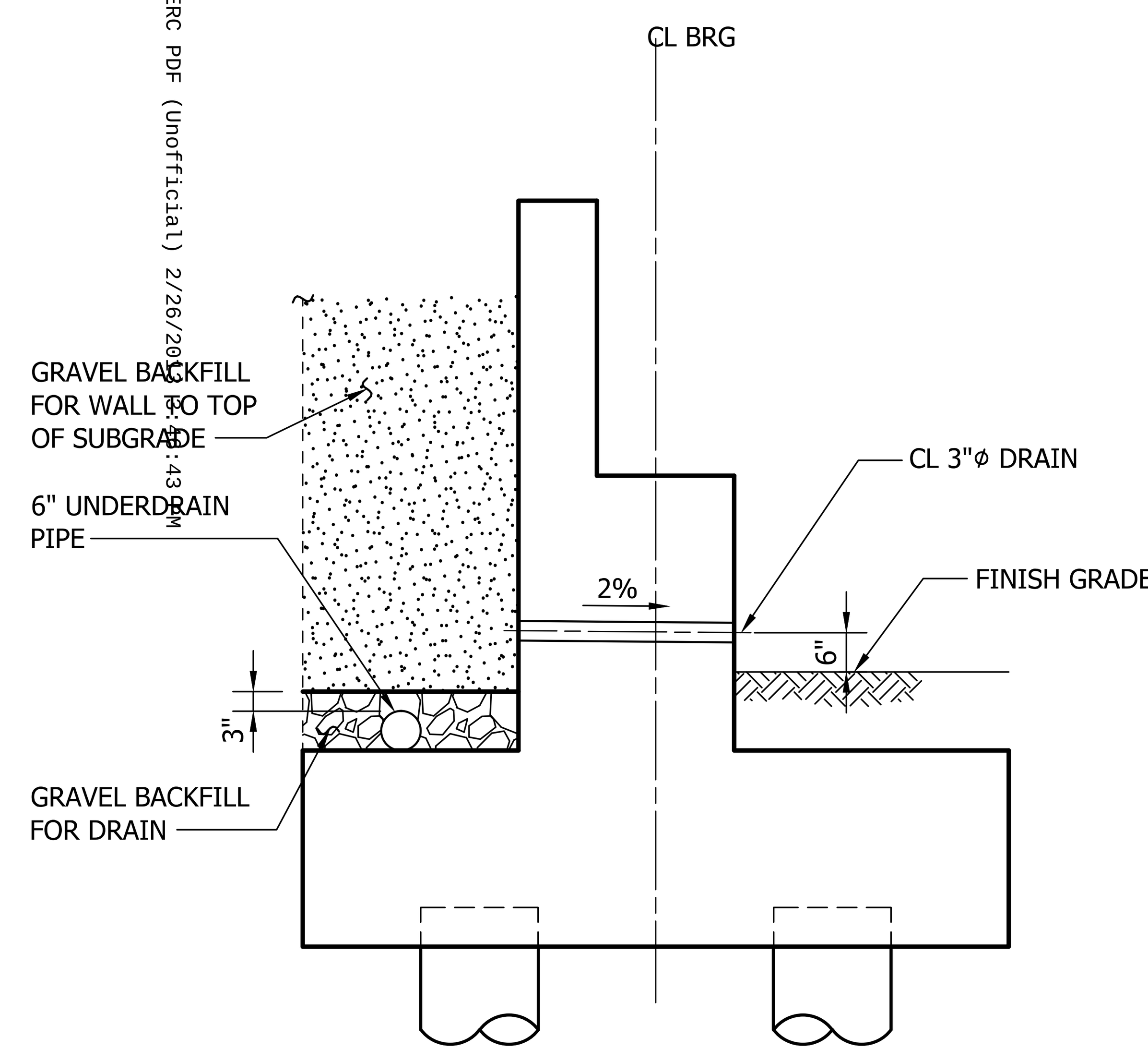
SHT. NO. S10.0

SCALE: AS SHOWN



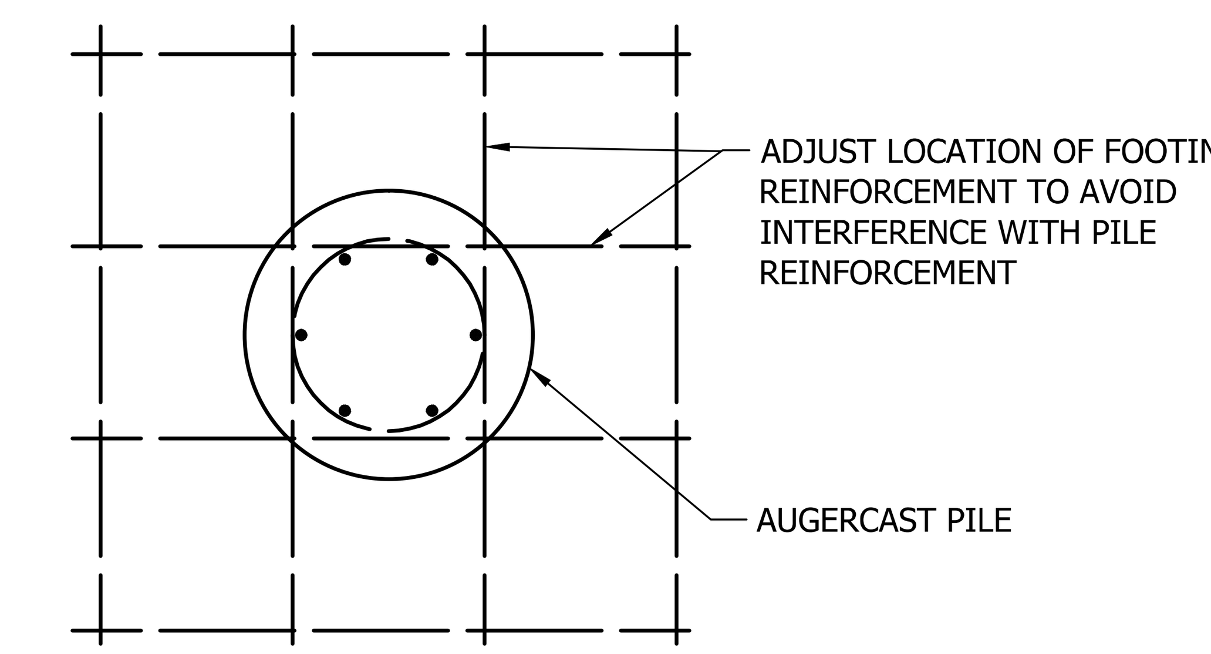
A S9.0 S10.0 SECTION

1' 4' 0 2'



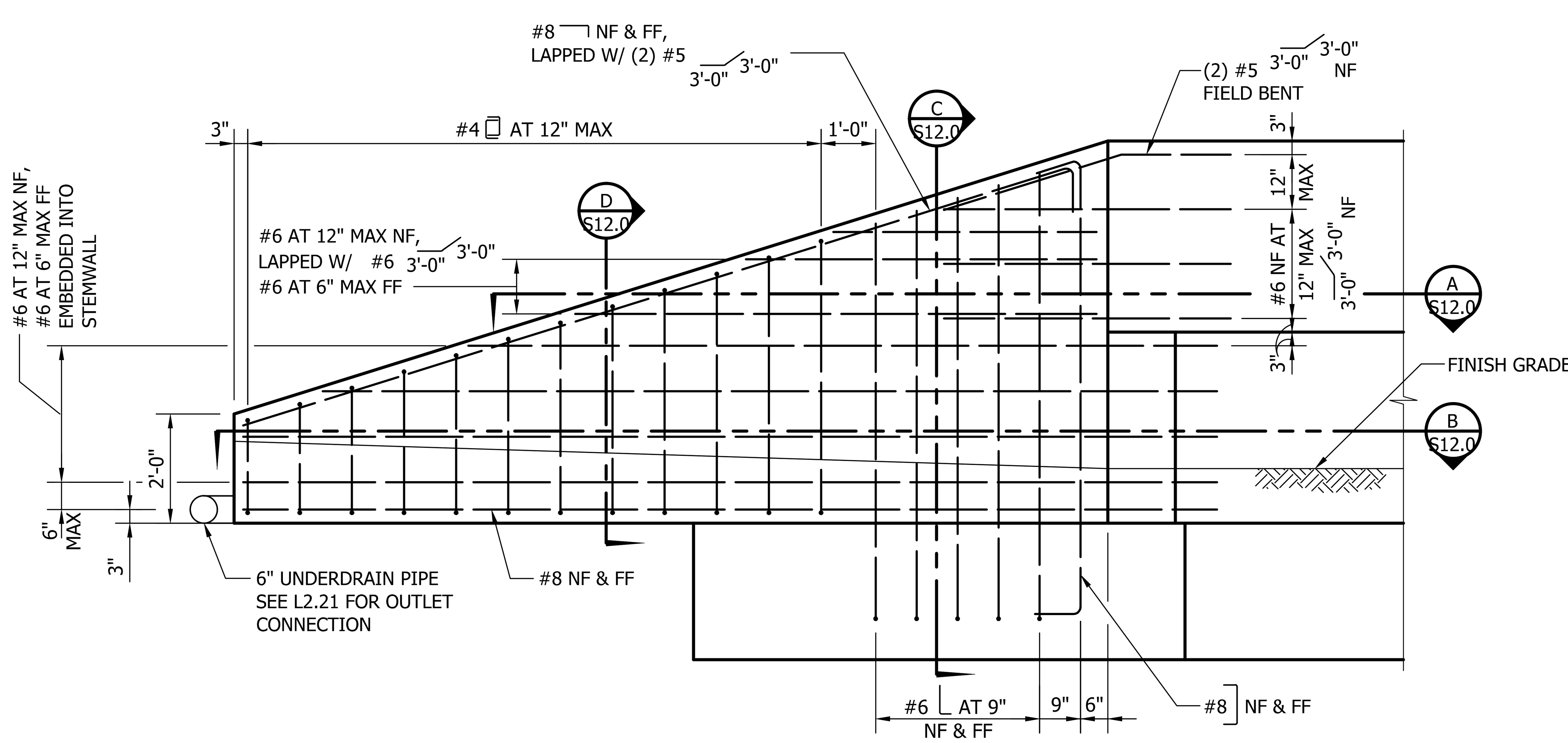
1 S5.0 DRAINAGE DETAIL

1' 4' 0 2'



2 - AUGERCAST PILE/ FOOTING REINFORCEMENT

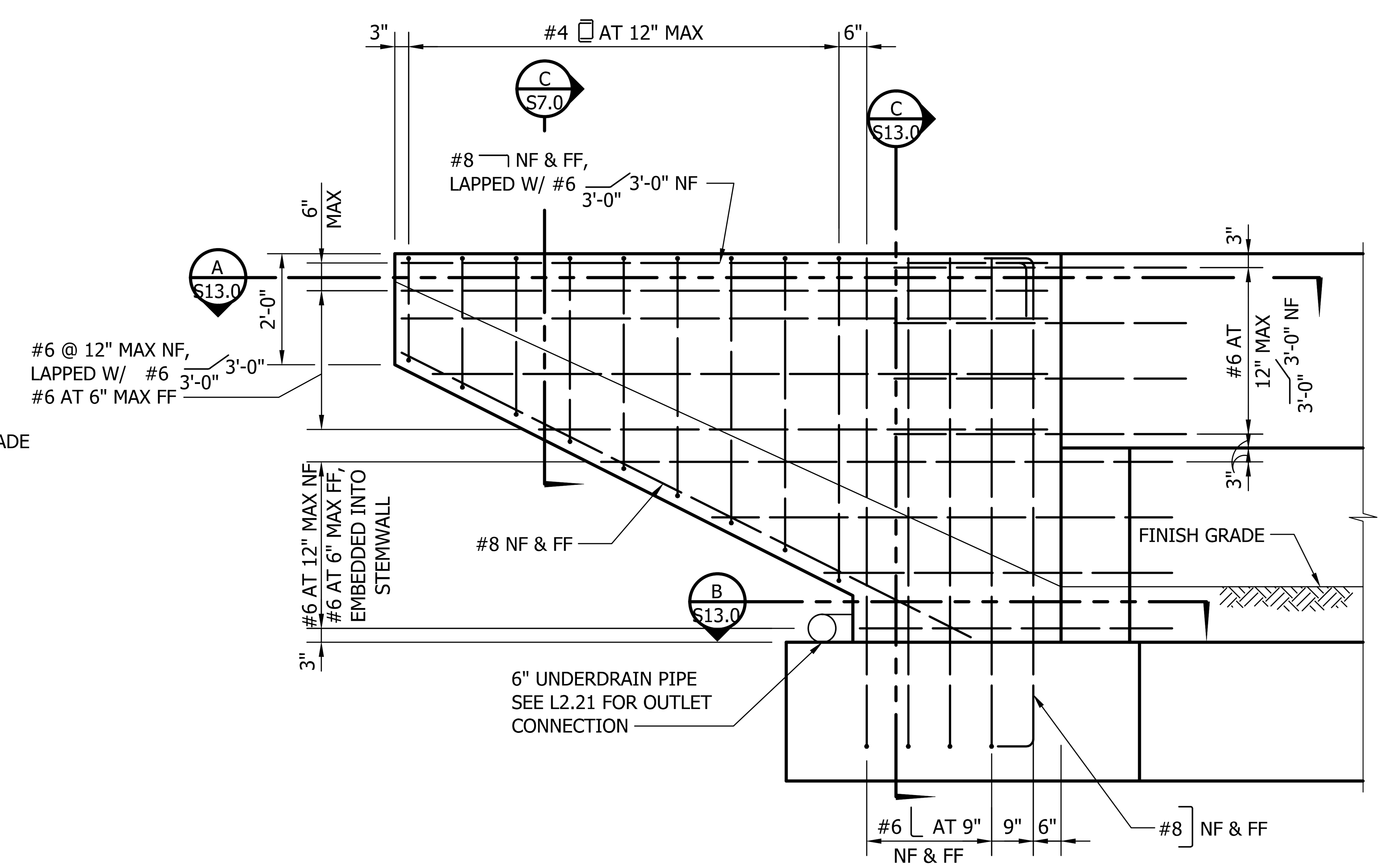
1' 0 2'



B S9.0 VIEW

1' 4' 0 2'

NOTE: AUGERCAST PILES NOT SHOWN.



C S10.0 VIEW

1' 4' 0 2'

NOTE: AUGERCAST PILES NOT SHOWN.

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ROCKY REACH TRAIL PHASE ONE

NORTH BRIDGE ABUTMENT SECTIONS 1

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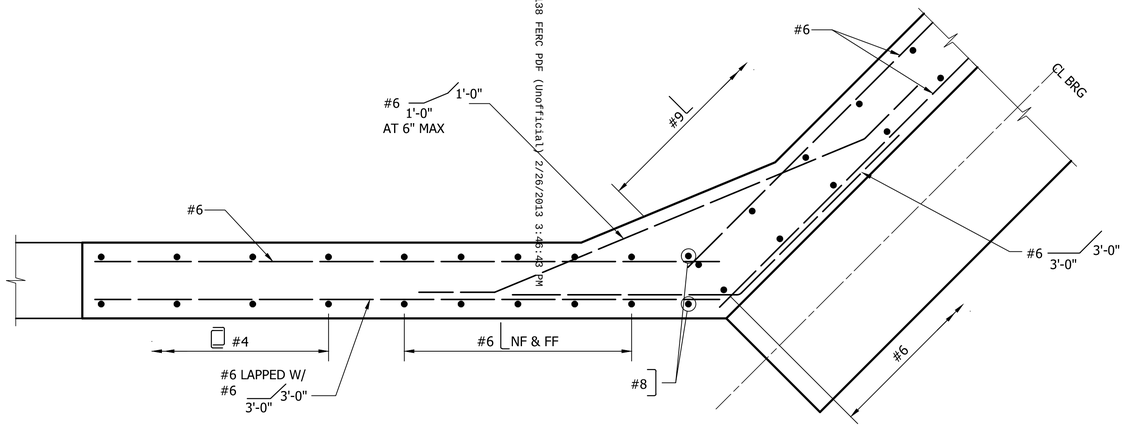
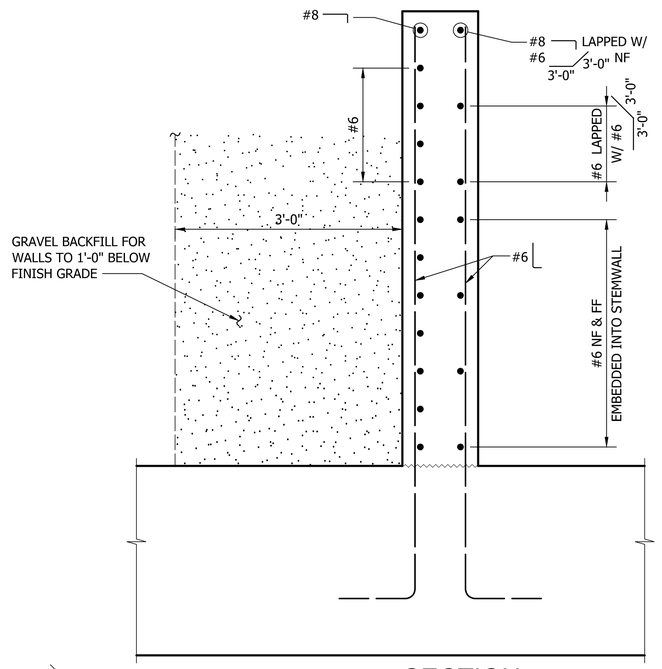
SHT. NO. S11.0

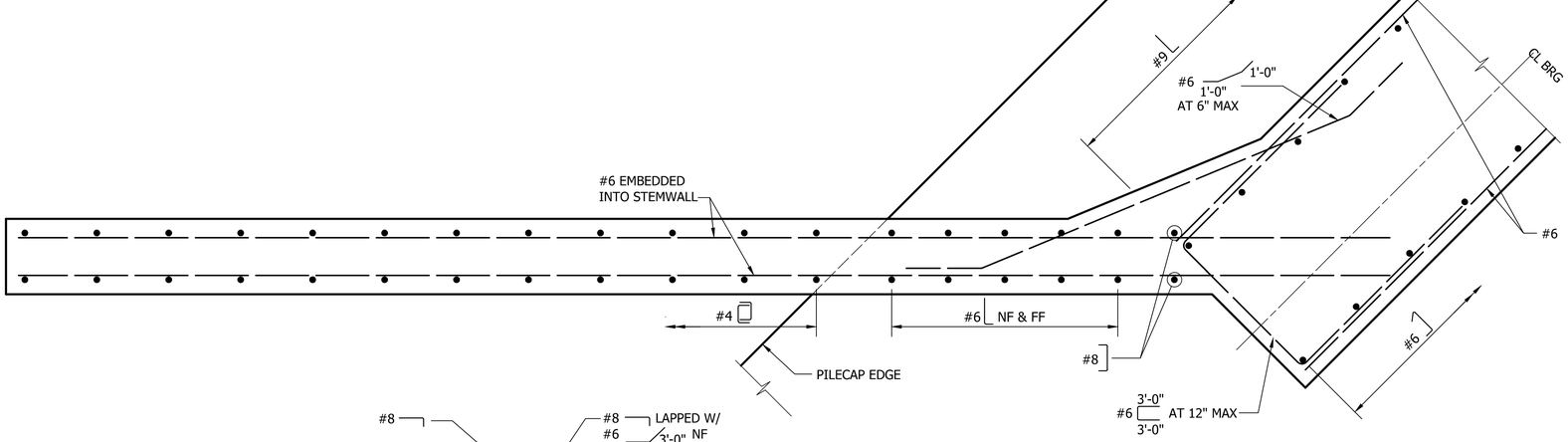
SCALE: AS SHOWN



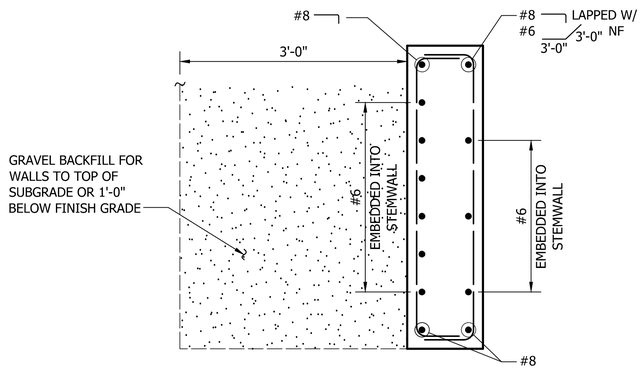
A S11.0 SECTION



C S11.0 SECTION



B S11.0 SECTION



D S11.0 SECTION

1' 0 2'

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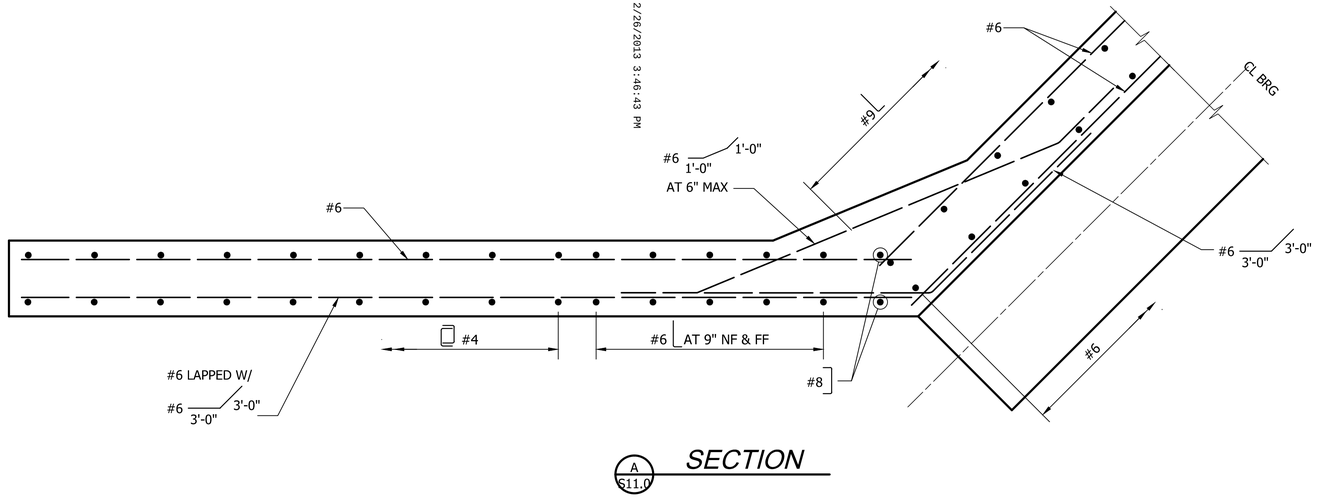
WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

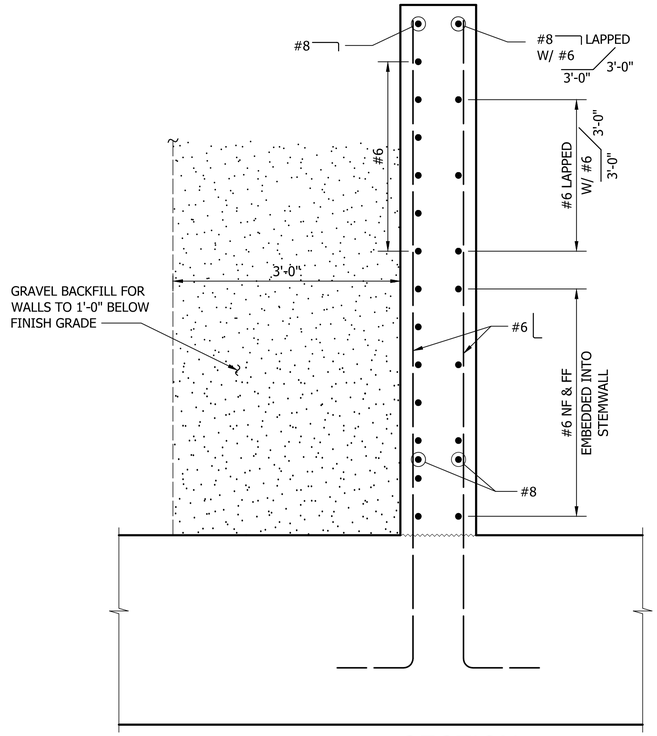
NORTH BRIDGE ABUTMENT SECTIONS 2

S12.0
SHT. NO.

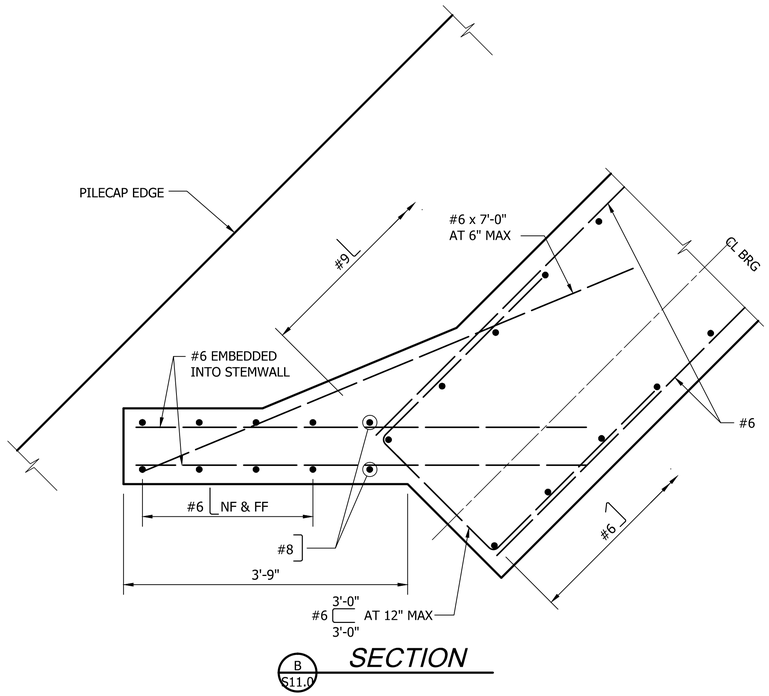
SCALE: AS SHOWN



A S11.0 SECTION



C S11.0 SECTION



B S11.0 SECTION

1' 0 2'

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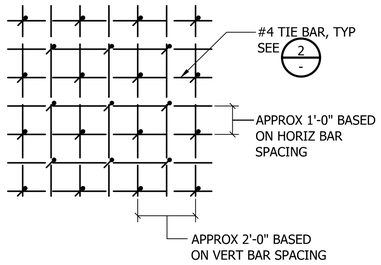
ROCKY REACH TRAIL
PHASE ONE

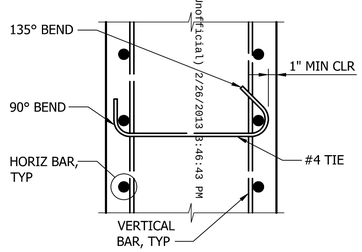
NORTH BRIDGE ABUTMENT SECTIONS 3

SHT. NO. S13.0

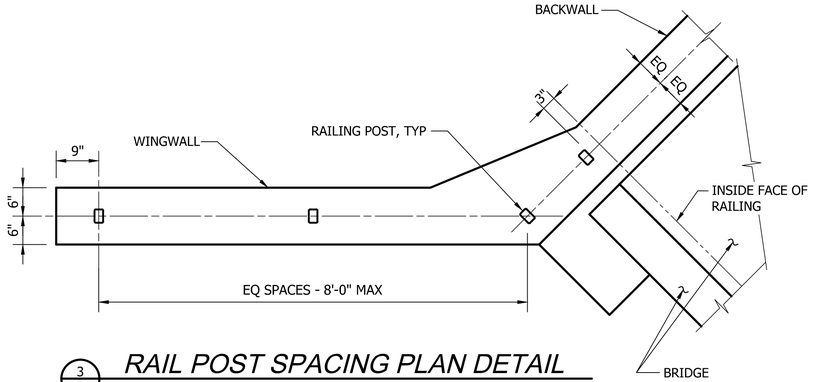
SCALE: AS SHOWN



1 - TIE SPACING DETAIL

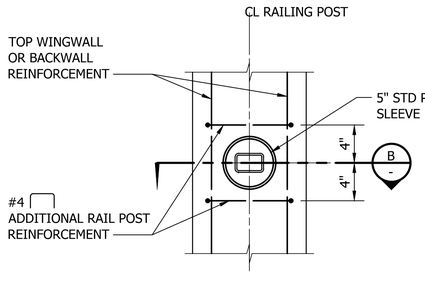


2 - TIE BAR DETAIL
ALTERNATE HOOKS ON TIE BAR

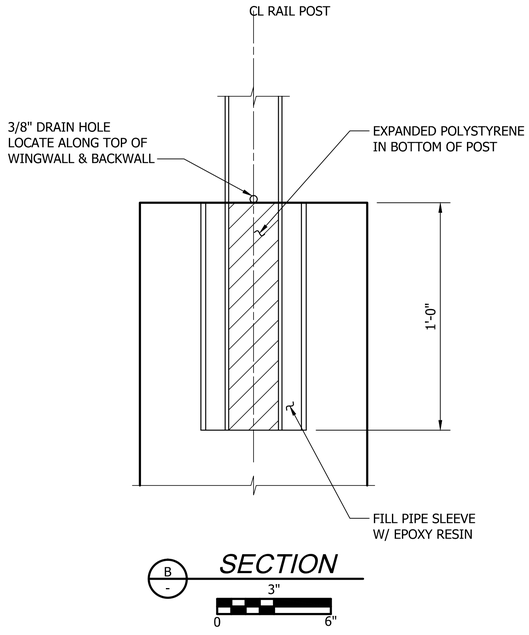


3 - RAIL POST SPACING PLAN DETAIL
SCALE: 3/4" = 1'-0"

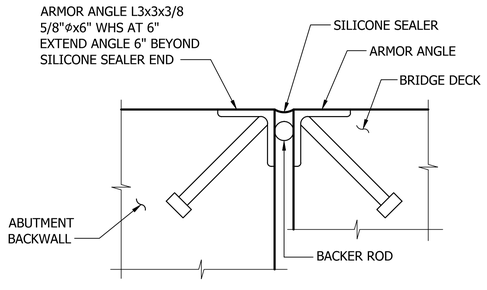
4 - RAILING DETAIL
6"
0
1'



A - SECTION
6"
0
1'



B - SECTION
3"
0
6"



NOTE: SILICONE SEALER, ARMOR ANGLE, AND BACKER ROD PER BRIDGE SUPPLIER

5 - EXPANSION JOINT DETAIL
3"
0
6"

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WASHINGTON STATE PARKS AND RECREATION COMMISSION

ROCKY REACH TRAIL
PHASE ONE

MISCELLANEOUS DETAILS

SHT. NO. S14.0

SCALE: AS SHOWN

ROCKY REACH TRAIL PHASE 1

SOIL EROSION AND SEDIMENT CONTROL PLAN

Public Utility District No. 1 of Chelan County (District) owns and operates the Rocky Reach Hydroelectric Project. Rocky Reach Dam is located on the Columbia River approximately five miles north of Wenatchee, Washington (about Columbia River mile 473.5). As a requirement of the Rocky Reach FERC license the District also owns Lincoln Rock State Park. The park is located on the Rocky Reach Reservoir as illustrated below.

In accordance with the FERC License Order dated February 19, 2009 and the Order on Rehearing & Clarification dated May 21, 2009, for the Rocky Reach Hydroelectric Project No. 2154, the District will participate with Washington State Parks in the construction of a paved one-mile long, non-motorized trail from Lincoln Rock State Park to a fish bypass viewing station. State Parks and the District intend to start construction of the trail during 2013.

The following constitutes the Soil Erosion and Sediment Control Plan (ESCP) as required in Article 303 of the FERC License Order.

1.0 SCOPE OF WORK

The scope of work for the project titled: Rocky Reach Trail Phase 1 includes:

- Clearing and grubbing
- Site excavation and embankment for 5,335 linear feet of trail
- Structural concrete construction
- Bridge erection
- Surfacing placement
- Hot mix asphalt paving
- Site furnishings and signage
- Site restoration

Temporary and permanent erosion control measures will be employed during construction. The locations and details for these measures are as shown in the construction drawings.

2.0 IMPLEMENTATION

All sediment and erosion control measures will be performed by the Contractor in accordance with the contract plans, specifications and Temporary Erosion and Sediment Control (TESC) Plan.

The Contractor shall identify the Erosion and Sediment Control (ESC) Lead at the preconstruction conference and in the TESC plan. The ESC Lead shall have, for the life of the contract, a current Certification of Training in Construction Site Erosion and Sediment Control from a course approved by Washington State Department of Ecology.

The ESC Lead shall implement the TESC Plan. Implementation shall include, but is not limited to:

1) Prior to ground disturbing activity, the installation and maintenance of all temporary erosion and sediment control Best Management Practices (BMPs) included in the TESC Plan to assure continued performance of their intended function. Damaged or inadequate TESC BMPs shall be corrected immediately.

2) Updating the TESC Plan to reflect current field conditions.

3) Inspect all areas disturbed by construction activities, all on-site erosion and sediment control BMPs and all stormwater discharge points every calendar week and within 24 hours of run-off events in which stormwater discharges from the site, or as directed by the Construction Manager.

The Owner's Construction Manager and/or Inspectors will monitor TESC measures on a daily basis for compliance with contract provisions.

3.0 CONCLUSION

Construction of the Rocky Reach Trail Phase 1 includes the implementation of both temporary and permanent measures to control sedimentation and erosion within the project work limits consistent with Article 303 of the FERC license order.

Quality Control and Inspection Plan

Rocky Reach Trail Phase 1

Rocky Reach Hydroelectric Project
FERC Project No. 2145

January 2013

Table of Contents

Appendices

1. Introduction

Public Utility District No. 1 of Chelan County (District) owns and operates the Rocky Reach Hydroelectric Project. Rocky Reach Dam is located on the Columbia River approximately five miles north of Wenatchee, Washington (about Columbia River mile 473.5). As a requirement of the Rocky Reach FERC license the District also owns Lincoln Rock State Park. The park is located on the Rocky Reach Reservoir as illustrated below.

In accordance with the FERC License Order dated February 19, 2009 and the Order on Rehearing & Clarification dated May 21, 2009, for the Rocky Reach Hydroelectric Project No. 2154, the District will participate with Washington State Parks in the construction of a paved one-mile long, non-motorized trail from Lincoln Rock State Park to a fish bypass viewing station. State Parks and the District intend to start construction of the trail during 2013.

State Parks staff, along with some outside A&E consultants, will oversee construction of the project. All construction activities will take place within the park, surrounding open project lands, and Washington State Department of Transportation right of way, therefore no impacts or damage risk will be imposed on the Rocky Reach project.

2. Organization and Staffing Responsibilities

A. Titles, duties, and responsibilities of staff

An organizational chart for this project is presented in Appendix A. Responsibilities for maintaining quality assurance are as follows:

(1) Construction Manager

George A. Rapozo, Jr., P.E.
Washington State Parks
270 9th Street NE, Suite 220
East Wenatchee, WA 98802
Telephone: (509) 665-4338 office
(509) 670-2725 mobile
(509) 667-9288 home

- Review the Contractor's Quality Control documents and project reports.
- Provide Quality Assurance by personally observing or observing via an inspection team the construction on a full-time basis to ensure compliance with contract drawings and specifications.
- Maintain daily records of type, quantity, location, and quality of construction work.
- Conduct construction progress meetings, prepare documentation of meeting discussions, and distribute copies of the discussions.
- Perform final inspection with Project Team.
- File daily inspection reports, review/evaluation of material submittal reports as appropriate.
- Prepare contract change orders and progress payments.

(2) Park's Engineer, 1st Alternate

Erik V. Folke, P.E., L,S.I.T.
Washington State Parks
270 9th Street NE, Suite 220
East Wenatchee, WA 98802
Telephone: (509) 665-4332 office
(509) 679-2548 mobile

- Review Contractor's submittals.
- Provide ongoing technical assistance/clarification, as needed.
- Provide on-site inspection, as requested, at critical phases of construction.

- Provide clarification of drawings and specifications.
- Provide project record drawings at completion of the project.
- Periodically review quality control documents and project reports.

(3) Testing Service(s)

To be determined on an as-needed per project basis. Likely testing services include geotechnical (density tests, compressive strength, etc.) and hot mix asphalt testing.

(4) Construction Contractor(s)

- Construct project to meet project specifications.
- Perform testing and inspection as necessary to control the quality of the work.
- Submit documents, material certificates, shop drawings, product data, and testing results to Parks and/or Consultant Engineer as specified in the *Technical Specifications*.
- Implement and monitor jobsite safety program and be responsible for jobsite safety.
- Implement and monitor construction techniques and procedures so that project quality control standards will be met as verified by the Construction Manager, Inspectors and Engineers.

B. Approval and rejection of work

The approval and rejection of work will be subject to the judgment of the Construction Manager.

C. Authority to stop work

The Construction Manager will have the authority to stop work on the project.

D. Resumes

Resumes of key quality control personnel are included in Appendix B.

3. Inspection Plan and Field Practices

A. Inspection criteria

Criteria for evaluating the quality of work under the contract are contained in the specifications, drawings, and other contract documents. The following items will be completed to make effective use of the contract documents.

- Prior to the start of work at the site, the Construction Manager and Inspectors shall take the time to become familiar with the contract documents.

- The Construction Manager and Inspectors shall review relevant portions of the documents daily as the work progresses.
- The Construction Manager shall ensure that the Inspection staff receives copies of any revisions to the contract documents in a timely manner and shall discuss the revisions to ensure a common understanding of them.

B. Inspection equipment and resources

The independent testing service will supply qualified personnel and appropriate testing equipment to satisfy the requirements of the Quality Control Inspection Plan (QCIP).

C. Contractor operations

The Contractor is responsible for choosing equipment and methods adequate to perform the work specified in the contract documents and for actually achieving the required results. For this reason, the Construction Manager will avoid direction or control of the Contractor's operations. The Construction Manager is responsible for verifying that the contract documents are being followed and the required results are being/have been achieved.

D. Coordination with Contractor's schedule

The Contractor's proposed construction schedule will be submitted after the contract is awarded at the preconstruction conference. The Contractor will be required to notify the Owner 48 hours in advance of starting, or restarting, on-site construction activities.

E. QCIP operations

The Construction Manager and Inspectors are chiefly responsible for observing details of the Contractor's work as it progresses to verify that it meets requirements of the contract documents. This will require the Construction Manager and Inspectors to:

- Be familiar with the contract documents, including the technical specifications and drawings.
- Be present at key times to verify and approve items as they come up.
- Be present to observe and document progress of the work as outlined below.
- Understand the intent of the drawings and specifications as a basis for exercising judgment, as appropriate, during the work.

The Construction Manager and Inspectors shall notify the Contractor immediately upon discovery of any item of work, completed or in progress, which does not meet requirements of the contract documents.

If conditions are encountered that require redesign or substantial modification of the work, the Inspector shall contact the Construction Manager and Engineer for guidance. The contact shall be made in a timely manner to avoid or minimize delay of the work.

If the Construction Manager or Inspector observes work being performed by the Contractor in such a way that it could negatively impact human safety or cause significant damage to property, he shall immediately notify the Contractor. If the problem is not addressed by the Contractor in a timely manner, the Construction Manager or Inspector shall issue an order to the Contractor to stop work until the apparent problem is resolved.

The Construction Manager or designated Inspector also is present to serve as the interface between the Contractor and the Owner's other personnel on site. For this purpose, the Construction Manager or Inspector will be present at all times when the Contractor is working on the site.

F. Frequency of inspections

During construction, the Independent Testing Service and Engineer will perform site visits as required to comply with the specifications. Full-time observation services will be provided by the Construction Manager during fieldwork. The Engineer will provide on-site inspection during critical phases of the construction, as requested by the Construction Manager.

4. Documentation

The Construction Manager is responsible to maintain certain records as the construction progresses. The types of documentation are outlined below, and sample forms are attached, as appropriate.

A. Preconstruction Conference Report

Prior to the start of physical construction, a preconstruction conference will be held for the project. The preconstruction conference will cover all aspects of the project including, but not limited to, the proposed project schedule, contract document requirements, safety for workers and public, environmental protections and work limits.

B. Inspection Reports

An Inspection Report (IR) will be completed daily to document work progress, site conditions, non-conforming work, and other relevant items. A sample IR form is presented in Appendix C.

C. Material Test Reports

Materials sampling and test reports will describe the type and location of the material being tested, as well as the date, time, and weather conditions when obtaining the sample or performing the test. A record of the tests performed, applicable standards, and test results shall be distributed to the Engineer and Contractor.

D. Maintenance of records

All documents, correspondence, and data pertaining to the project must be clearly identified, organized, and filed with the Construction Manager. The Construction Manager will maintain one set of record drawings in the field for use in preparing final record drawings.

E. Photographs

Photographs of significant construction activities will be taken throughout the construction period by the Construction Manager and Inspectors. All photographs will be dated with identification, as appropriate, of the object being photographed.

5. Training

No formal training is proposed for this project.

6. Material Testing

The number and type of tests to be performed during construction are presented in the specifications and drawings. Material testing will be performed in accordance with the Contract Documents, by an Independent Testing Service where appropriate.

7. Environmental Compliance

A. Environmental Compliance Plan

The proposed construction will involve clearing/grubbing, site excavation and embankment, structural concrete construction, bridge erection, surfacing placement, asphalt paving, and other minor items of construction to build a linear non-motorized trail. The Contractor will submit the following in order to ensure environmental compliance:

- Adopt or modify the Temporary Erosion and Sediment Control plan (TESC).
- Prepare and implement a project-specific spill prevention, control and countermeasure plan for the project.

B. Frequency of inspections

Inspections for compliance with the contract requirements will take place on a daily basis during active physical construction. The Contractor's Erosion and Sediment Control (ESC) lead shall inspect all areas disturbed by construction, ESC facilities and discharge points every calendar week and within 24 hours of runoff events, in which stormwater discharges from the site.

C. Documentation and corrective actions

Documentation of any observed violations of environmental requirements of the Contract Documents will be included in an IR along with their resolution. The ESC lead will be required to complete and Erosion and Sediment Control Inspection Form (WSDOT Form 220-030) after each inspection.

8. Start and finish dates

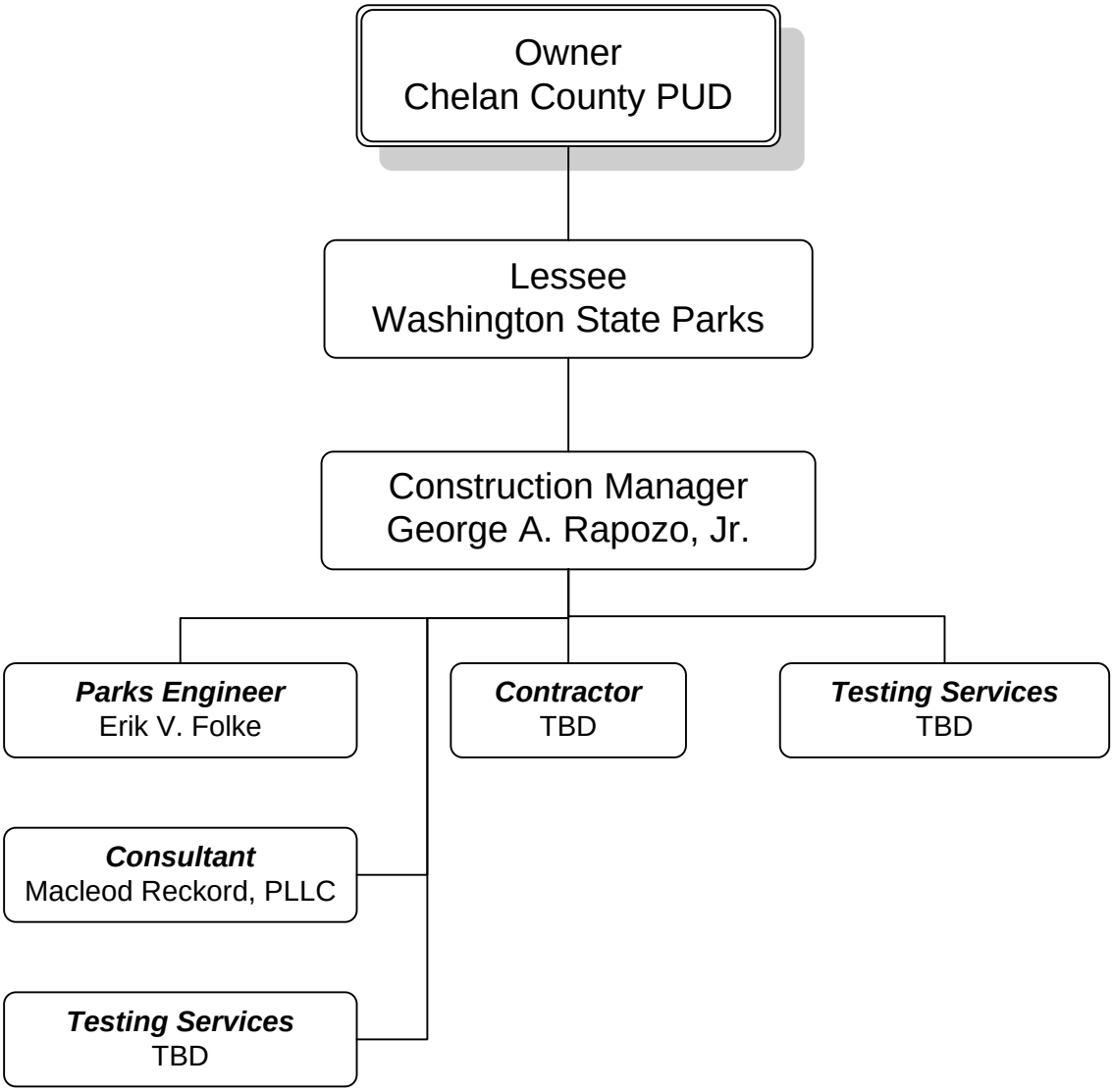
Construction of the project is anticipated to begin May 2013 and be completed before the end of the year.

9. Planned use of consultants

Consultants were used for the project design. They will be involved on a limited basis during construction for review and approval of the bridge shop drawings/submittals, along with any requests for clarification of the drawings and/or specifications.

Appendix A. Organizational Chart

Organizational Chart
Rocky Reach Project
Rocky Reach Trail Phase 1



Appendix B. QCIP Personnel Resumes

George A. Rapozo, Jr., P.E.

Education:

1983 Bachelor of Science Civil Engineering, Washington State University

Professional Registration:

1992 Professional Civil Engineer, Washington

Work History:

2009 – Present	Construction Project Coordinator 4, Washington State Parks and Recreation
1997 – 2009	Region Engineering Manager, Washington State Parks and Recreation
1985 – 1997	Civil Engineer, Washington State Parks and Recreation
1984 – 1985	Engineering Technician, City of Moses Lake
1980 – 1983 (summers)	Equipment Operator, Grant County Public Works

Professional Experience:

Mr. Rapozo has over 29 years of engineering design, construction administration, and project management experience in the civil engineering field. As a civil engineer, he has worked on projects involving roadway, bridge, site development, water supply and transmission, sewage collection and treatment, stormwater collection and treatment, marine facilities, site electrical, building, and agricultural engineering. Working as Project Engineer, Mr. Rapozo has been responsible for all aspects of engineering during the design and construction phases of public works projects including plans, specifications, estimates, utility negotiations, construction staking and inspection. As Construction Project Coordinator and Engineering Manager, Mr. Rapozo has been responsible for the management of engineering services for the Eastern Region of State Parks. These management responsibilities include the organization, direction, and supervision of professional engineers, engineering technicians, construction project managers, and consultants through all phases of the Parks' Capital budget implementation.

Key Projects:

- **Mount Spokane:** Road Construction, Stages 1, 2, 2B, and 2C
- **Lewis & Clark:** Park Wide Water System
- **Wanapum Recreation Area:** Boat Ramp and Parking Improvements
- **Maryhill State Park:** Boat Ramp and Parking Improvements
- **Conconully State Park:** Comfort Station and Parking Improvements
- **Moran State Park:** Mount Constitution Road Guardrail
- **Potholes State Park:** Boat Ramp and Parking Improvements
- **Larrabee State Park:** Park Improvements
- **Pearrygin Lake State Park:** Water System Improvements
- **Twenty Five Mile Creek:** Marine Improvements
- **Centennial Trail State Park:** West Link Bridge

Erik V. Folke, P.E., L.S.I.T.

Education:

1992 Master of Science in Civil Engineering, GIS Course Emphasis, University of Washington
1984 Bachelor of Science in Civil Engineering, Summa cum Laude
1981 Associate of Science in Land Surveying

Professional Registrations:

1997 Land Surveyor-In-Training, Washington
1993 Professional Civil Engineer, Washington

Work History:

2009 – Present	Region Engineering Manager, Washington State Parks and Recreation
1999 – 2009	Civil/Environmental Engineer, Washington State Parks and Recreation
1992 – 1999	Transportation Engineer, Washington State Department of Transportation
1987 – 1990	Remote Sensing Analyst, National Board of Survey, Finland
1985 – 1987	Civil Engineer, Jensen and Lockman Consulting Engineers

Professional Experience:

Mr. Folke has performed Civil Engineering for the past 32 years, 14 of which have been with Washington State Parks and Recreation Commission. As the Region Engineering Manager he is responsible for project design, public works contract administration, and surveying for location, design, and construction. The variety of projects Mr. Folke has completed includes new campgrounds, trails, roads, water wells, development of water distribution systems, buildings, sewage lift stations, gravity sewers, sewage lagoons, campground electrification, landscaping, and irrigation. Prior to his tenure with Washington State Parks, Mr. Folke worked as a Transportation Engineer for the Washington State Department of Transportation where he was responsible for design, surveys, and construction administration for highways, bridges, drainage, and other highway facilities. Mr. Folke also worked for a private consulting engineering firm for 3 years. There he completed site designs for commercial and residential subdivisions, and worked in the specialty field of methane gas engineering, providing remediation of natural and manmade sources and protection of new and existing facilities adjoining these sources. Mr. Folke has worked out of state and abroad as a surveyor for the Army Corps of Engineers, for several highway departments, and as an analyst of satellite imagery in Finland.

Key Projects:

- **Campgrounds:** Sun Lakes-Dry Falls, Riverside State Park
- **Comfort Stations:** Curlew Lake, Columbia Hills State Park, Sun Lakes-Dry Falls
- **Marine Projects:** Wanapum, Steamboat Rock State Park, including Jones & Osborn Bay
- **Roads:** Mt. Spokane State Park and components of other State Park projects
- **Trails:** Columbia Plateau Trail, Centennial Trail
- **Test Wells:** Bridgeport State Park, Pearrygin Lake State Park
- **Domestic Water and Wellhead Development:** Bridgeport, Spokane House, Lewis & Clark Trail, Sun Lakes-Dry Falls, Wanapum, Steamboat, Camp Wooten & Central Ferry
- **Lift Stations, Force Mains, and Lagoons:** Curlew, Osoyoos Lake, Sun Lakes-Dry Falls
- **RV Electrification:** Lake Wenatchee, Yakima

Appendix C. Construction Management Sample Forms

SAMPLE FORM

STATE OF WASHINGTON
WASHINGTON STATE PARKS AND RECREATION COMMISSION
CAPITAL PROGRAM
270 9th Street NE, Suite 200, East Wenatchee, Washington 98802 • (509) 665-4343 • Fax (509) 886-0478
Internet Address: http://www.parks.wa.gov

_____ (Date)

TO: File

FROM: _____

SUBJECT: **Contract #EW-###, Rocky Reach Trail Phase 1**
Preconstruction Conference Report

Date of Contract Award

_____ *(fill in)*

Notice to Proceed Date

_____ *(fill in)*

Contract Calendar Days

_____ *(fill in)*

Contract Termination Date

_____ *(fill in)*

Date of Preconstruction Conference

_____ *(fill in)*

Representing the Contractor

_____ *(fill in)*

Representing the State

_____ *(fill in)*

SAMPLE FORM

CONTACT INFORMATION

TITLE	NAME	ADDRESS	TELEPHON	EMAIL
Contractor				
Company Name				
Project Manager		"	"	
Superintendent		"	"	
Safety Engineer		"	"	
Office Manager		"	"	
State Parks				
Project Engineer		270 9th Street NE, Ste 200, East Wenatchee, WA 98802		
Field Inspector		270 9th Street NE, Ste 200, East Wenatchee, WA 98802		
Park Manager				

All correspondence pertaining to this project shall be directed to _____ _____. *(fill in)*

Coordinate all field operations, materials approvals, and onsite inspections with the Field Inspector.

All correspondence from subcontractors is to be routed through the prime contractor.

PLANS/SPECIFICATIONS

DESCRIPTION	NUMBER FURNISHED
Plans/Special Provisions (Large Set)	
Plans/Special Provisions (Half Size)	

FORMS TO BE RECEIVED FROM THE CONTRACTOR

DESCRIPTION	RECEIVED (YES/NO)	EXPLANATION
Subcontractors List		
Progress Schedule (Bar Graph)		
Lump Sum Bid Breakdown		
MSDS Sheets		
Critical Materials Order List		
Letter Designating Superintendent		

SAMPLE FORM

INFORMATION TO BE FURNISHED BY UTILITIES

INFORMATION	OWNER	REPRESENTATIVE	SPECIFIC ITEMS
Gas			
Water			
Sewer			
Power			
Telephone			

DISCUSSION CHECKLIST

1. CONTRACTOR'S RESPONSIBLE REPRESENTATIVE

   Discussed requirement for superintendent (or approved designee) to be at the project at all times while work is in progress.

2. APPROVAL OF QUALIFIED SUBCONTRACTORS

   Discussed subcontracts per General Conditions of the Contract.

3. SCHEDULE OF VALUES BREAKDOWN

   Delivery of a lump sum breakdown at the preconstruction conference is mandatory. No construction shall take place until the breakdown is in the Engineer's possession, and has been approved.

4. PAYMENTS

   Payments are made on or about the 20th of each month based on completed, in place work.

   The process of reviewing the schedule of values with the project superintendent was discussed. No payments will be processed unless the Schedule Of Values and up-to-date Construction Progress Schedule are on file with the Engineer.

5. STATEMENT OF INTENT TO PAY PREVAILING WAGES AND OTHER REGULATORY RELEASES

   Discussed payment holdups if Intent(s) are not filed for by the contractor and subcontractors before payments are made for work completed. The remaining releases required for payment of contract retainage (Part 6.03 of Supplemental General Conditions) was also discussed. If filed using manual methods, please send the original approved forms to Brett Taylor, Washington State Parks and Recreation Commission, Financial Services Section, Post Office Box 42659, Olympia, Washington 98504-2659. A copy of all forms and releases shall be mailed to the Project Engineer.

6. ENVIRONMENTAL AND CULTURAL RESOURCE PERMITTING

   _____ *(fill in)*

SAMPLE FORM

7. ARCHAEOLOGICAL

If any archaeological material is discovered, the contractor shall immediately stop work and contact our office and the park manager.

8. SAFETY FOR THE CONTRACTOR, OWNER, AND THE PUBLIC

The contractor shall comply with all applicable WISHA and OSHA rules. The contractor shall erect safety fencing around all excavations that will be left open overnight.

9. EMERGENCY SPILL RESPONSE NOTIFICATION

Under state law, Ecology must be notified when any amount of regulated waste or hazardous material that poses an imminent threat to life, health, or the environment is released to the air, land, or water, or whenever oil is spilled on land or to waters of the state. The spiller is always responsible for reporting a spill. Failure to report a spill in a timely manner may result in enforcement actions. If you are not responsible for a spill, making the initial notification does not make you liable for the spill. However, please consult with Ecology's response team before attempting any type of response or cleanup.

If oil or hazardous materials are spilled to state waters, the spiller must notify both federal and state spill response agencies. The federal agency is the **National Response Center** at 1-800-424-8802. For state notification, call the Washington Emergency Management Division (EMD) at **1-800-258-5990** or **1-800-OILS-911** ***AND*** the appropriate Ecology regional office for your county (*see numbers below*). An Ecology spill responder will normally call the reporting party back to gather more information. The agency will then determine its response actions.

Ecology Regional Spill Reporting Numbers:

- **Northwest Regional Office: (425) 649-7000** (*Island, King, Kitsap, San Juan, Skagit, Snohomish, and Whatcom counties)* **TDD:** (425) 649-4259*
- **Southwest Regional Office: (360) 407-6300** *(Clallam, Clark, Cowlitz, Grays Harbor, Jefferson, Lewis, Mason, Pacific, Pierce, Skamania, Thurston, and Wahkiakum counties)* **TDD:** (360) 407-6306*
- **Central Regional Office: (509) 575-2490** *(Benton, Chelan, Douglas, Kittitas, Klickitat, Okanogan, and Yakima counties)* **TDD:** (509) 454-7673*
- **Eastern Regional Office: (509) 456-2926** *(Adams, Asotin, Columbia, Ferry, Franklin, Garfield, Grant, Lincoln, Pend Oreille, Spokane, Stevens, Walla Walla, and Whitman counties)* **TDD:** (509) 458-2055*

*Note: TDD numbers answered during office hours only.

10. RESPONSIBILITY FOR LOCATING UTILITIES

_____ *(fill in)*

11. RESPONSIBILITY FOR DAMAGE

The contractor is responsible for any damage to existing park roads/facilities or utilities damaged by construction.

SAMPLE FORM

12. MATERIAL SUBMITTALS

Submittals as outlined in the General Requirements were discussed. Submittals are required for all materials to be incorporated into or used on the project. For both the contractor's and the Engineer's convenience, every submittal shall be assigned a number that corresponds to the submittal log. Please refer to Section 01340 of the specifications.

13. CRITICAL MATERIALS FOR THE PROJECT ARE AS FOLLOWS

_____ *(fill in)*

14. SHOP DRAWINGS REQUIRED FOR THE PROJECT

_____ *(fill in)*

15. LIST OF MATERIALS FABRICATED OR MANUFACTURED OFF THE PROJECT

_____ *(fill in)*

16. ACCEPTANCE AND APPROVAL OF THE WORK

The Engineer/Field Inspector shall be the only person who shall accept/approve work on the project.

17. INSPECTION REPORTS

Inspection reports will be prepared by the Engineer/Field Inspector for each visit to the project. Inspections will be conducted on a random basis as necessary for actual work in progress on a pre-determined schedule with the superintendent and/or as requested by the contractor.

18. CHANGE ORDERS

Change orders were discussed. If there is any major change on the project, a change order shall be prepared for the work. Field Authorizations may be granted to allow protection of life/property, and to preserve critical path. No work under a change order process may take place prior to final change order approval at Olympia.

19. INTERPRETATION OF THE PLANS/SPECIFICATIONS REQUESTED BY THE CONTRACTOR

Please immediately contact the Engineer prior to implementing the work for clarification.

20. CONFLICTS OR OMISSIONS IN THE PLANS/SPECIFICATIONS

Please immediately contact the Engineer for resolution of the conflict.

21. CONTRACTOR'S PLAN OF OPERATION AND CONSTRUCTION PROGRESS SCHEDULE

Please deliver the Construction Progress Schedule at the preconstruction conference. No construction shall take place until the schedule is in the Engineer's possession, and has been reviewed.

SAMPLE FORM

22. TYPES OF EQUIPMENT CONTRACTOR PLANS TO USE ON PROJECT

_____ *(fill in)*

23. TRAFFIC/PUBLIC CONTROL

_____ *(fill in)*

24. REVIEW OF THE PLANS/SPECIFICATIONS

_____ *(fill in)*

25. AN ON SITE INSPECTION WAS CONDUCTED WITH THE CONTRACTOR (LIST QUESTIONS/PROBLEMS.)

_____ *(fill in)*

26. CLEANUP

The project site shall be cleaned up to meet or exceed preconstruction conditions.

27. OTHER PROBLEMS OR QUESTIONS CONCERNING THE WORK

_____ *(fill in)*

SAMPLE FORM

Washington State Parks and Recreation Commission
CAPITAL PROGRAM
Eastern Region

INSPECTION REPORT

STATE PARK: _____ DATE: _____
PROJECT: _____ CONTRACT # _____
CONTRACTOR: _____ INSPECTOR: _____
REPORT PERIOD: _____ WEATHER: _____

_____
Signature

Page _____ of _____

NOTE: The Contractor will be allowed 10 days from the date of receipt of this report to protest in writing the correctness of this report, otherwise it will be accepted as correct.

White: Engineer
Yellow: Contractor
Pink: File

SAMPLE FORM

Washington State Department of Transportation

Erosion and Sediment Control Inspection

Project Name			Contract Number
ESC Lead Name		Inspection Location	
Date	Time	Current Weather Conditions	Precipitation in 24 Hours

BMP Designation	Status	BMP Location & Condition, Corrective Action, General Notes
1. **Mark Clearing Limits**: Are high visibility and perimeter fences in good condition?	☐ OK ☐ Not OK ☐ N/A	
2. **Construction Access Stabilized**: Is track-out of sediment prevented?	☐ OK ☐ Not OK ☐ N/A	
3. **Control Flow Rates**: Are flow rates causing erosion in pond, outlets, etc?	☐ OK ☐ Not OK ☐ N/A	
4. **Install Sediment Controls**: Are sediment controls in place & functioning properly?	☐ OK ☐ Not OK ☐ N/A	
5. **Stabilize Soils**: Has bare soil been protected and is erosion prevented?	☐ OK ☐ Not OK ☐ N/A	
6. **Protect Slopes**: Have slopes been protected and is erosion prevented?	☐ OK ☐ Not OK ☐ N/A	
7. **Protect Drain Inlets**: below grate filters	☐ OK ☐ Not OK ☐ N/A	
above grate protection	☐ OK ☐ Not OK ☐ N/A	
grate covers	☐ OK ☐ Not OK ☐ N/A	
check dams or other	☐ OK ☐ Not OK ☐ N/A	
8. **Stabilize Channels & Outlets**: Are conveyances (channels & outlets) Stabilized?	☐ OK ☐ Not OK ☐ N/A	

SAMPLE FORM

BMP Designation	Status	BMP Location & Condition, Corrective Action, General Notes
9. **Control Pollutants**: Are all pollutants handled & disposed of in a way that doesn't contaminate storm water?	☐ OK ☐ Not OK ☐ N/A	
10. **Control Dewatering/Water Management**: Is groundwater being dealt with in accordance with 8-01.3(1)C?	☐ OK ☐ Not OK ☐ N/A	
11. **Maintain BMPs**: Are all BMPs maintained to assure continued performance of their intended function?	☐ OK ☐ Not OK ☐ N/A	
12. **Manage the Project**: Are TESC plan sheets reflective of current field conditions?	☐ OK ☐ Not OK ☐ N/A	
Describe water discharging from site, if present. Note any suspended sediment, "cloudiness," discoloration, or oil sheen.		
This site is in compliance with the TESC and SPCC plans and the NPDES permit.	☐ OK ☐ Not OK ☐ N/A	

Comments

Document Content(s)